



Content: **Tentbook (according to EN 13782)**

Owner Tentbook: **Organic Concept**

Tent system: **20x15m Stretchtent**

Document code: **21.03.00226.1**

Author: **ir. Ruud van Bommel**

Date: **26.03.2021**

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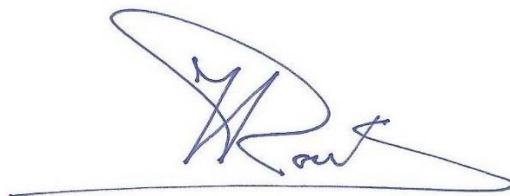
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Valid until: 26.03.2026

## Revisions

Version	Date	Revision	Paragraph
17.04.00297	24.04.2017	Document composed	
17.04.00297.2	27.06.2018	New version of 3-ply fabric	H.2.1, H.7.1, I
		Aluminium poles added	D, E, H.2.5, H.3, H.7, J
17.04.00297.3	26.03.2021	New certificates added	
21.03.00226.1	26.03.2021	Ascribed to Organic Concept	



## A. Introduction

Organic Concept a Belgian company that rents and sells tent structures made out of a stretchable membrane, so called Stretch Tents or Bedouin tents. This enables a freedom of form as there is not a pre-described shape necessary. Depending on the location, the number of poles, length of the poles, placement of the poles, number and type of tie-downs can be varied. Resulting in a custom made cover at each new location.

This freedom of form is enabled by the stretch fabric, as the desired form is stretched in shape. The drawback of this flexibility in shape is the difficulty to investigate all the different possibilities and to put them into a kind of order in a static analysis.

This report only shows the static analysis of a 20m x 15m tent. A variant with closed side walls is investigated and a variant with open side walls. These two variants are characteristic / leading shapes for this tent structure. The total size can also be achieved by connecting multiple fabric panels.

This document contains the data required for a tent book, according to EN 13782, for the 20x15m stretch tents of Organic Concept, including:

- Ownership data;
- Drawings of the different variants of the tent, including dimensions, indications of elements and required anchoring;
- Permitted live load;
- Maximum wind speeds (according to EN 1991-1-4:2005);
- Structural analysis (according to EN 13782:2015);
- Material certificates (strength properties and fire properties).

Utrecht, 26.03.2021,

ir. Ruud van Bommel

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### C. Codes and standards

- EN 1990 Eurocode, Basis of structural Design
- EN 1991 Eurocode 1, Part 1-4: General actions - wind actions.
- EN 1993 Eurocode 3, Design of steel structures
- EN 1995 Eurocode 5, Design of wooden structures
- EN 1999 Eurocode 9, Design of aluminum structures
- EN 13782 Temporary Structures – Tents - Safety
- EN 10204 Products of steel –inspections documents
- EN 12195-2 Belts
- ISO 1141 Synthetic fiber ropes Polyester
- ISO 1346 Synthetic fiber ropes Polypropylene
- ISO 1969 Synthetic fiber ropes Polyethylene

## D. Summary

Owner:	Organic Concept  Molenveldstraat 18 B 26 30 AARTSELAAR België +322709 0 950
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### General information:

Main dimensions		<b>20x15m</b>	
	Width:	15m	
	Length:	20m	
	Side height membrane:	3.0m	
	Max. height:	5.5m	
		<b>Wood</b> <b>[Eucalyptus, D35]</b>	<b>Aluminium</b> <b>[6063 T6]</b>
	<u>Floating configuration:</u>		
	Center pole – 5.5m	Ø 120mm	Lower to 5m, Ø90x3/Ø76x5mm
	Center pole – 5.0m	Ø 110mm	Ø90x3mm/Ø76x4mm
	Entrance pole – 3.0m	Ø 85mm	Ø76x3mm
	Corner pole – 2.5m	Ø 70mm	Ø50x3mm
	<u>Closed configuration:</u>		
	Center pole – 5.5m	Ø 120mm	Ø90x3mm/Ø76x4mm
	Center pole – 5.0m	Ø 110mm	Ø90x3mm/Ø76x4mm
	Center pole – 4.0m	Ø 85mm	Ø76x3mm
	Entrance pole – 3.0m	Ø 85mm	Ø76x3mm
	Corner poles – 2.5m	Ø 70mm	Ø50x3mm
	Side wall poles – 2.5m	Ø 70mm	Ø50x3mm
	Stormbelt:	Belt: PES, Min. Breaking load: 5000 kg	
	Guy ropes:	Rope: synthetic, Min. Breaking load: 5870 kg * OR Belt: PES, Min. Breaking load: 3000 kg	
	Circumference rope:	Rope: synthetic, Min. Breaking load: 4500 kg * OR Belt: PES, Min. Breaking load: 2750 kg	
	Connections elements at guy rope:	Min. breaking load: 3000 kg	

\* The required breaking load ( $BL_{tot}$ ) may be achieved by using multiple rope sections (n):  $BL_{rope} = BL_{tot} / n$

**Loads:**

User load:	max. additional load of 10 kg per column is allowed, if the load is applied centric.
Snow load:	0.1 kN/m <sup>2</sup> (equal to 4cm of snow) according to the French CTS.
Wind load:	<p>The calculation is based on a wind pressure of <math>p_w = 500 \text{ N/m}^2</math>, according to EN 13782 par. 7.4.2.2. This value corresponds to a peak value of the wind speed <math>v = 31.1 \text{ m/s}</math> (<math>\pm 113 \text{ km/h}</math>) at 10m height. The wind pressure is recalculated to the corresponding wind speeds<sup>(1)</sup> for Europe (not country specific), shown in the table below.</p> <p><u>Storm belts</u> are necessary from a certain reduced wind pressure, as stated in paragraph H.5.5.1. The corresponding wind speeds are stated on the drawings.</p>

**Wind speeds for default European terrain categories (not Country specific)**

	Out of service <sup>(1)</sup>				
	Coast	Flattened, open area	Rural area	Village	City
10 min. average wind speed <sup>(2)</sup>	> 17.53 m/s > 63.11 km/h	> 18.39 m/s > 66.20 km/h	> 20.36 m/s > 73.30 km/h	> 24.99 m/s > 89.96 km/h	> 26.08 m/s > 93.89 km/h
Beaufort <sup>(3)</sup>	> 7 BFT	> 7 BFT	> 7 BFT	> 9 BFT	> 9 BFT
Peak wind speed <sup>(4)</sup>	> 113 km/h	> 113 km/h	> 113 km/h	> 113 km/h	> 113 km/h

(1) 'Out of service' means: above the given wind speed the structure is no longer guaranteed regarding strength and/or stability.

(2) 10min average wind speed at 10m height measured at the nearest weather stations.

(3) wind data in Beaufort (BFT) are indicative values.

(4) 3 second peak wind speed measured on site at 10m height.

**Safety against sliding, overturning and uplifting:**

Anchor forces:

Assumptions: angle of 45 degrees

The following design resistance of the anchor forces\* is required :  
**“Floating” tent**

	Full wind		Reduced wind Up to and until 7 BFT	
Guy ropes - short side	11.75	kN	7.73	kN
Guy ropes - long side	5.98	kN	4.18	kN
Guy ropes - corner	19.19	kN	13.52	kN
Guy ropes - valley	7.76	kN	12.70	kN
Storm belts	17.98	kN		

**“Closed” tent**

	Full wind		Reduced wind Up to and until 7 BFT	
Guy ropes	7.24	kN	11.60	kN
Ground point – front corner	14.53	kN	12.73	kN
Ground point – back corner	6.57	kN	4.25	kN
Ground point - short side	5.81	kN	6.72	kN
Ground points - long side	4.15	kN	3.31	kN
Storm belts - width direction	14.69	kN		
Storm belts - length direction	17.35	kN		

\*See H.8.3 for Anchor tests according to BS-EN 13782

Required anchor stakes:

Based on dense, non-cohesive soil (e.g. sandy soils).

In case anchors **Ø35mm x 1200mm** (effective length) are being used:  
**“Floating” tent**

	Full wind	Reduced wind Up to and until 7 BFT
Guy ropes - short side	2x	2x
Guy ropes - long side	1x	1x
Guy ropes – corner ( 2 guy ropes)	2x	1x
Guy ropes - valley	2x	2x
Storm belts	3x	-

**“Closed” tent**

	Full wind	Reduced wind Up to and until 7 BFT
Guy ropes	1x	2x
Ground point – front corner	2x	2x
Ground point – back corner	1x	1x
Ground point - short side	1x	1x
Ground points - long side	1x	1x
Storm belts - width direction	2x	-
Storm belts - length direction	3x	-

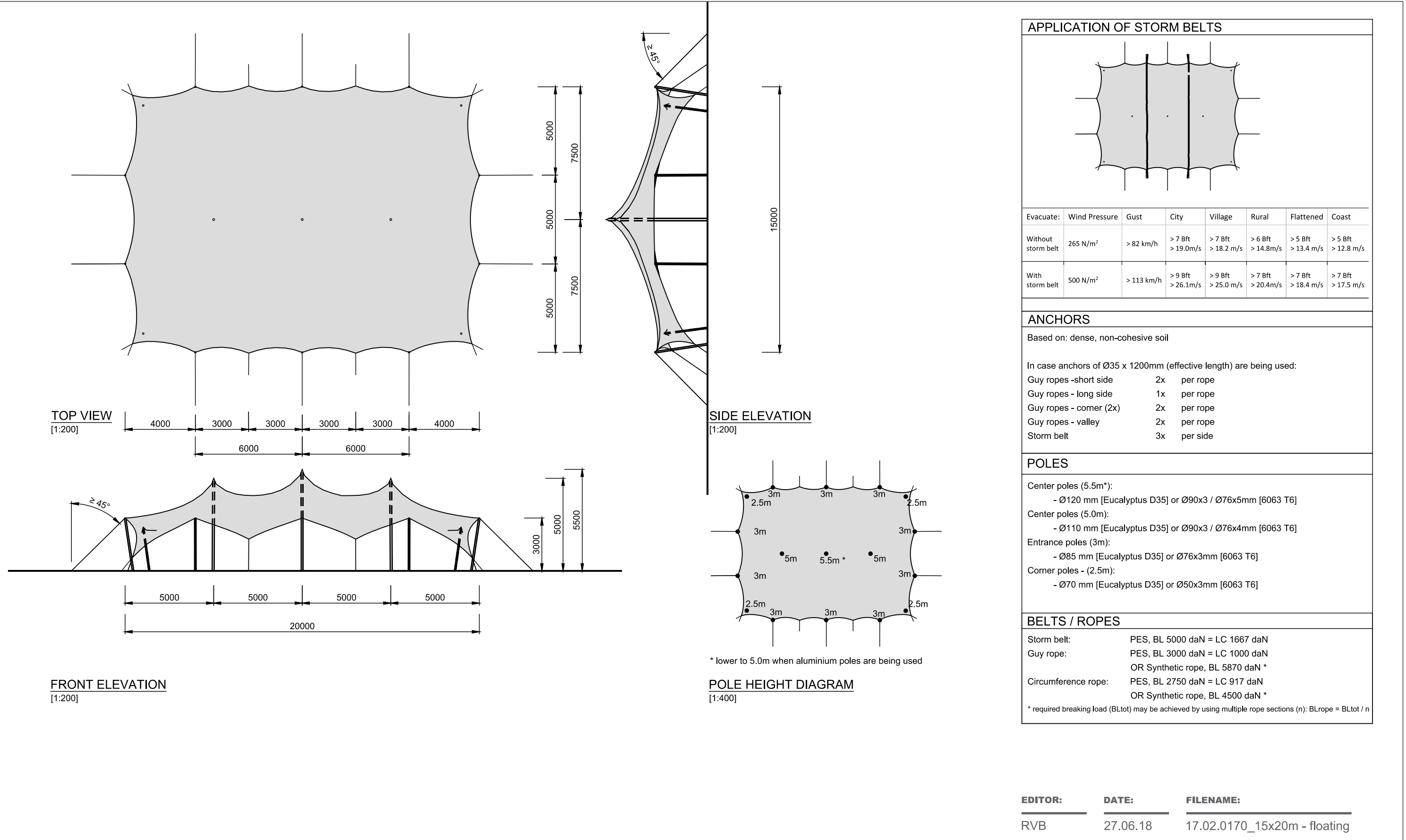
## E. Drawings: main dimensions and anchoring

The following pages contain the drawings for:

Analysed sizes:

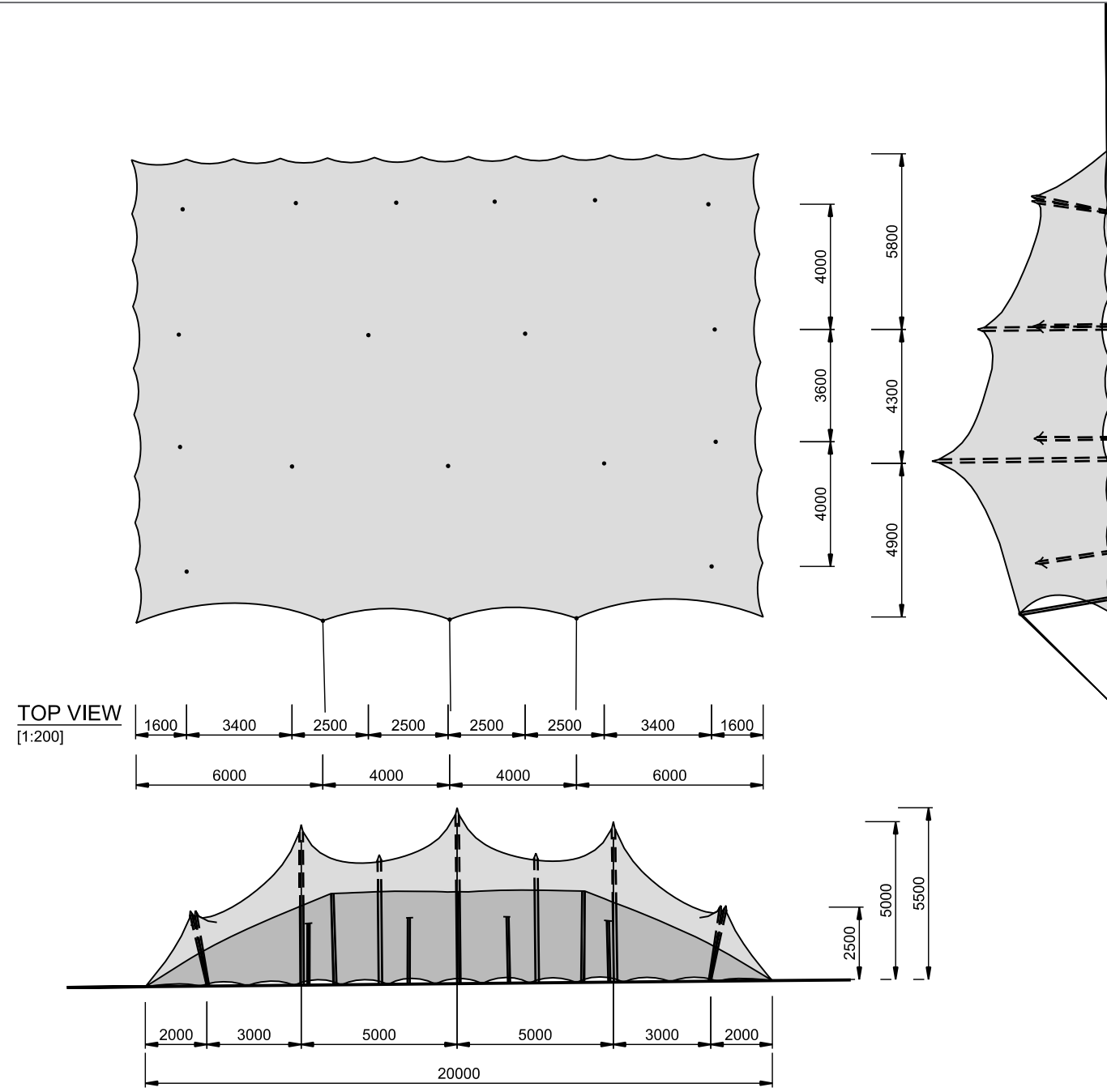
- 20x15m - floating
- 20x15m – closed

### E.1. 20x15m – Floating





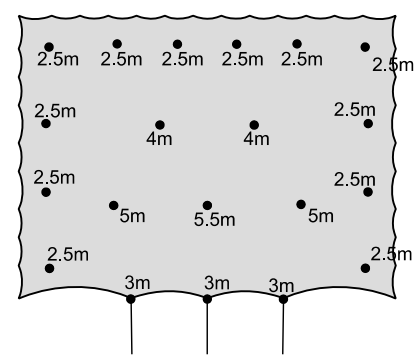
## E.2. 20x15m – Closed



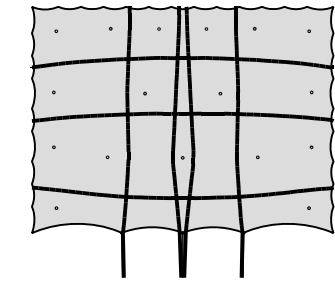
**FRONT ELEVATION**  
[1:200]

**SIDE ELEVATION**  
[1:200]

**POLE HEIGHT DIAGRAM**  
[1:400]



### APPLICATION OF STORM BELTS



Evacuate:	Wind Pressure	Gust	City	Village	Rural	Flattened	Coast
Without storm belt	265 N/m <sup>2</sup>	> 82 km/h	> 7 Bft > 19.0m/s	> 7 Bft > 18.2 m/s	> 6 Bft > 14.8m/s	> 5 Bft > 13.4 m/s	> 5 Bft > 12.8 m/s
With storm belt	500 N/m <sup>2</sup>	> 113 km/h	> 9 Bft > 26.1m/s	> 9 Bft > 25.0 m/s	> 7 Bft > 20.4m/s	> 7 Bft > 18.4 m/s	> 7 Bft > 17.5 m/s

### ANCHORS

Based on: dense, non-cohesive soil

In case anchors of Ø35 x 1200mm (effective length) are being used:

Guy ropes	2x	per rope
Ground point - front corner	2x	per rope
Ground point - back corner	1x	per rope
Ground point - short side	1x	per rope
Ground point - long side	1x	per rope
Storm belt - width (4x)	2x	per side
Storm belt - length (3x)	3x	per side

### POLES

- Center poles (5.5m):  
- Ø120 mm [Eucalyptus D35] or Ø90x3 / Ø76x4mm [6063 T6]
- Center poles (5.0m):  
- Ø110 mm [Eucalyptus D35] or Ø90x3 / Ø76x4mm [6063 T6]
- Center poles (4m) / Entrance poles (3m):  
- Ø85 mm [Eucalyptus D35] or Ø76x3mm [6063 T6]
- Corner poles - (2.5m):  
- Ø70 mm [Eucalyptus D35] or Ø50x3mm [6063 T6]
- Side wall poles - (2.5m):  
- Ø70 mm [Eucalyptus D35] or Ø50x3mm [6063 T6]

### BELTS / ROPES

Storm belt:	PES, BL 5000 daN = LC 1667 daN
Guy rope:	PES, BL 3000 daN = LC 1000 daN OR Synthetic rope, BL 5870 daN *
Circumference rope:	PES, BL 2750 daN = LC 917 daN OR Synthetic rope, BL 4500 daN *

\* required breaking load (BLtot) may be achieved by using multiple rope sections (n): BL<sub>rope</sub> = BL<sub>tot</sub> / n

EDITOR:	DATE:	FILENAME:
RVB	27.06.18	17.02.0170_15x20m - closed

## F. Important terms and conditions

This document applies to the built structure if the following principles and conditions are met:

- The used materials, parts and sections (membrane, poles, ties, anchoring) are in accordance with this document.
- The dimensions of the built structure match the dimensions stated in this document.
- Parts (poles, ties, anchors) may not be removed.
- Obstacles should be placed at least 0.5m from the membrane (measured perpendicular to the fabric); The fabric needs a certain freedom to deform in all directions to prevent damages caused by collision with objects located closely to the fabric (see also NEN-EN 13782, article 8.7).
- Above the maximum allowable wind speeds (see summary, part wind load) the structure should be evacuated and access for the public must be denied.
- Only decorations, music- and light installations of less than 10 kg per pole, can be attached to the structure.
- A conventional load of 0.1 kN/m<sup>2</sup> is taken into account according to EN 13782, which corresponds with the required snowload (4cm) according to the French CTS.
- Anchoring is based on dense, non-cohesive soil. When soil differs, additional anchoring might be necessary or anchor tests need to be performed.

## G. Allowable wind speeds

The maximum wind speed is converted into a basic wind speed for a coastal area, flattened/open area, rural area, village and city according to EN 1991-1-4. Terrain roughness is taken according to the recommended general values for the different terrain categories for Europe. (not country specific) Illustrations of these terrain categories are presented at page 18.

Storm belts are needed from a certain reduced wind pressure, as stated in paragraph H.5.5.1.

	Full wind load	$\alpha = 0.53$	
Wind pressure $p_w$	500 N/m <sup>2</sup>	265 N/m <sup>2</sup>	At <b>5m</b> height
Corresponding peak wind pressure $p_{w,peak}$	605 N/m <sup>2</sup>	321 N/m <sup>2</sup>	At <b>10m</b> height

### Peak wind speed at 10m height

Equation:

$$605 = \frac{1}{2} \times \rho \times v^2 = \frac{1}{2} \times 1.25 \times v^2 \rightarrow v = 31.1 \text{ m/s} \rightarrow \pm 113 \text{ km/h}$$

Eq. 4.10 NEN-EN 1991-1-4  
Basic wind pressure

$$321 = \frac{1}{2} \times \rho \times v^2 = \frac{1}{2} \times 1.25 \times v^2 \rightarrow v = 22.7 \text{ m/s} \rightarrow \pm 82 \text{ km/h}$$

Eq. 4.10 NEN-EN 1991-1-4  
Basic wind pressure

### Wind speed coastal area at 10m height

$$K_r = 0.19 \times \left(\frac{z_0}{0.05}\right)^{0.07} = 0.19 \times \left(\frac{0.003}{0.05}\right)^{0.07} = 0.156$$

Eq. 4.5 NEN-EN 1991-1-4  
Terrain factor for coastal area

$$C_r = K_r \times \ln\left(\frac{z}{z_0}\right) = 0.156 \times \ln\left(\frac{5.0}{0.003}\right) = 1.158$$

Eq. 4.4 NEN-EN 1991-1-4  
Roughness factor at 4m height  
 $Z = 5.0 > Z_{min} = 1$

$$V_m = C_r \times V_b = 1.158 \times V_b$$

Eq. 4.3 NEN-EN 1991-1-4  
Average wind speed at height

$$\sigma_v = K_r \times V_b = 0.156 \times V_b$$

Eq. 4.6 NEN-EN 1991-1-4  
Standard deviation of turbulence

$$L_v = \frac{\sigma_v}{V_m} = \frac{0.156 \times V_b}{1.158 \times V_b} = 0.135$$

Eq. 4.7 NEN-EN 1991-1-4  
Turbulence intensity

$$Q_p = (1 + 7 \times L_v) \times \frac{1}{2} \times \rho \times V_m^2 = 1.628 \times V_b^2$$

Eq. 4.8 NEN-EN 1991-1-4  
Extreme wind pressure

Equation:

$$500 = 1.628 \times V_b^2 \rightarrow \text{solving gives} \rightarrow V_b = 17.53 \text{ m/s}$$

Characteristic wind speed

$$265 = 1.628 \times V_b^2 \rightarrow \text{solving gives} \rightarrow V_b = 12.76 \text{ m/s}$$

Characteristic wind speed

### Wind speed flattened, open area at 10m height

$$K_r = 0.19 \times \left(\frac{z_0}{0.05}\right)^{0.07} = 0.19 \times \left(\frac{0.01}{0.05}\right)^{0.07} = 0.170$$

$$C_r = K_r \times \ln\left(\frac{z}{z_0}\right) = 0.170 \times \ln\left(\frac{5.0}{0.01}\right) = 1.055$$

$$V_m = C_r \times V_b = 1.055 \times V_b$$

$$\sigma_v = K_r \times V_b = 0.170 \times V_b$$

$$L_v = \frac{\sigma_v}{V_m} = \frac{0.170 \times V_b}{1.055 \times V_b} = 0.161$$

$$Q_p = (1 + 7 \times L_v) \times \frac{1}{2} \times \rho \times V_m^2 = 1.479 \times V_b^2$$

Equation:

$$500 = 1.479 \times V_b^2 \rightarrow \text{solving gives} \rightarrow V_b = 18.39 \text{ m/s}$$

$$265 = 1.479 \times V_b^2 \rightarrow \text{solving gives} \rightarrow V_b = 13.39 \text{ m/s}$$

Eq. 4.5 NEN-EN 1991-1-4  
Terrain factor for coastal area

Eq. 4.4 NEN-EN 1991-1-4  
Roughness factor at 3.5m height  
 $Z = 5.0 > Z_{\min} = 1$

Eq. 4.3 NEN-EN 1991-1-4  
Average wind speed at height

Eq. 4.6 NEN-EN 1991-1-4  
Standard deviation of turbulence

Eq. 4.7 NEN-EN 1991-1-4  
Turbulence intensity

Eq. 4.8 NEN-EN 1991-1-4  
Extreme wind pressure

Characteristic wind speed

Characteristic wind speed

### Wind speed rural area at 10m height

$$K_r = 0.19 \times \left(\frac{z_0}{0.05}\right)^{0.07} = 0.19 \times \left(\frac{0.05}{0.05}\right)^{0.07} = 0.190$$

$$C_r = K_r \times \ln\left(\frac{z}{z_0}\right) = 0.190 \times \ln\left(\frac{5.0}{0.05}\right) = 0.875$$

$$V_m = C_r \times V_b = 0.875 \times V_b$$

$$\sigma_v = K_r \times V_b = 0.190 \times V_b$$

$$L_v = \frac{\sigma_v}{V_m} = \frac{0.190 \times V_b}{0.875 \times V_b} = 0.217$$

$$Q_p = (1 + 7 \times L_v) \times \frac{1}{2} \times \rho \times V_m^2 = 1.206 \times V_b^2$$

Equation:

$$500 = 1.206 \times V_b^2 \rightarrow \text{solving gives} \rightarrow V_b = 20.36 \text{ m/s}$$

$$265 = 1.206 \times V_b^2 \rightarrow \text{solving gives} \rightarrow V_b = 14.82 \text{ m/s}$$

Eq. 4.5 NEN-EN 1991-1-4  
Terrain factor for unbuilt area

Eq. 4.4 NEN-EN 1991-1-4  
Roughness factor at 4m height  
 $Z = 5.0 > Z_{\min} = 2$

Eq. 4.3 NEN-EN 1991-1-4  
Average wind speed at height

Eq. 4.6 NEN-EN 1991-1-4  
Standard deviation of turbulence

Eq. 4.7 NEN-EN 1991-1-4  
Turbulence intensity

Eq. 4.8 NEN-EN 1991-1-4  
Extreme wind pressure

Characteristic wind speed

Characteristic wind speed

### Wind speed village at 10m height

$$K_r = 0.19 \times \left(\frac{z_0}{0.05}\right)^{0.07} = 0.19 \times \left(\frac{0.3}{0.05}\right)^{0.07} = 0.215$$

$$C_r = K_r \times \ln\left(\frac{z}{z_0}\right) = 0.215 \times \ln\left(\frac{5}{0.3}\right) = 0.606$$

$$V_m = C_r \times V_b = 0.606 \times V_b$$

$$\sigma_v = K_r \times V_b = 0.215 \times V_b$$

$$L_v = \frac{\sigma_v}{V_m} = \frac{0.215 \times V_b}{0.606 \times V_b} = 0.355$$

$$Q_p = (1 + 7 \times L_v) \times \frac{1}{2} \times \rho \times V_m^2 = 0.801 \times V_b^2$$

Equation:

$$500 = 0.801 \times V_b^2 \rightarrow \text{solving gives } \rightarrow V_b = 24.99 \text{ m/s}$$

$$265 = 0.801 \times V_b^2 \rightarrow \text{solving gives } \rightarrow V_b = 18.19 \text{ m/s}$$

Eq. 4.5 NEN-EN 1991-1-4  
Terrain factor for unbuilt area

Eq. 4.4 NEN-EN 1991-1-4  
Roughness factor at 7m height  
 $Z = Z_{\min} = 5$

Eq. 4.3 NEN-EN 1991-1-4  
Average wind speed at height

Eq. 4.6 NEN-EN 1991-1-4  
Standard deviation of turbulence

Eq. 4.7 NEN-EN 1991-1-4  
Turbulence intensity

Eq. 4.8 NEN-EN 1991-1-4  
Extreme wind pressure

Characteristic wind speed

Characteristic wind speed

### Wind speed city at 10m height

$$K_r = 0.19 \times \left(\frac{z_0}{0.05}\right)^{0.07} = 0.19 \times \left(\frac{1}{0.05}\right)^{0.07} = 0.234$$

$$C_r = K_r \times \ln\left(\frac{z}{z_0}\right) = 0.234 \times \ln\left(\frac{10}{1}\right) = 0.540$$

$$V_m = C_r \times V_b = 0.540 \times V_b$$

$$\sigma_v = K_r \times V_b = 0.234 \times V_b$$

$$L_v = \frac{\sigma_v}{V_m} = \frac{0.234 \times V_b}{0.540 \times V_b} = 0.434$$

$$Q_p = (1 + 7 \times L_v) \times \frac{1}{2} \times \rho \times V_m^2 = 0.735 \times V_b^2$$

Equation:

$$500 = 0.735 \times V_b^2 \rightarrow \text{solving gives } \rightarrow V_b = 26.08 \text{ m/s}$$

$$265 = 0.735 \times V_b^2 \rightarrow \text{solving gives } \rightarrow V_b = 18.99 \text{ m/s}$$

Eq. 4.5 NEN-EN 1991-1-4  
Terrain factor for unbuilt area

Eq. 4.4 NEN-EN 1991-1-4  
Roughness factor at 7m height  
 $Z = Z_{\min} = 10$

Eq. 4.3 NEN-EN 1991-1-4  
Average wind speed at height

Eq. 4.6 NEN-EN 1991-1-4  
Standard deviation of turbulence

Eq. 4.7 NEN-EN 1991-1-4  
Turbulence intensity

Eq. 4.8 NEN-EN 1991-1-4  
Extreme wind pressure

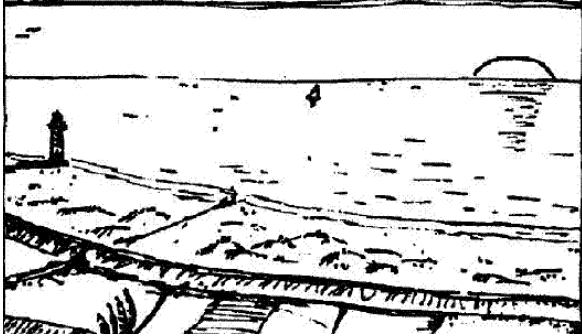
Characteristic wind speed

Characteristic wind speed

## Terrain categories

Default European terrain categories (not Country specific)

0: Coastal area:



I: Flattened, open area:



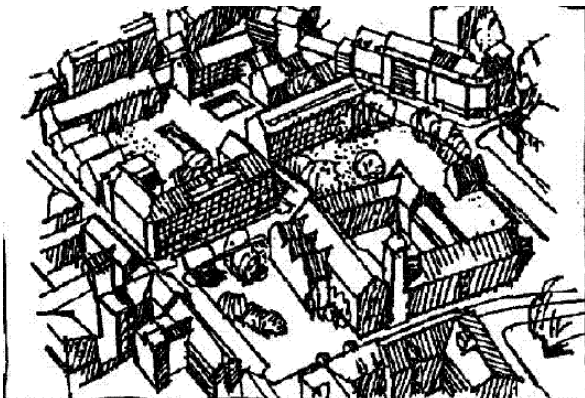
II: Rural area



III: Village



IV: City



## H. Static Analysis

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## H.1. Project description

### H.1.1 Description

The principle of a stretchtent is based on a rectangular piece of stretchable membrane with fixing possibilities along the boundary. The membrane is supported by poles, both at the edge and in the field. The side poles are stabilized by guy ropes. Besides, it is also possible to tie down the edge of the membrane directly to the ground. The poles do not require a fixed position which ensures a freedom of shape.

Tentech has performed a static analysis of the 20x15m stretch tent, for which two configurations have been considered in detail:

- The 20x15m 'floating model'  
Elevated from the ground by center poles and entrance poles. The corner poles are positioned slightly inward and the corners of the fabric are tied down to the ground with a short rope. On the long sides the fabric is tied to the ground with guy ropes at the entrance poles and guy ropes between the entrance poles to create more curvature.
- The 20x15m 'three sides closed' model  
Opened on one side with entrance poles. The other three membrane edges are directly connected to the ground each  $\approx 1.5\text{m}$ . The 'walls' are created by placing side wall poles in the fabric, positioned ca. 2m from the edge. The rest of the membrane is supported by center poles.

Storm belts are only needed from a certain wind load. These conditions are specified in paragraph H.5.5.1.

The total size can be achieved by joining multiple membrane parts. The connection between two membrane parts can be verified on the assumption that the connection itself is stronger than a single membrane layer. Therefore, it is possible to construct a tent size that has been analyzed as one part also with multiple parts, as long as the arrangement stays the same.



## H.1.2 Analyzed configurations

### H.1.2.1 20x15m – Floating

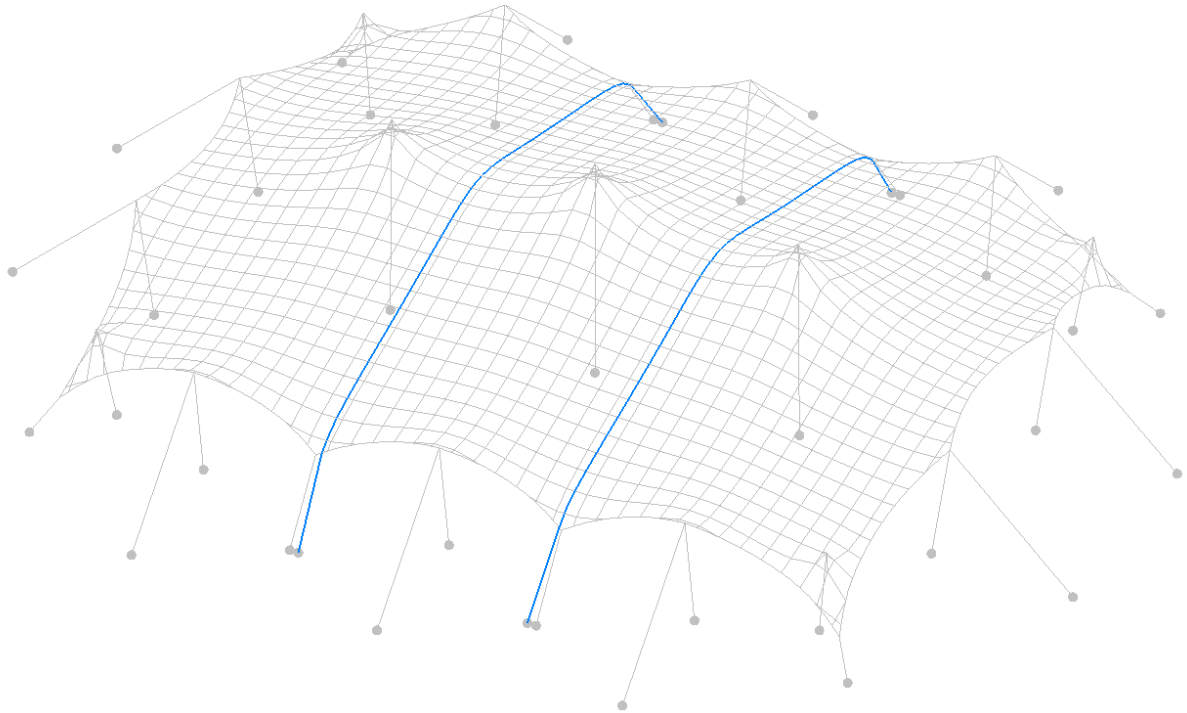


Figure 1. 20x15m floating

### H.1.2.2 20x15m – three sides closed

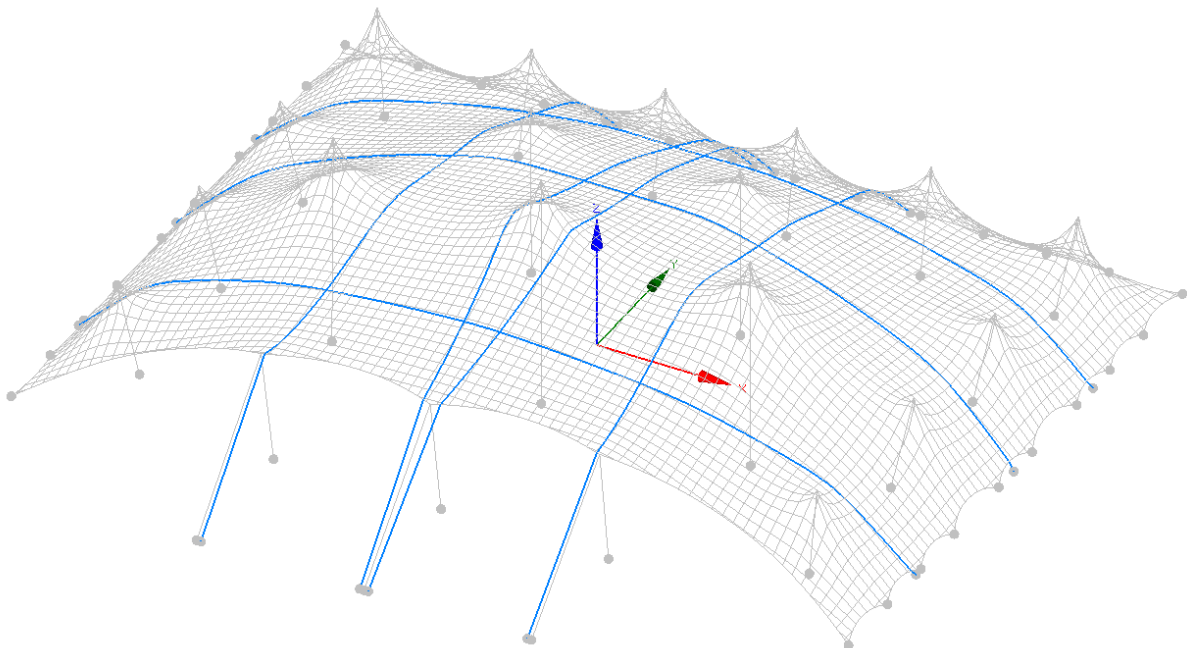


Figure 2. 20x15m closed

## H.2. Materials

### H.2.1 Fabric

Design tensile strength	$f_d$	$f_{tk} / \gamma_m$	art 8.6EN 13782
Characteristic tensile strength (warp)	$f_{tk, \text{warp}}$		
Characteristic tensile strength (weft)	$f_{tk, \text{weft}}$		
Material factor – global, permanent load	$\gamma_m$	2.5	tbl 4. EN13782
Material factor – global, short duration load	$\gamma_m$	2.0	tbl 4. EN13782

**Table 1. Used symbols, codes and standard for fabric materials**

Material	Type	Weight	$f_{rd; \text{warp}; \text{perm}}$	$f_{rd; \text{weft}; \text{perm}}$	$f_{rd; \text{warp}; \text{short}}$	$f_{rd; \text{weft}; \text{short}}$
Contour 3xl FR	-	≈ 500 gr/m <sup>2</sup>	8.0 kN/m	5.6 kN/m	10.0 kN/m	7.0 kN/m
Contour X FR	-	≈ 750 gr/m <sup>2</sup>	7.6 kN/m	6 kN/m	9.5 kN/m	7.5 kN/m

**Table 2. Used fabrics**

The calculations provided in this tentbook are based on the lowest tensile strength of the two fabrics.

### H.2.2 Belts

Design resistance	$F_{rd}$	$R_m / \gamma_{m1}$	art 10.2. EN13782
Characteristic breaking strength	$R_m$	$LC \times \gamma_{m2}$	art 10.2. EN13782
Lashing capacity	LC		Conform EN 12195-2
Material factor	$\gamma_{m1}$	2.0	art 10.2. EN13782
Material factor	$\gamma_{m2}$	3.0	EN1492-1

**Table 3. Used symbols, codes and standard for belt materials**

Material	LC	$R_m$	$F_{rd}$
Storm belt [PES] EN 12195-2	1667 daN 16.67 kN	5000 daN 50 kN	25 kN

**Table 4. Used belts**

### H.2.3 Ropes

Design resistance	$F_{rd}$	$R_m / \gamma_{m1}$	art 10.2. EN13782
Characteristic tensile strength	$R_m$		art 10.2. EN13782
Material factor	$\gamma_{m1} < 12\text{mm}$	4.0	art 10.3. EN13782
	$\gamma_{m1} > 12\text{mm}$	3.3	art 10.3. EN13782

Table 5. Used symbols, codes and standard for belt materials

Material	Cross section	Breaking strength	$R_m$	$F_{rd}$
14mm Polyester Braid rope	$\varnothing$ 14 mm	$\geq 3640$ daN	36.4 kN	11.0 kN
7mm Polyester Braid rope	$\varnothing$ 7 mm	$\geq 910$ daN	9.1 kN	2.3 kN

Table 6. Used ropes

### H.2.4 Wood, Eucalyptus D35

Material	$\gamma_{m1}$	1.3	tbl. 2.3. EN 1995-1-1
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Table 7. Used material factors

Material	Weight	$E_{0.05}$	$F_{c0k}$	$F_{mk}$
Wood, Eucalyptus D35 strength class	540 kg/m <sup>3</sup>	8.7 kN/m <sup>2</sup>	25 N/mm <sup>2</sup>	35 N/mm <sup>2</sup>

Table 8. Used wooden materials

### H.2.5 Aluminium, 6063 T6

Material factor (strength)	$\gamma_{m0}$	1.1	tbl. 6.1. EN 1999-1-1
Material factor (stability)	$\gamma_{m1}$	1.1	tbl. 6.1. EN 1999-1-1
Material factor (tension to fracture / connections)	$\gamma_{m2}$	1.25	tbl. 2.1. EN 1999-1-1

Table 9. Used material factors

Material	Weight	E-modulus	$F_y$	$F_u$
EN-AW 6063 T6	2700 kg/m <sup>3</sup>	70000 N/mm <sup>2</sup>	160 N/mm <sup>2</sup>	195 N/mm <sup>2</sup>

Table 10. Used aluminum materials

### H.3. Cross sections

Profile	Material	Ø mm	t mm	G kg/m	A mm <sup>2</sup>	I <sub>y</sub> mm <sup>4</sup>	W <sub>el,y</sub> mm <sup>3</sup>	W <sub>pl,y</sub> mm <sup>3</sup>
Ø ≈ 120 mm *	Wood D35	120	n/a	6.11	11310	10178760	169646	
Ø ≈ 110 mm *	Wood D35	110	n/a	5.13	9503	7186884	130671	
Ø ≈ 85mm *	Wood D35	85	n/a	3.06	5675	2562392	60292	
Ø ≈ 70 mm *	Wood D35	70	n/a	2.08	3849	1178588	33674	
Ø ≈ 65 mm *	Wood D35	65	n/a	1.79	3318	876241	26961	
Ø90x3mm	6063 T6	90	3	2.21	820	776703	17260	22716
Ø76x5mm	6063 T6	76	5	3.01	1115	706242	18585	25247
Ø76x4mm	6063 T6	76	4	2.44	905	588106	15476	20757
Ø76x3mm	6063 T6	76	3	1.86	688	459074	12081	15996
Ø50x3mm	6063 T6	50	3	1.20	443	122812	4912	6636

**Table 11. Used cross sections**

\* the average diameter, as a minimum required at the middle of the pole.

## H.4. Calculation method

### H.4.1 Modeling

The analysis of the structure is performed with the software package EASY FCS supplied by TECHNET GmbH, Berlin. This software is specially developed for structures with large deformability, such as membrane structures. The performed analysis is a full non-linear second order analysis.

The membrane structure is modeled in 3D. The membrane is modeled as a cable net structure and supported by poles. These center poles will be stabilized by the tensioned membrane. The side poles are stabilized and tied down by tension belts, which are attached to ground anchors. Alternatively, the membrane is directly tied to a ground anchor.

The membrane edges are reinforced. They consist of multiple layers of fabric about 10cm wide with additional patches where the lugs are placed, so point loads are introduced gradually into the fabric.

When required, stormbelts are placed in the valleys between the field poles.

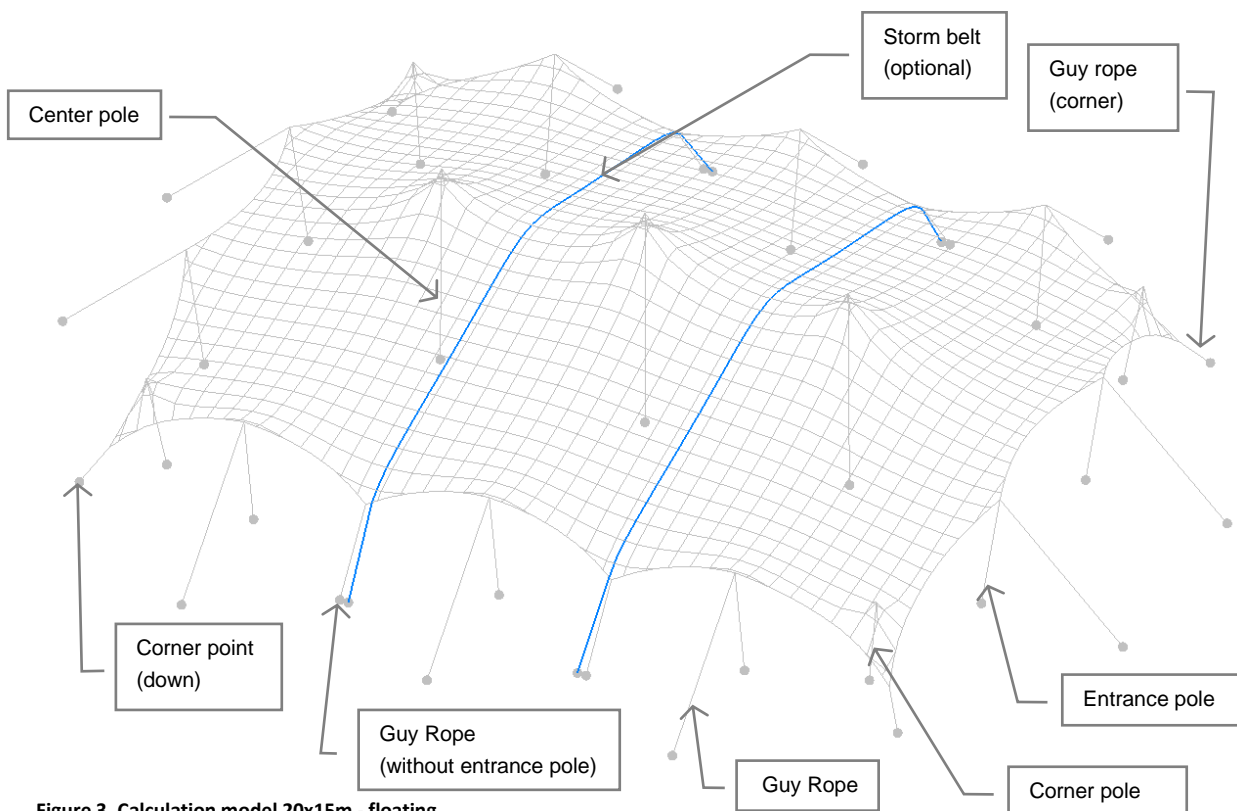


Figure 3. Calculation model 20x15m - floating

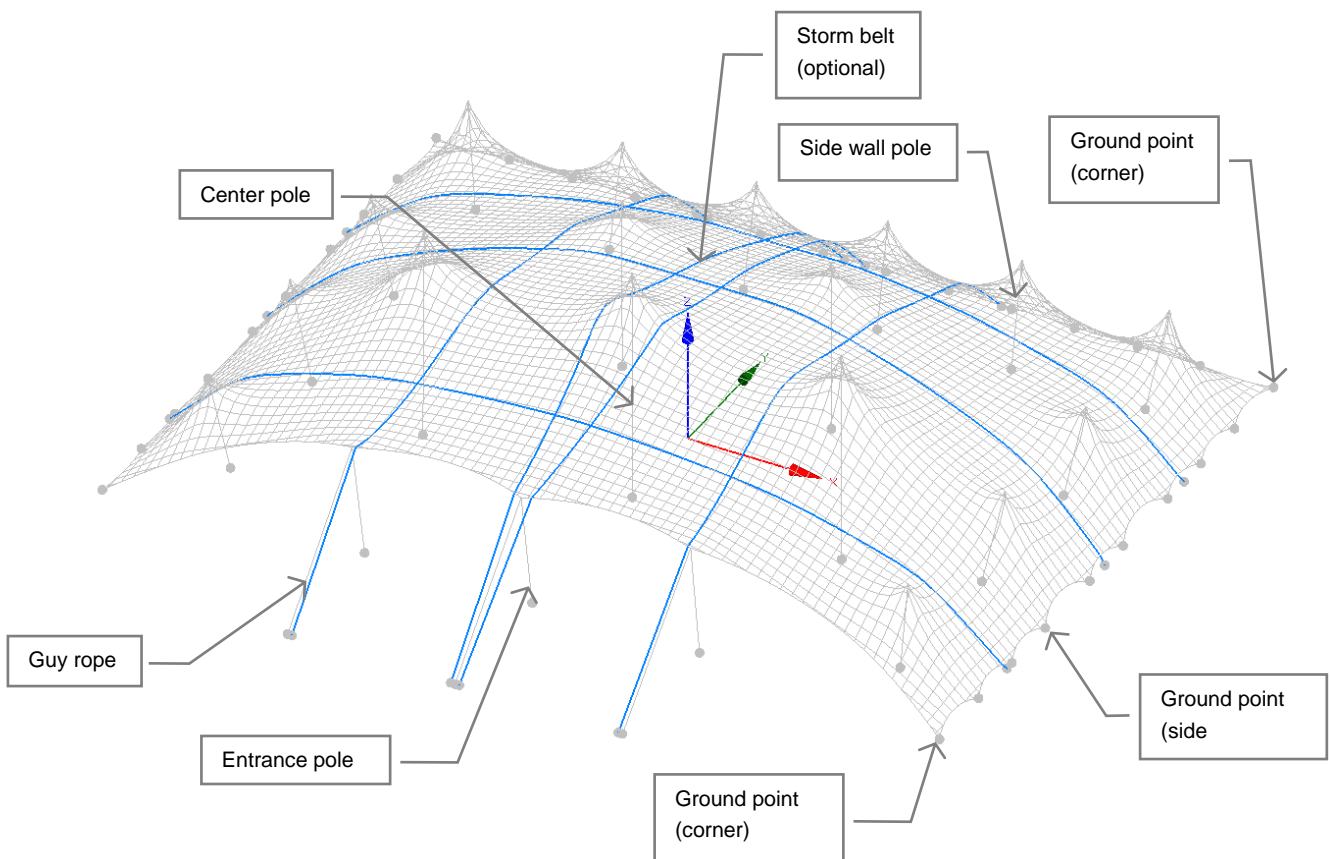


Figure 4. Calculation model 20x15m - closed

#### H.4.2 Structural behavior of membrane structures

Since the fabric is a highly deformable material, it is only possible to calculate stresses and deformations with a non-linear method. FEM-software EASY is used to perform these calculations. Because of the non-linearity of the calculations the partial safety factors are not applied beforehand, since the deformations will be greater due to these safety factors, resulting in lower stresses in the fabric. See figure below.

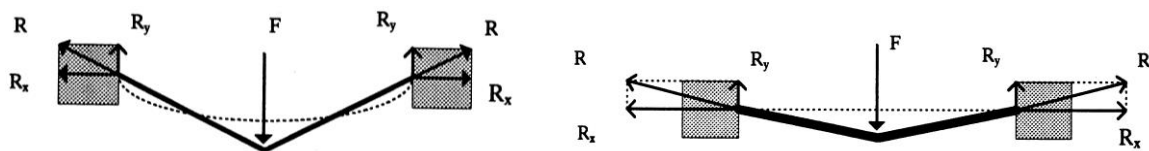


Figure 5. membrane behavior

As a membrane structure is a form-active structure, article 6.3 (4) b) of the EN-1990:2002 applies:

*When the action effect increases less than the action, the partial factor  $\gamma_f$  should be applied to the action effect of the representative value of the action.*

This means that no load factors are applied on the load beforehand, but afterwards.

### H.4.3 Load combinations

#### H.4.3.1 Fundamental - Ultimate limit state

For the purpose of determination of strength and check of elements and connections.

	One variable load	Multiple variable loads
Unfavorable permanent load	$1.35 \times G + 1.5 \times Q$	$1.35 \times G + \sum 1.35 \times Q_i$
Favorable permanent load	$1.0 \times G + 1.5 \times Q$	$1.0 \times G + \sum 1.35 \times Q_i$

**Table 12. Load combinations according to NEN-EN 13782**

This means the following load combinations will be checked/calculated:

1. 1.0 x Own weight + 1.5 x Wind load
2. 1.35 x Own weight + 1.5 x Conventional load

#### H.4.3.2 Safety against overturning, sliding and uplifting - Ultimate limit state

For the purpose of determination and check of needed contra weight and/or anchor pins

	One or multiple variable loads
Unfavorable permanent load	$1.1 \times G + 1.2 \times Q_{wind} + \sum 1.3 \times Q_i$
Favorable permanent load	$1.0 \times G + 1.2 \times Q_{wind} + \sum 1.3 \times Q_i$

**Table 13. Load combinations according to NEN-EN 13782**

This means the following load combinations will be checked/calculated:

- A. 1.0 x Own weight + 1.2 x Wind load

## H.5. Load cases

### H.5.1 Own weight

The own weight of the fabric is  $750 \text{ g/m}^2 = 0.0075 \text{ kN/m}^2$  and is added as separate load case.

### H.5.2 Pretension

The structure will be pretensioned with ropes. The pretension varies between 150-200 kg of pressure in a center pole.

### H.5.3 Conventional / snow load

Conventional load according to EN 13782: The stability shall be checked with a conventional vertical load of  $0,1 \text{ kN/m}^2$ . This load shall not be combined with other load cases, except self-weight. This can be seen as a snow load of  $0.1 \text{ kN/m}^2$  (4cm) according the French CTS.

### H.5.4 User load

A user defined load (for light, sound and/or decoration purposes) is set on 10 kg per pole and is added after the analysis while performing checks.

### H.5.5 Wind

#### H.5.5.1 Wind pressure

Wind load according to EN 13782, 7.4.2.2:

This leads to a peak velocity pressure  $q_p(z_e)$  of  **$0.50 \text{ kN/m}^2$** .

In case of wind suction, storm belts are needed from a certain reduced wind pressure ( $p_{w,red}$ ):

Floating model – no stormbelts:	$p_{w,red} = \alpha \times p_w = 0.53 \times 500 = \mathbf{265 \text{ N/m}^2}$
Floating model – 2 stormbelts:	full wind load
Closed model – no stormbelts:	$p_{w,red} = \alpha \times p_w = 0.53 \times 500 = \mathbf{265 \text{ N/m}^2}$
Closed model – 7 stormbelts:	full wind load



### H.5.5.2 Wind shape values (C<sub>p</sub>-factors)

Two different wind situations are reviewed for the membrane:

1. The whole tent is subjected to wind suction (conform C<sub>p</sub> values given in EN 13782)
2. The whole tent is subjected to wind pressure (conform C<sub>p</sub> values given in EN 13782)

#### Wind suction - floating configuration

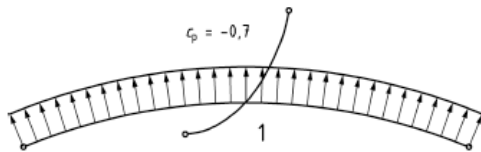


figure 6. C<sub>p</sub> values for rectangle structures BS-EN 13782

P <sub>w</sub>	p <sub>w,rep</sub>
500 N/m <sup>2</sup>	-0.7 x 0.500 = <b>-0.350 kN/m<sup>2</sup></b>
265 N/m <sup>2</sup>	-0.7 x 0.265 = <b>-0.186 kN/m<sup>2</sup></b>

#### Wind suction – three sides closed configuration (closed)

Wind coefficients (C<sub>p</sub> – values) for tent constructions according to EN 1991-1-4, for a free standing canopy with a blockage of φ = 1.0, C<sub>p</sub> = 1.3

P <sub>w</sub>	p <sub>w,rep</sub>
500 N/m <sup>2</sup>	-1.3 x 0.500 = <b>-0.650 kN/m<sup>2</sup></b>
265 N/m <sup>2</sup>	-1.3 x 0.265 = <b>-0.345 kN/m<sup>2</sup></b>

#### Wind pressure

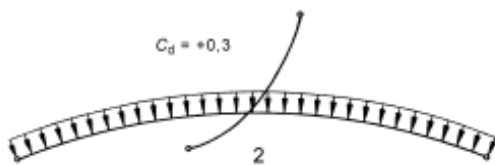


figure 7. C<sub>p</sub> values for rectangle structures BS-EN 13782

P <sub>w</sub>	p <sub>w,rep</sub>
500 N/m <sup>2</sup>	0.3 x 0.500 = <b>0.150 kN/m<sup>2</sup></b>
265 N/m <sup>2</sup>	0.3 x 0.265 = <b>0.080 kN/m<sup>2</sup></b>

## H.6. Calculation results

### H.6.1 Listing of calculated load combinations

LC1 = Pretension

LC2 = Own weight

LC3 = Conventional load / Snow load

LC4 = Wind pressure

LC5 = Wind suction – floating – reduction 0.53

LC6 = Wind suction – floating – full wind load

LC7 = Wind suction – closed – reduction 0.53

LC8 = Wind suction – closed – full wind load

The following load combinations are taken into account:

*partial safety factors are added after the static analysis (see H.4.2).*

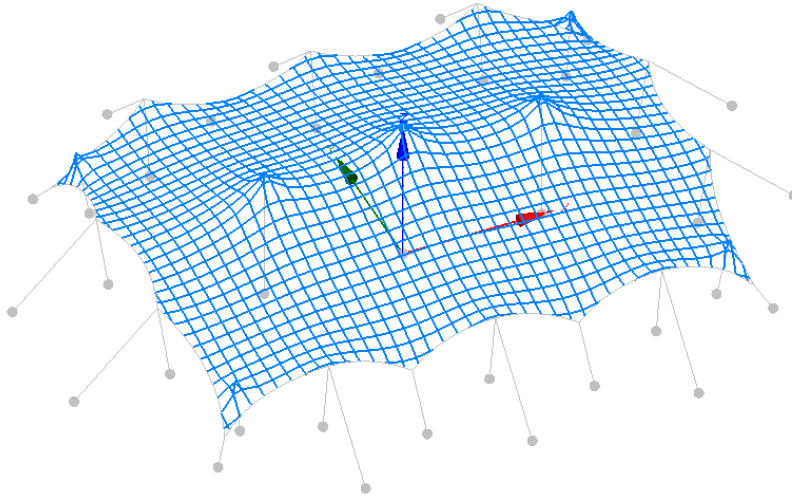
	LC 1	LC2	LC 3	LC 4	LC 5	LC6	LC7	LC8
CO 1	1 x	1 x						
CO 2	1 x	1 x	1 x					
CO 3	1 x	1 x		1 x				
CO 4	1 x	1 x			1 x			
CO 5	1 x	1 x				1 x		
CO 6	1 x	1 x					1x	
CO 7	1 x	1 x						1x

table 14. Combinations (CO)

Storm belts are only necessary above a certain wind speed. Only when required, storm belts are added to the calculation model. In H.6.2 Overview: Global results of static analysis, it is explicitly mentioned when a stormbelt is added.

## H.6.2 Overview: Global results of static analysis

### H.6.2.1 Membrane

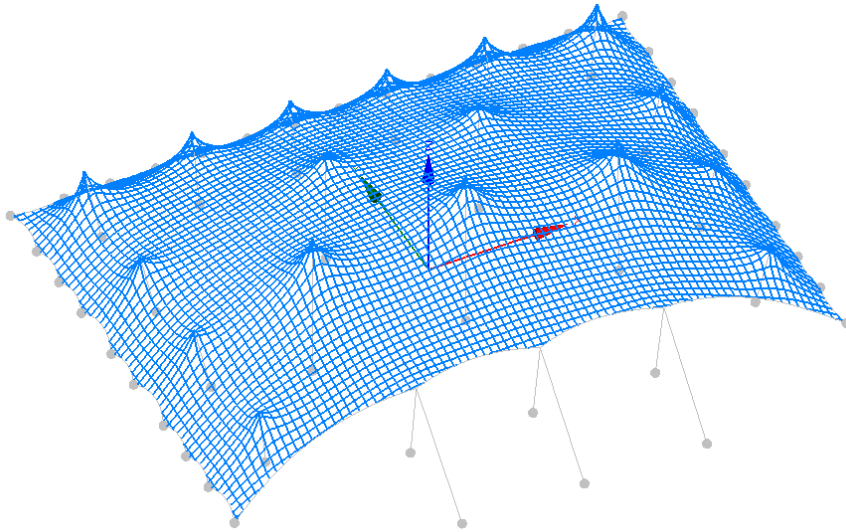


Floating model

Floating	Load combination	$F_{rep}$	Pag.
Warp	CO1. Own weight + pretension	0.59 kN/m	100
	CO2. Own weight + pretension + conventional / snow	2.64 kN/m	103
	CO3. Own weight + pretension + wind pressure	3.43 kN/m	106
	CO4. Own weight + pretension + wind suction – floating – reduction 0.53	3.00 kN/m	109
	<b>Max</b> CO5. Own weight + pretension + wind suction – floating – full wind load *	3.66 kN/m	112
Weft	CO1. Own weight + pretension	0.53 kN/m	100
	CO2. Own weight + pretension + conventional / snow	3.46 kN/m	103
	CO3. Own weight + pretension + wind pressure	4.76 kN/m	106
	<b>Max</b> CO4. Own weight + pretension + wind suction – floating – reduction 0.53	4.76 kN/m	109
	CO5. Own weight + pretension + wind suction – floating – full wind load *	4.18 kN/m	112

Table 15. Leading forces membrane – floating model

\* 2 stormbelts required. See Figure 1. 20x15m floating and/or drawing 20x15m – Floating in chapter E.1



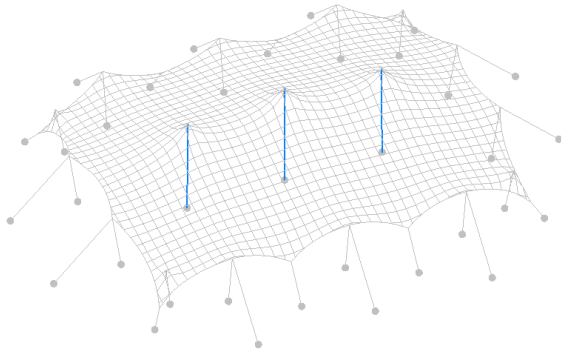
Closed model

Closed	Load combination	F <sub>rep</sub>	Pag.
Warp	CO1. Own weight + pretension	1.45 kN/m	115
	CO2. Own weight + pretension + conventional / snow	3.57 kN/m	118
	CO3. Own weight + pretension + wind pressure	5.06 kN/m	121
	CO6. Own weight + pretension + wind suction – closed – reduction 0.53	6.33 kN/m	124
	<b>Max</b> CO7. Own weight + pretension + wind suction – closed – full wind load *	6.33 kN/m	127
Weft	CO1. Own weight + pretension	1.16 kN/m	115
	CO2. Own weight + pretension + conventional / snow	3.59 kN/m	118
	<b>Max</b> CO3. Own weight + pretension + wind pressure	4.67 kN/m	121
	CO6. Own weight + pretension + wind suction – closed – reduction 0.53	4.67 kN/m	124
	CO7. Own weight + pretension + wind suction – closed – full wind load *	4.21 kN/m	127

Table 16. Leading forces membrane – closed model

\* 7 stormbelts required. See Figure 2. 20x15m closed and/or Drawing 20x15m – Closed in chapter E.2

### H.6.2.2 Center poles

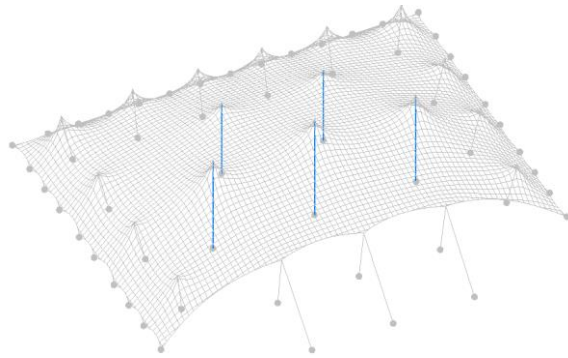


Floating model

Floating	Load combination	F <sub>rep</sub>	Pag.
Center pole 5.5m	CO1. Own weight + pretension	-1.47 kN	102
	CO2. Own weight + pretension + conventional / snow	-8.11 kN	105
	<b>Max</b> CO3. Own weight + pretension + wind pressure	-11.0 kN	108
	CO4. Own weight + pretension + wind suction – floating – reduction 0.53	-0.20 kN	111
	CO5. Own weight + pretension + wind suction – floating – full wind load *	-0.14 kN	114
Center pole 5.0m	CO1. Own weight + pretension	-1.30 kN	102
	CO2. Own weight + pretension + conventional / snow	-6.93 kN	105
	<b>Max</b> CO3. Own weight + pretension + wind pressure	-9.50 kN	108
	CO4. Own weight + pretension + wind suction – floating – reduction 0.53	-0.13 kN	111
	CO5. Own weight + pretension + wind suction – floating – full wind load *	-0.02 kN	114

Table 17. Leading forces Center poles – floating model

\* 2 stormbelts required. See Figure 1. 20x15m floating and/or drawing 20x15m – Floating in chapter E.1



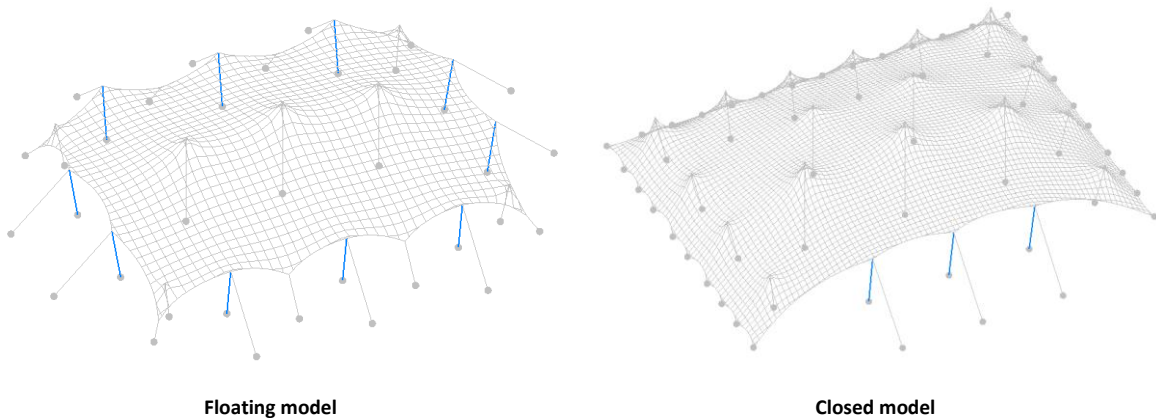
Closed model

Closed	Load combination	F <sub>rep</sub>	Pag.
Center pole 5.5m	CO1. Own weight + pretension	-1.81 kN	117
	CO2. Own weight + pretension + conventional / snow	-5.69 kN	120
	<b>Max</b> CO3. Own weight + pretension + wind pressure	-7.43 kN	123
	CO6. Own weight + pretension + wind suction – closed – reduction 0.53	-0.98 kN	126
	CO7. Own weight + pretension + wind suction – closed – full wind load *	-2.4 kN	129
	CO1. Own weight + pretension	-2.01 kN	117
	CO2. Own weight + pretension + conventional / snow	-5.96 kN	120
Center pole 5.0m	<b>Max</b> CO3. Own weight + pretension + wind pressure	-8.35 kN	123
	CO6. Own weight + pretension + wind suction – closed – reduction 0.53	-1.23 kN	126
	CO7. Own weight + pretension + wind suction – closed – full wind load *	-1.44 kN	129
	CO1. Own weight + pretension	-1.06 kN	117
	CO2. Own weight + pretension + conventional / snow	-3.75 kN	120
	<b>Max</b> CO3. Own weight + pretension + wind pressure	-5.45 kN	123
	CO6. Own weight + pretension + wind suction – closed – reduction 0.53	-0.18 kN	126
Center pole 4.0m	CO7. Own weight + pretension + wind suction – closed – full wind load *	-0.28 kN	129

Table 18. Leading forces Center poles – closed mode

\* 7 stormbelts required. See Figure 2. 20x15m closed and/or Drawing 20x15m – Closed in chapter E.2

### H.6.2.3 Entrance poles



<b>Floating</b>	Load combination	$F_{rep}$	Pag.
Entrance poles 3.0m	CO1. Own weight + pretension	-1.91 kN	102
	CO2. Own weight + pretension + conventional / snow	-6.19 kN	105
	<b>Max</b> CO3. Own weight + pretension + wind pressure	-8.81 kN	108
	CO4. Own weight + pretension + wind suction – floating – reduction 0.53	-5.19 kN	111
	CO5. Own weight + pretension + wind suction – floating – full wind load *	-7.00 kN	114

**Table 19. Leading forces entrance poles – floating model**

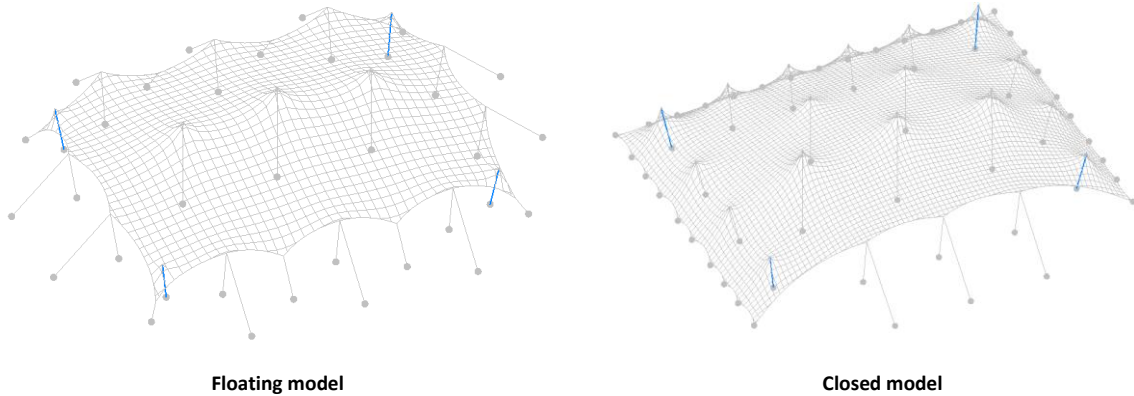
\* 2 stormbelts required. See Figure 1. 20x15m floating and/or drawing 20x15m – Floating in chapter E.1

<b>Closed</b>	Load combination	$F_{rep}$	Pag.
Entrance poles 3.0m	CO1. Own weight + pretension	-2.61 kN	117
	CO2. Own weight + pretension + conventional / snow	-4.78 kN	120
	CO3. Own weight + pretension + wind pressure	-6.58 kN	123
	CO6. Own weight + pretension + wind suction – closed – reduction 0.53	-5.41 kN	126
	<b>Max</b> CO7. Own weight + pretension + wind suction – closed – full wind load *	-9.79 kN	129

**Table 20. Leading forces entrance poles – closed model**

\* 7 stormbelts required. See Figure 2. 20x15m closed and/or Drawing 20x15m – Closed in chapter E.2

### H.6.2.4 Corner poles



Floating	Load combination	$F_{rep}$	Pag.
Corner poles 2.5m	CO1. Own weight + pretension	-0.94 kN	102
	CO2. Own weight + pretension + conventional / snow	-2.63 kN	105
	<b>Max</b> CO3. Own weight + pretension + wind pressure	-3.90 kN	108
	CO4. Own weight + pretension + wind suction – floating – reduction 0.53	-0.96 kN	111
	CO5. Own weight + pretension + wind suction – floating – full wind load *	-0.95 kN	114

**Table 21. Leading forces corner poles – floating model**

\* 2 stormbelts required. See Figure 1. 20x15m floating and/or drawing 20x15m – Floating in chapter E.1

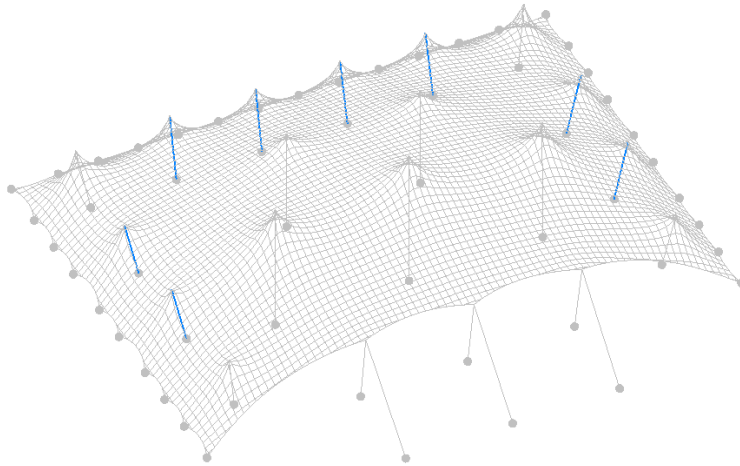
Closed	Load combination	$F_{rep}$	Pag.
Corner poles 2.5m	CO1. Own weight + pretension	-1.57 kN	117
	CO2. Own weight + pretension + conventional / snow	-3.40 kN	120
	<b>Max</b> CO3. Own weight + pretension + wind pressure	-5.76 kN	123
	CO6. Own weight + pretension + wind suction – closed – reduction 0.53	-1.14 kN	126
	CO7. Own weight + pretension + wind suction – closed – full wind load *	-1.31 kN	129

**Table 22. Leading forces corner poles – closed model**

\* 7 stormbelts required. See Figure 2. 20x15m closed and/or Drawing 20x15m – Closed in chapter E.2



### H.6.2.5 Side wall poles



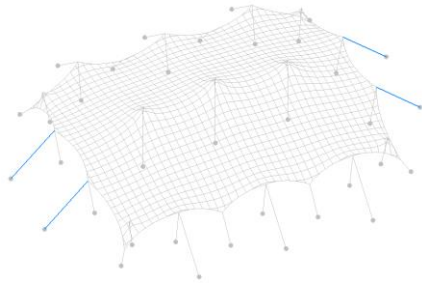
Closed model

Closed	Load combination	$F_{rep}$	Pag.
Side wall poles 2.5m	CO1. Own weight + pretension	-1.11 kN	117
	CO2. Own weight + pretension + conventional / snow	-3.16 kN	120
	<b>Max</b> CO3. Own weight + pretension + wind pressure	-5.37 kN	123
	CO6. Own weight + pretension + wind suction – closed – reduction 0.53	-0.93 kN	126
	CO7. Own weight + pretension + wind suction – closed – full wind load *	-0.94 kN	129

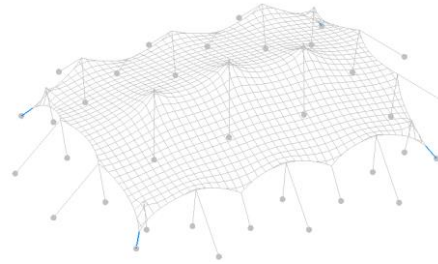
Table 23. Leading forces side wall poles – closed model

\* 7 stormbelts required. See Figure 2. 20x15m closed and/or Drawing 20x15m – Closed in chapter E.2

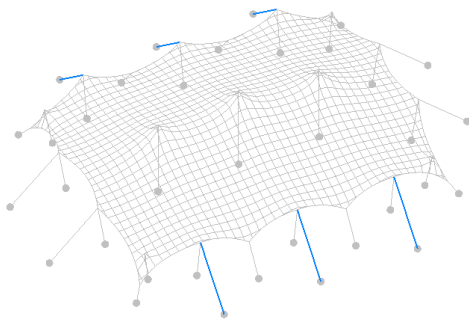
### H.6.2.6 Guy ropes



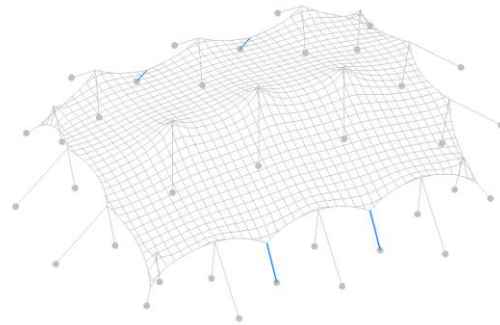
Floating model – guy ropes short side



Floating model – guy ropes corner



Floating model – guy ropes long side

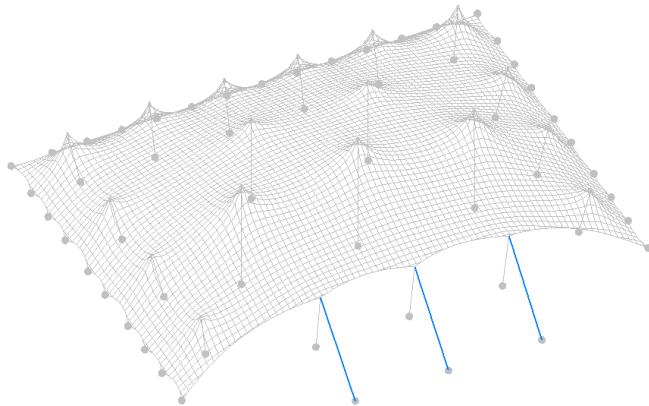


Floating model – guy ropes valley

Floating	Load combination	F <sub>rep</sub>	Pag
Guy ropes short side	CO1. Own weight + pretension	1.37 kN	101
	CO2. Own weight + pretension + conventional / snow	4.08 kN	104
	CO3. Own weight + pretension + wind pressure	5.91 kN	107
	CO4. Own weight + pretension + wind suction – floating – reduction 0.53	6.44 kN	111
	<b>Max</b> CO5. Own weight + pretension + wind suction – floating – full wind load *	9.79 kN	113
Guy ropes long side	CO1. Own weight + pretension	1.09 kN	101
	CO2. Own weight + pretension + conventional / snow	4.28 kN	104
	<b>Max</b> CO3. Own weight + pretension + wind pressure	6.43 kN	107
	CO4. Own weight + pretension + wind suction – floating – reduction 0.53	3.48 kN	111
	CO5. Own weight + pretension + wind suction – floating – full wind load *	4.98 kN	113
Guy ropes corner	CO1. Own weight + pretension	3.13 kN	101
	CO2. Own weight + pretension + conventional / snow	7.02 kN	104
	CO3. Own weight + pretension + wind pressure	9.82 kN	107
	CO4. Own weight + pretension + wind suction – floating – reduction 0.53	11.27 kN	111
	<b>Max</b> CO5. Own weight + pretension + wind suction – floating – full wind load *	15.99 kN	113
Guy ropes valley	CO1. Own weight + pretension	1.35 kN	101
	CO2. Own weight + pretension + conventional / snow	2.52 kN	104
	CO3. Own weight + pretension + wind pressure	3.53 kN	107
	<b>Max</b> CO4. Own weight + pretension + wind suction – floating – reduction 0.53	10.58 kN	111
	CO5. Own weight + pretension + wind suction – floating – full wind load *	6.47 kN	113

Table 24. Leading forces guy ropes – floating model

\* 2 stormbelts required. See Figure 1. 20x15m floating and/or drawing 20x15m – Floating in chapter E.1



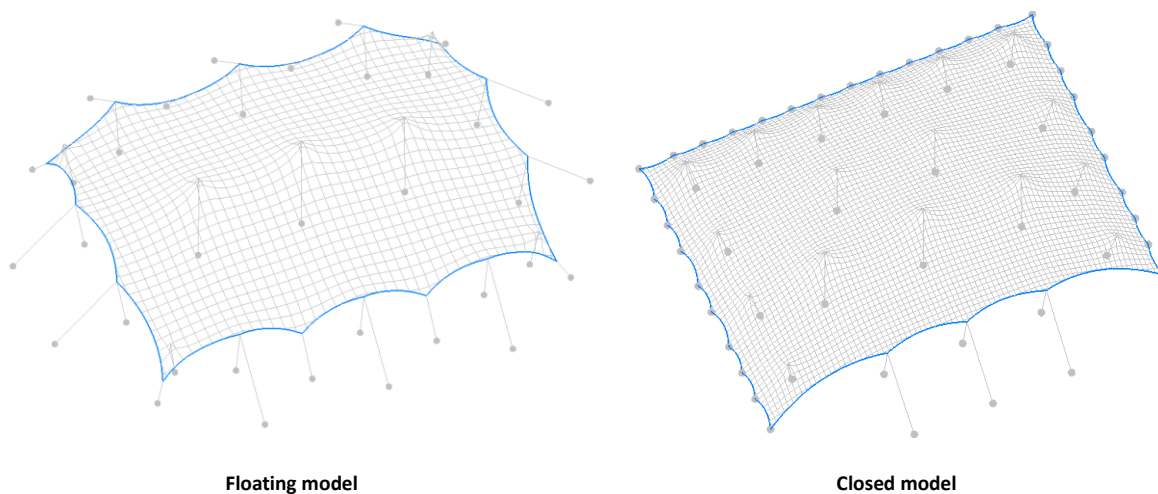
Closed model – guy ropes

Closed	Load combination	F <sub>rep</sub>	Pag.
Guy ropes	CO1. Own weight + pretension	2.21 kN	116
	CO2. Own weight + pretension + conventional / snow	4.17 kN	119
	CO3. Own weight + pretension + wind pressure	5.80 kN	123
	<b>Max</b> CO6. Own weight + pretension + wind suction – closed – reduction 0.53	9.66 kN	125
	CO7. Own weight + pretension + wind suction – closed – full wind load *	6.03 kN	128

Table 25. Leading forces guy ropes – closed model

\* 7 stormbelts required. See Figure 2. 20x15m closed and/or Drawing 20x15m – Closed in chapter E.2

### H.6.2.7 Circumference rope



<b>Floating</b>	Load combination	$F_{rep}$	Pag.
Circumference rope	CO1. Own weight + pretension	1.84 kN	101
	CO2. Own weight + pretension + conventional / snow	4.24 kN	104
	CO3. Own weight + pretension + wind pressure	5.62 kN	107
	CO4. Own weight + pretension + wind suction – floating – reduction 0.53	6.42 kN	110
	<b>Max</b> CO5. Own weight + pretension + wind suction – floating – full wind load *	9.12 kN	113

**Table 26. Leading forces Circumference rope – floating model**

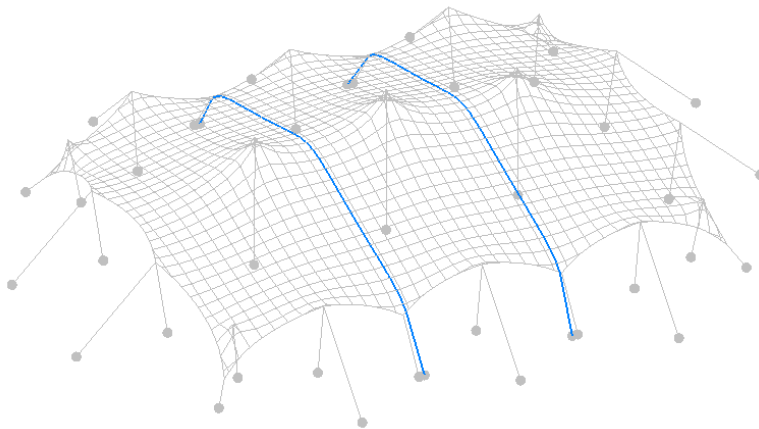
\* 2 stormbelts required. See Figure 1. 20x15m floating and/or drawing 20x15m – Floating in chapter E.1

<b>Closed</b>	Load combination	$F_{rep}$	Pag.
Circumference rope	CO1. Own weight + pretension	3.34 kN	116
	CO2. Own weight + pretension + conventional / snow	4.43 kN	119
	CO3. Own weight + pretension + wind pressure	5.74 kN	122
	CO6. Own weight + pretension + wind suction – closed – reduction 0.53	7.43 kN	125
	<b>Max</b> CO7. Own weight + pretension + wind suction – closed – full wind load *	8.27 kN	128

**Table 27. Leading forces Circumference rope – closed model**

\* 7 stormbelts required. See Figure 2. 20x15m closed and/or Drawing 20x15m – Closed in chapter E.2

### H.6.2.8 Storm belts

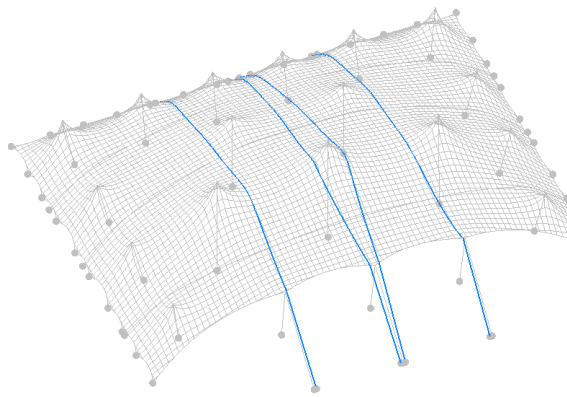


Floating model – 2 stormbelts

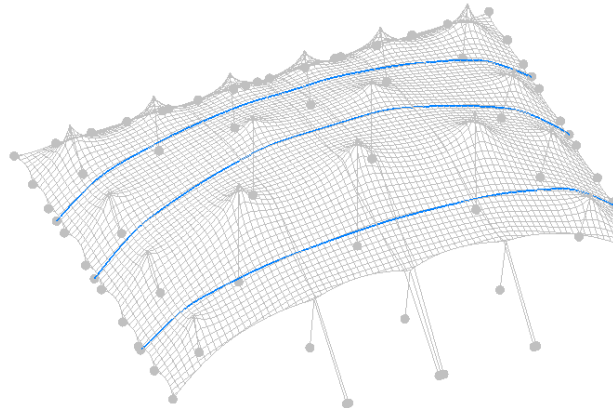
Floating	Load combination	F <sub>rep</sub>	Pag.
Storm Belts	CO1. Own weight + pretension	No stormbelt	-
	CO2. Own weight + pretension + conventional / snow	No stormbelt	-
	CO3. Own weight + pretension + wind pressure	No stormbelt	-
	CO4. Own weight + pretension + wind suction – floating – reduction 0.53	No stormbelt	-
	<b>Max</b> CO5. Own weight + pretension + wind suction – floating – full wind load *	14.98 kN	114

Table 28. Leading forces storm belts – floating model

\* 2 stormbelts required. See Figure 1. 20x15m floating and/or drawing 20x15m – Floating in chapter E.1



Closed model – stormbelts width direction



Closed model – stormbelts length direction

Closed	Load combination	F <sub>rep</sub>	Pag.
Storm Belts width	CO1. Own weight + pretension	No stormbelt	-
	CO2. Own weight + pretension + conventional / snow	No stormbelt	-
	CO3. Own weight + pretension + wind pressure	No stormbelt	-
	CO6. Own weight + pretension + wind suction – closed – reduction 0.53	No stormbelt	-
	<b>Max</b> CO7. Own weight + pretension + wind suction – closed – full wind load *	12.24 kN	129
Storm Belts length	CO1. Own weight + pretension	No stormbelt	-
	CO2. Own weight + pretension + conventional / snow	No stormbelt	-
	CO3. Own weight + pretension + wind pressure	No stormbelt	-
	CO6. Own weight + pretension + wind suction – closed – reduction 0.53	No stormbelt	-
	<b>Max</b> CO7. Own weight + pretension + wind suction – closed – full wind load *	14.46 kN	129

Table 29. Leading forces storm belts – closed model

\* 7 stormbelts required. See Figure 2. 20x15m closed and/or Drawing 20x15m – Closed in chapter E.2

## H.7. Check elements

### H.7.1 Membrane

Load combination	Element	Representative stress	Design value stress	Pag.
CO7. Own weight + pretension + wind suction – closed – full wind load	Membrane Short term load warp direction	6.33 kN/m *	9.50 kN/m ( $\gamma = 1.5$ )	31
CO4. Own weight + pretension + wind suction – floating – reduction 0.53	Membrane Short term load weft direction	4.76 kN/m *	7.14 kN/m ( $\gamma = 1.5$ )	31

\* As shown in Annex B, the stresses in the membrane exceed the representative limit of 6.58 kN/m for the warp direction and 4.76 kN/m for the weft direction only at local points where either the membrane is reinforced and consists out of multiple layers or the calculated stress is higher than in reality because of the cable net modeling.

Contour 3xl FR or Contour X FR is being used.

<b>UC.1a</b>	<b><math>S_{Ed} / S_{rd} &lt; 1</math></b>	<b><math>9.50 / 9.5 = 1 = 1</math></b>	<b>OK</b>
<b>UC.1b</b>	<b><math>S_{Ed} / S_{rd} &lt; 1</math></b>	<b><math>7.14 / 7.0 = 1.02 \approx 1</math></b>	<b>ACCEPTABLE</b>

For capacity of membrane see H.2, page 22

## H.7.2 Center poles

Load combinations	Element	Representative force	Design value force	Pag.
CO3. Own weight + pretension + wind pressure	Center pole 5.5m floating	-11.0 kN	-16.5 kN ( $\gamma = 1.5$ )	33
CO3. Own weight + pretension + wind pressure	Center pole 5.0m floating	-9.5 kN	-14.25 kN ( $\gamma = 1.5$ )	33
CO3. Own weight + pretension + wind pressure	Center pole 5.5m closed	-7.43 kN	-11.15 kN ( $\gamma = 1.5$ )	33
CO3. Own weight + pretension + wind pressure	Center pole 5.0m closed	-8.35 kN	-12.53 kN ( $\gamma = 1.5$ )	33
CO3. Own weight + pretension + wind pressure	Center pole 4.5m closed	-5.45 kN	-8.18 kN ( $\gamma = 1.5$ )	33

User load of max. 10 kg is applied, loaded centrally.

5.5m pole, floating	$N_{ed} = -16.5 + (1.35 \times -0.1) = -16.6 \text{ kN}$
5.0m pole, floating	$N_{ed} = -14.25 + (1.35 \times -0.1) = -14.4 \text{ kN}$
5.5m pole, closed	$N_{ed} = -11.15 + (1.35 \times -0.1) = -11.3 \text{ kN}$
5.0m pole, closed	$N_{ed} = -12.53 + (1.35 \times -0.1) = -12.7 \text{ kN}$
4.0m pole, closed	$N_{ed} = -8.18 + (1.35 \times -0.1) = -8.3 \text{ kN}$

### H.7.2.1 5.5m, floating - Wood, Eucalyptus D35

Profile	=	Pole, $\varnothing \approx 120 \text{ mm}$ average diameter, as a minimum required at the middle of the pole
Length	=	max. 5.5m
Quality	=	D35

The poles are considered as hinged poles; the buckling length is equivalent to the pole length.

<b>UC.2a</b>	<b>5.5m</b>	$\sigma_{c,0,d} / (f_{c,0,d} \times k_{cy}) = 1.47 / (17.31 \times 0.096) = 0.88 < 1$	<b>OK</b>
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See Annex C.1 Center pole 5.5m, floating – wood, on page 130 for the elaborate check

### H.7.2.2 5.5m\*, floating - Aluminium, 6063 T6

Profile	=	Pole, $\varnothing 90 \times 3 \text{ OR } \varnothing 76 \times 5 \text{ mm}$
Length	=	max. 5.0m *
Quality	=	6063 T6

\* Lower 5.5m center pole to 5.0m when using aluminium profiles.

The poles are considered as hinged poles; the buckling length is equivalent to the pole length.

<b>UC.2b</b>	<b>5.0m* - <math>\varnothing 90 \times 3</math></b>	$(N_{ed} / \chi \omega N_{rd})^{0.8} = (16.6 / 0.150 * 1 * 119.27)^{0.8} = 0.94 < 1$	<b>OK</b>
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See Annex C.2 Center pole 5.0m\*, floating – aluminium 90x3mm, on page 131 for the elaborate check

<b>UC.2c</b>	<b>5.0m* - <math>\varnothing 76 \times 5</math></b>	$(N_{ed} / \chi \omega N_{rd})^{0.8} = (16.6 / 0.102 * 1 * 162.22)^{0.8} = 1.00 < 1$	<b>OK</b>
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See Annex C.3 Center pole 5.0m\*, floating – aluminium 76x5mm, on page 132 for the elaborate check



### H.7.2.3 5.0m, floating - Wood, Eucalyptus D35

Profile	=	Pole, $\varnothing \approx 110$ mm average diameter, as a minimum required at the middle of the pole
Length	=	max. 5.0m
Quality	=	D35

The poles are considered as hinged poles; the buckling length is equivalent to the pole length.

<b>UC.3a</b>	<b>5m</b>	$\sigma_{c,0,d} / (f_{c,0,d} \times k_{cy}) = 1.52 / (17.31 \times 0.0976) = 0.90 < 1$	<b>OK</b>
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See Annex C.4 Center pole 5.0m, floating – wood, on page 133 for the elaborate check

### H.7.2.4 5.0m, floating – Aluminium, 6063 T6

Profile	=	Pole, $\varnothing 90 \times 3$ OR $\varnothing 76 \times 4$ mm
Length	=	max. 5.0m
Quality	=	6063 T6

The poles are considered as hinged poles; the buckling length is equivalent to the pole length.

<b>UC.3b</b>	<b>5m - <math>\varnothing 90 \times 3</math></b>	$(N_{ed} / \chi \omega N_{rd})^{0.8} = (14.4 / 0.150 * 1 * 119.27)^{0.8} = 0.84 < 1$	<b>OK</b>
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See Annex C.5 Center pole 5.0m, floating – aluminium 90x3mm, on page 134 for the elaborate check

<b>UC.3c</b>	<b>5m - <math>\varnothing 76 \times 4</math></b>	$(N_{ed} / \chi \omega N_{rd})^{0.8} = (14.4 / 0.105 * 1 * 131.60)^{0.8} = 1.04 = 1$	<b>OK</b>
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See Annex C.6 Center pole 5.0m, floating – aluminium 76x4mm, on page 135 for the elaborate check

### H.7.2.5 5.5m, closed - Wood, Eucalyptus D35

Profile	=	Pole, $\varnothing \approx 110$ mm average diameter, as a minimum required at the middle of the pole
Length	=	max. 5.5m
Quality	=	D35

The poles are considered as hinged poles; the buckling length is equivalent to the pole length.

<b>UC.4a</b>	<b>5.5m</b>	$\sigma_{c,0,d} / (f_{c,0,d} \times k_{cy}) = 1.19 / (17.31 \times 0.0812) = 0.85 < 1$	<b>OK</b>
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See Annex C.7 Center pole 5.5m, closed – wood, on page 136 for the elaborate check

### H.7.2.6 5.5m, closed – Aluminium, 6063 T6

Profile	=	Pole, Ø90x3 OR Ø76x4mm
Length	=	max. 5.5m
Quality	=	6063 T6

The poles are considered as hinged poles; the buckling length is equivalent to the pole length.

<b>UC.4b</b>	<b>5.5m - Ø90x3</b>	$(N_{ed} / \chi \omega N_{rd})^{0.8} = (11.3 / 0.125 * 1 * 119.27)^{0.8} = 0.80 < 1$	<b>OK</b>
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See Annex C.8 Center pole 5.5m, closed – aluminium 90x3mm, on page 137 for the elaborate check

<b>UC.4c</b>	<b>5.5m - Ø76x4</b>	$(N_{ed} / \chi \omega N_{rd})^{0.8} = (11.3 / 0.087 * 1 * 131.60)^{0.8} = 0.99 < 1$	<b>OK</b>
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See Annex C.9 Center pole 5.5m, closed – aluminium 76x4mm, on page 138 for the elaborate check

### H.7.2.7 5.0m, closed - Wood, Eucalyptus D35

Profile	=	Pole, Ø ≈ 110 mm average diameter, as a minimum required at the middle of the pole
Length	=	max. 5.0m
Quality	=	D35

The poles are considered as hinged poles; the buckling length is equivalent to the pole length.

<b>UC.5a</b>	<b>5m</b>	$\sigma_{c,0,d} / (f_{c,0,d} \times k_{cy}) = 1.34 / (17.31 \times 0.0976) = 0.79 < 1$	<b>OK</b>
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See Annex C.10 Center pole 5.0m, closed – wood, on page 139 for the elaborate check

### H.7.2.8 5.0m, closed – Aluminium, 6063 T6

Profile	=	Pole, Ø90x3 OR Ø76x4mm
Length	=	max. 5.0m
Quality	=	6063 T6

The poles are considered as hinged poles; the buckling length is equivalent to the pole length.

<b>UC.5b</b>	<b>5m - Ø90x3</b>	$(N_{ed} / \chi \omega N_{rd})^{0.8} = (12.7 / 0.150 * 1 * 119.27)^{0.8} = 0.76 < 1$	<b>OK</b>
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See Annex C.11 Center pole 5.0m, closed – Aluminium 90x3mm, on page 140 for the elaborate check

<b>UC.5c</b>	<b>5m - Ø76x4</b>	$(N_{ed} / \chi \omega N_{rd})^{0.8} = (12.7 / 0.105 * 1 * 131.60)^{0.8} = 0.94 < 1$	<b>OK</b>
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See Annex C.12 Center pole 5.0m, closed – Aluminium 76x4mm, on page 141 for the elaborate check

#### H.7.2.9 4.0m, closed - Wood, Eucalyptus D35

Profile	=	Pole, Ø90x3 OR Ø76x4mm average diameter, as a minimum required at the middle of the pole
Length	=	max. 4.0m
Quality	=	D35

The poles are considered as hinged poles; the buckling length is equivalent to the pole length.

<b>UC.6a</b>	<b>4m</b>	$\sigma_{c,0,d} / (f_{c,0,d} \times k_{cy}) = 1.46 / (17.31 \times 0.0913) = 0.93 < 1$	<b>OK</b>
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See Annex C.13 Center pole 4.0m, closed – wood, on page 142 for the elaborate check

#### H.7.2.10 4.0m, closed – Aluminium, 6063 T6

Profile	=	Pole, Ø76x3mm
Length	=	max. 4.0m
Quality	=	6063 T6

The poles are considered as hinged poles; the buckling length is equivalent to the pole length.

<b>UC.6b</b>	<b>4m</b>	$(N_{ed} / \chi \omega N_{rd})^{0.8} = (8.3 / 0.164 * 1 * 100.07)^{0.8} = 0.58 < 1$	<b>OK</b>
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See Annex C.14 Center pole 4.0m, closed – Aluminium 76x4mm, on page 143 for the elaborate check

### H.7.3 Entrance poles

Load combinations	Element	Representative force	Design value force	Pag.
CO3. Own weight + pretension + wind pressure	Entrance pole 3.0m floating	-8.81 kN	-13.2 kN ( $\gamma = 1.5$ )	33
CO7. Own weight + pretension + wind suction – closed – full wind load	Entrance pole 3.0m closed	-9.79 kN	-14.7 kN ( $\gamma = 1.5$ )	33

User load of max. 10 kg is applied, loaded centrally.

3m pole, floating  $N_{ed} = -13.2 + (1.35 \times -0.1) = -13.34$  kN

3m pole, closed  $N_{ed} = -14.70 + (1.35 \times -0.1) = -14.84$  kN

#### H.7.3.1 3m, floating - Wood, Eucalyptus D35

Profile = Pole,  $\varnothing \approx 85$  mm  
average diameter, as a minimum required at the middle of the pole  
Length = max. 3.0m  
Quality = D35

The poles are considered as hinged poles; the buckling length is equivalent to the pole length.

<b>UC.7a</b>	<b>3m</b>	$\sigma_{c,0,d} / (f_{c,0,d} \times k_{cy}) = 2.35 / (17.31 \times 0.1586) = 0.86 < 1$	<b>OK</b>
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See Annex C.15 Entrance pole 3.0m, floating– wood, on page 144 for the elaborate check

#### H.7.3.2 3m, floating - Aluminium, 6063 T6

Profile = Pole,  $\varnothing 76 \times 3$ mm  
Length = max. 3.0m  
Quality = 6063 T6

The poles are considered as hinged poles; the buckling length is equivalent to the pole length.

<b>UC.7b</b>	<b>3m</b>	$(N_{ed} / \chi \omega N_{rd})^{0.8} = (13.34 / 0.279 * 1 * 100.07)^{0.8} = 0.55 < 1$	<b>OK</b>
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See Annex C.16 Entrance pole 3.0m, floating – Aluminium 76x3mm, on page 145 for the elaborate check

### H.7.3.3 3m, closed - Wood, Eucalyptus D35

Profile	=	Pole, $\varnothing \approx 85$ mm average diameter, as a minimum required at the middle of the pole
Length	=	max. 3.0m
Quality	=	D35

The poles are considered as hinged poles; the buckling length is equivalent to the pole length.

<b>UC.8a</b>	<b>3m</b>	$\sigma_{c,0,d} / (f_{c,0,d} \times k_{cy}) = 2.62 / (17.31 \times 0.1586) = 0.95 < 1$	<b>OK</b>
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See Annex C.17 Entrance pole 3.0m, closed– wood, on page 146 for the elaborate check

### H.7.3.4 3m, closed - Aluminium, 6063 T6

Profile	=	Pole, $\varnothing 76 \times 3$ mm
Length	=	max. 3.0m
Quality	=	6063 T6

The poles are considered as hinged poles; the buckling length is equivalent to the pole length.

<b>UC.8b</b>	<b>3m</b>	$(N_{ed} / \chi \omega N_{rd})^{0.8} = (14.84 / 0.279 * 1 * 100.07)^{0.8} = 0.60 < 1$	<b>OK</b>
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See Annex C.18 Entrance pole 3.0m, closed – Aluminium 76x3mm, on page 147 for the elaborate check

#### H.7.4 Corner poles

Load combinations	Element	Representative force	Design value force	Pag.
CO3. Own weight + pretension + wind pressure	Corner pole 2.5m floating	-3.90 kN	-5.85 kN ( $\gamma = 1.5$ )	33
CO3. Own weight + pretension + wind pressure	Corner pole 2.5m closed	-5.76 kN	-8.64 kN ( $\gamma = 1.5$ )	33

User load of max. 10 kg is applied, loaded centrally.

2.5m corner pole, floating  $N_{ed} = -5.85 + (1.35 \times -0.1) = -5.99$  kN

2.5m corner pole, closed  $N_{ed} = -8.64 + (1.35 \times -0.1) = -8.78$  kN

##### H.7.4.1 2.5m, floating - Wood, Eucalyptus D35

Profile = Pole,  $\varnothing \approx 65$  mm  
 average diameter, as a minimum required at the middle of the pole  
 Length = max. 2.5m  
 Quality = D35

The poles are considered as hinged poles; the buckling length is equivalent to the pole length.

<b>UC.9a</b>	<b>2.5m</b>	$\sigma_{c,0,d} / (f_{c,0,d} \times k_{cy}) = 1.81 / (17.31 \times 0.1346) = 0.77 < 1$	<b>OK</b>
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See Annex C.19 Corner pole 2.5m, floating – wood, on page 148 for the elaborate check

##### H.7.4.2 2.5m, floating - Aluminium, 6063 T6

Profile = Pole,  $\varnothing 50 \times 3$ mm  
 Length = max. 2.5m  
 Quality = 6063 T6

The poles are considered as hinged poles; the buckling length is equivalent to the pole length.

<b>UC.9b</b>	<b>2.5m</b>	$(N_{ed} / \chi \omega N_{rd})^{0.8} = (5.99 / 0.174 * 1 * 64.43)^{0.8} = 0.61 < 1$	<b>OK</b>
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See Annex C.20 Corner pole 2.5m, floating – Aluminium 50x3mm, on page 149 for the elaborate check

#### H.7.4.3 2.5m, closed - Wood, Eucalyptus D35

Profile	=	Pole, $\varnothing \approx 70$ mm average diameter, as a minimum required at the middle of the pole
Length	=	max. 2.5m
Quality	=	D35

The poles are considered as hinged poles; the buckling length is equivalent to the pole length.

<b>UC.10a</b>	<b>2.5m</b>	$\sigma_{c,0,d} / (f_{c,0,d} \times k_{cy}) = 2.28 / (17.31 \times 0.1551) = 0.85 < 1$	<b>OK</b>
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See Annex C.21 Corner pole 2.5m, closed – wood, on page 150 for the elaborate check

#### H.7.4.4 2.5m, closed - Aluminium, 6063 T6

Profile	=	Pole, $\varnothing 50 \times 3$ mm
Length	=	max. 2.5m
Quality	=	6063 T6

The poles are considered as hinged poles; the buckling length is equivalent to the pole length.

<b>UC.10b</b>	<b>2.5m</b>	$(N_{ed} / \chi \omega N_{rd})^{0.8} = (8.78 / 0.174 * 1 * 64.43)^{0.8} = 0.82 < 1$	<b>OK</b>
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See Annex C.22 Corner pole 2.5m, closed – Aluminium 50x3mm, on page 151 for the elaborate check

### H.7.5 Side wall poles

Load combinations	Element	Representative force	Design value force	Pag.
CO3. Own weight + pretension + wind pressure	Side wall pole 2.5m closed	-5.37 kN	-8.06 kN ( $\gamma = 1.5$ )	33

User load of max. 10 kg is applied, loaded centrally.

2.5m side wall pole, closed  $N_{ed} = -8.06 + (1.35 \times -0.1) = -8.19$  kN

#### H.7.5.1 2.5m, closed - Wood, Eucalyptus D35

Profile = Pole,  $\varnothing \approx 70$  mm  
 average diameter, as a minimum required at the middle of the pole  
 Length = max. 2.5m  
 Quality = D35

The poles are considered as hinged poles; the buckling length is equivalent to the pole length.

<b>UC.11a</b>	<b>2m</b>	$\sigma_{c,0,d} / (f_{c,0,d} \times k_{cy}) = 2.13 / (17.31 \times 0.1551) = 0.79 < 1$	<b>OK</b>
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See Annex C.23 Side wall pole 2.5m, closed – wood, on page 152 for the elaborate check

#### H.7.5.2 2.5m, closed - Aluminium, 6063 T6

Profile = Pole,  $\varnothing 50 \times 3$ mm  
 Length = max. 2.5m  
 Quality = 6063 T6

The poles are considered as hinged poles; the buckling length is equivalent to the pole length.

<b>UC.11b</b>	<b>2.5m</b>	$(N_{ed} / \chi \omega N_{rd})^{0.8} = (8.19 / 0.174 * 1 * 64.43)^{0.8} = 0.78 < 1$	<b>OK</b>
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See Annex C.24 Side wall pole 2.5m, closed – Aluminium 50x3mm, on page 153 for the elaborate check



## H.7.6 Guy ropes

Load combination	Element	Representative force	Design value force	Pag.
CO5. Own weight + pretension + wind suction – floating – full wind load	Guy ropes floating short side	9.79 kN	14.69 kN ( $\gamma = 1.5$ )	38
CO3. Own weight + pretension + wind pressure	Guy ropes floating long side	6.43 kN	9.65 kN ( $\gamma = 1.5$ )	38
CO5. Own weight + pretension + wind suction – floating – full wind load	Guy ropes floating Corner*	$15.99 / 2 = 8.00 \text{ kN}^*$	12.0 kN ( $\gamma = 1.5$ )	38
CO4. Own weight + pretension + wind suction – floating – reduction 0.53	Guy ropes floating valley	10.58 kN	15.87 kN ( $\gamma = 1.5$ )	38
CO6. Own weight + pretension + wind suction – closed – reduction 0.53	Guy ropes closed	9.66 kN	14.49 kN ( $\gamma = 1.5$ )	38

\* In the calculation model only 1 guy rope is modelled at the corner. In practice, two individual guy ropes will be necessary / used.

It is possible to use either a rope or a belt.

The 7mm guy ropes have a minimum breaking strength of 910 kg  $\rightarrow$   $F_{rd,7mm} = 2.3 \text{ kN}$ .

The 14mm guy ropes have a minimum breaking strength of 3640 kg  $\rightarrow$   $F_{rd, 14mm} = 11 \text{ kN}$

The ropes need to be tied back, creating multiple rope sections that together will take the load.

<b>UC.12a</b>	Guy ropes floating short side	7 sections	$F_d / F_{rd} < 1$	$14.69 / (7 \times 2.3) = 0.91 < 1$	<b>OK</b>
		2 sections		$14.69 / (2 \times 11) = 0.67 < 1$	<b>OK</b>
<b>UC.12b</b>	Guy ropes floating long side	4 sections	$F_d / F_{rd} < 1$	$9.65 / (4 \times 2.3) = 1.05 \approx 1$	<b>Acceptable</b>
		1 sections		$9.65 / (1 \times 11) = 0.87 < 1$	<b>OK</b>
<b>UC.12c</b>	Guy ropes floating corner	5 sections	$F_d / F_{rd} < 1$	$12.0 / (5 \times 2.3) = 1.04 \approx 1$	<b>Acceptable</b>
		2 sections		$12.0 / (2 \times 11) = 0.55 < 1$	<b>OK</b>
<b>UC.12d</b>	Guy ropes floating valley	7 sections	$F_d / F_{rd} < 1$	$15.87 / (7 \times 2.3) = 0.99 < 1$	<b>OK</b>
		2 sections		$15.87 / (2 \times 11) = 0.72 < 1$	<b>OK</b>
<b>UC.12e</b>	Guy ropes closed	7 sections	$F_d / F_{rd} < 1$	$14.49 / (7 \times 2.3) = 0.90 < 1$	<b>OK</b>
		2 sections		$14.49 / (2 \times 11) = 0.66 < 1$	<b>OK</b>

For capacity of ropes see 0, page 23

It is also possible to use alternative **ropes** with a higher breaking strength ( $BL_{\text{rope}}$ ) and less rope sections (n), as long as  $BL_{\text{rope}} \times n \geq 5870 \text{ kg}$ .

In case **PES belts** are used, a minimum breaking strength of **3000 kg** is required.

### H.7.7 Circumference rope

Load combination	Element	Representative force	Design value force	Pag.
CO5. Own weight + pretension + wind suction – floating – full wind load	Circumference rope Floating	9.12 kN	13.68 kN ( $\gamma = 1.5$ )	40
CO4. Own weight + pretension + wind suction – floating – reduction 0.53	Circumference rope Floating	6.42 kN	9.63 kN ( $\gamma = 1.5$ )	40
CO7. Own weight + pretension + wind suction – closed – full wind load	Circumference rope closed	8.27 kN	12.4 kN ( $\gamma = 1.5$ )	40
CO6. Own weight + pretension + wind suction – closed – reduction 0.53	Circumference rope closed	7.43 kN	11.15 kN ( $\gamma = 1.5$ )	40

It is possible to use either a rope or a belt.

<b>UC.13a</b>	Circumference rope	2 sections	$F_{d,red} / F_{rd} < 1$	$13.68 / (2 \times 11) = 0.62 < 1$	<b>OK</b>
<b>UC.13b</b>	Circumference rope	1 section	$F_d / F_{rd} < 1$	$9.63 / (1 \times 11) = 0.88 < 1$	<b>OK</b>
<b>UC.13a</b>	Circumference rope	2 sections	$F_{d,red} / F_{rd} < 1$	$12.4 / (2 \times 11) = 0.56 < 1$	<b>OK</b>
<b>UC.13b</b>	Circumference rope	1 section	$F_d / F_{rd} < 1$	$11.15 / (1 \times 11) = 1.01 \approx 1$	<b>ACCEPTABLE</b>

For capacity of ropes see H.2.3, page 23

When the tent is built for wind load up to and until 7 BFt, a 14mm rope with 1 rope section satisfies.

If the tent is built for full wind load, a 14mm rope with 2 rope sections is required.

Otherwise, a PES belt, with a minimum breaking strength of **2750 kg** can be used.

It is also possible to use alternative ropes with a higher breaking strength ( $BL_{rope}$ ) and less rope sections (n), as long as  $BL_{rope} \times n \geq 4500 \text{ kg}$  (for ropes sections  $\geq 14\text{mm}$ ).

### H.7.8 Storm belts

Load combination	Element	Representative force	Design value force	Pag.
CO5. Own weight + pretension + wind suction – floating – full wind load	Storm belt floating	14.98 kN	22.47 kN ( $\gamma = 1.5$ )	41

Belts are PES belts with a minimum breaking strength of **5000kg**.

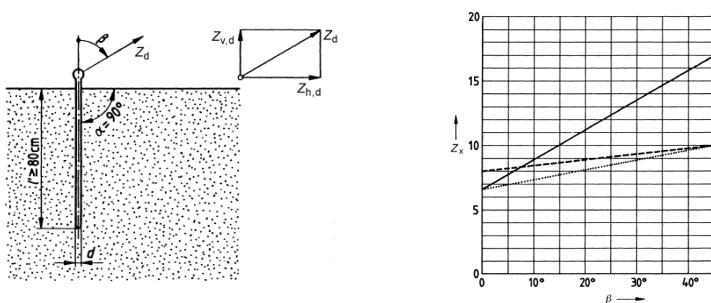
<b>UC.14</b>	Storm belt	$F_d / F_{rd} < 1$	$24.47 / 25 = 0.98 < 1.0$	<b>OK</b>
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For capacity of belts see H.2.2, page 22

## H.8. Safety against overturning, sliding and uplifting

The calculations in this chapter provides a guideline for dense cohesion less soil, in case no anchor tests have been performed. Anchor tests on location can show that a different amount of anchors or different size of anchors have sufficient capacity for the specific soil conditions. (see paragraph 0)

### H.8.1 Anchor capacity



Angle of pull	Load bearing capacity <i>N</i>
$\beta = 0$	$Z_d = 6,5 \text{ dl}'$ for stiff cohesive and for dense cohesion less soils
$\beta = 0$	$Z_d = 8 \text{ dl}'$ for very stiff cohesive soils
$\beta \geq 45$	$Z_d = 10 \text{ dl}'$ for cohesive soils of at least medium to stiff consistency
$\beta \geq 45$	$Z_d = 17 \text{ dl}'$ for dense cohesion less soils
$0 < \beta < 45$	The load bearing capacity for the soil types shall be determined by interpolation

$Z_d$  is the anchor service load (service load), in N;  
 $Z_{h,d}$  is the horizontal anchor service load, in N;  
 $Z_{v,d}$  is the vertical anchor service load, in N;  
 $d$  is the anchor diameter, in cm;  
 $l'$  is the depth of penetration in cm;  
 $\alpha$  is the angle of penetration;  
 $\beta$  is the angle of acting tensile force to the vertical

figure 8. Taken from from NEN-EN 13782: Figures 4 & 5, table 5

Anchor of **Ø35mm** are used, taking into account an **effective length of at least 1200 mm**.

If multiple anchors are placed at the same location, the anchors need to be at least  $5x \varnothing$  apart to exploit the full capacity of the anchors.

Anchor capacity for anchors based in dense cohesion less soil (sandy soil)

Ø35 x 1200 mm		
Angle	$\beta$	$\geq 45$
Effective length anchor	$l'$	120 cm
Diameter anchor	$d$	3.5 cm
Anchor capacity*	$Z_d$	7.14 kN

\*Calculated under the assumption the anchor is based in dense cohesion less soil.

## H.8.2 Required anchor pins

### H.8.2.1 Floating

#### H.8.2.1.1. Reduced wind pressure

Load combination	Element	Representative force	Design value force	Pag.
CO4: Wind suction – floating – reduction 0.53	Guy ropes short side	6.44 kN	7.73 kN (y=1.2)	111
CO4: Wind suction – floating – reduction 0.53	Guy ropes long side	3.48 kN	4.18 kN (y=1.2)	111
CO4: Wind suction – floating – reduction 0.53	Guy ropes corner	11.27 kN*	13.52 kN (y=1.2)	111
CO4: Wind suction – floating – reduction 0.53	Guy ropes valley	10.58 kN	12.70 kN (y=1.2)	111

Anchoring – dense cohesion less soil

<b>Guy ropes - short side</b>	<b>2x</b>	<b>Ø35 x 1200 mm</b>	$F_d / F_{rd} = 7.73 / (2 \times 7.14) = 0.54 < 1$	<b>OK</b>
<b>Guy ropes - long side</b>	<b>1x</b>	<b>Ø35 x 1200 mm</b>	$F_d / F_{rd} = 4.18 / (1 \times 7.14) = 0.59 < 1$	<b>OK</b>
<b>Guy ropes – corner</b>	<b>2x</b>	<b>Ø35 x 1200 mm*</b>	$F_d / F_{rd} = 13.52 / (2 \times 7.14) = 0.95 < 1$	<b>OK</b>
<b>Guy ropes – valley</b>	<b>2x</b>	<b>Ø35 x 1200 mm</b>	$F_d / F_{rd} = 12.70 / (2 \times 7.14) = 0.89 < 1$	<b>OK</b>

\*In the corners only 1 guy rope is modelled, however 2 will be applied in practice. Therefore, 2 guy ropes with 1 anchor each can be applied.

#### H.8.2.1.2. Full wind pressure

Load combination	Element	Representative force	Design value force	Pag.
CO5: Wind suction – floating – full wind load	Guy ropes short side	9.79 kN	11.75 kN (y=1.2)	113
CO5: Wind suction – floating – full wind load	Guy ropes long side	4.98 kN	5.98 kN (y=1.2)	113
CO5: Wind suction – floating – full wind load	Guy ropes corner	15.99 kN*	19.19 kN (y=1.2)	113
CO5: Wind suction – floating – full wind load	Guy ropes valley	6.47 kN	7.76 kN (y=1.2)	113
CO5: Wind suction – floating – full wind load	Storm belts	14.98 kN	17.98 kN (y=1.2)	114

Anchoring – dense cohesion less soil

<b>Guy ropes – short side</b>	<b>2x</b>	<b>Ø35 x 1200 mm</b>	$F_d / F_{rd} = 11.75 / (2 \times 7.14) = 0.82 < 1$	<b>OK</b>
<b>Guy ropes – long side</b>	<b>1x</b>	<b>Ø35 x 1200 mm</b>	$F_d / F_{rd} = 5.98 / (1 \times 7.14) = 0.84 < 1$	<b>OK</b>
<b>Guy ropes – corner</b>	<b>3x</b>	<b>Ø35 x 1200 mm*</b>	$F_d / F_{rd} = 19.19 / (3 \times 7.14) = 0.90 < 1$	<b>OK</b>
<b>Guy ropes – valley</b>	<b>2x</b>	<b>Ø35 x 1200 mm</b>	$F_d / F_{rd} = 7.76 / (2 \times 7.14) = 0.54 < 1$	<b>OK</b>
<b>Storm belts</b>	<b>3x</b>	<b>Ø35 x 1200 mm</b>	$F_d / F_{rd} = 17.98 / (3 \times 7.14) = 0.84 < 1$	<b>OK</b>

\*In the corners only 1 guy rope is modelled, however 2 will be applied in practice. Therefore, 2 guy ropes with 2 anchors each can be applied.

## H.8.2.2 Closed

### H.8.2.2.1. Reduced wind pressure

Load combination	Element	Representative force	Design value force	Pag.
CO6: Wind suction – closed – reduction 0.53	Guy ropes	9.66 kN	11.60 kN (y=1.2)	125
CO6: Wind suction – closed – reduction 0.53	Ground point Corner front		12.73 kN (y=1.2)*	172
CO6: Wind suction – closed – reduction 0.53	Ground point Corner back		4.25 kN (y=1.2)*	173
CO6: Wind suction – closed – reduction 0.53	Ground points Short side		6.72 kN (y=1.2)*	176
CO6: Wind suction – closed – reduction 0.53	Ground points Long side		3.31 kN (y=1.2)*	178

Anchoring – dense cohesion less soil

<b>Guy ropes</b>	<b>2x</b>	<b>Ø35 x 1200 mm</b>	$F_d / F_{rd} = 11.60 / (2 \times 7.14) = 0.81 < 1$	<b>OK</b>
<b>Ground point Corner front</b>	<b>2x</b>	<b>Ø35 x 1200 mm</b>	$F_d / F_{rd} = 12.73 / (2 \times 7.14) = 0.89 < 1$	<b>OK</b>
<b>Ground point Corner back</b>	<b>1x</b>	<b>Ø35 x 1200 mm</b>	$F_d / F_{rd} = 4.25 / (1 \times 7.14) = 0.60 < 1$	<b>OK</b>
<b>Ground points Short side</b>	<b>1x</b>	<b>Ø35 x 1200 mm</b>	$F_d / F_{rd} = 6.72 / (1 \times 6.99) = 0.96 < 1$	<b>OK</b>
<b>Ground points Long side</b>	<b>1x</b>	<b>Ø35 x 1200 mm</b>	$F_d / F_{rd} = 3.31 / (1 \times 6.56) = 0.50 < 1$	<b>OK</b>

\* 0 shows the acting forces and the verification of the anchoring for the ground points and stormbelts

#### H.8.2.2.2. Full wind pressure

Load combination	Element	Representative force	Design value force	Pag.
CO7: Wind suction – closed – full wind load	Guy ropes	6.03 kN	7.24 kN (y=1.2)*	128
CO7: Wind suction – closed – full wind load	Ground point Corner front		14.53 kN (y=1.2)*	172
CO7: Wind suction – closed – full wind load	Ground point Corner back		6.57 kN (y=1.2)*	173
CO7: Wind suction – closed – full wind load	Ground points Short side		5.81 kN (y=1.2)*	174
CO7: Wind suction – closed – full wind load	Ground points Long side		4.15 kN (y=1.2)*	176
CO7: Wind suction – closed – full wind load	Storm belts Width direction		14.69 kN (y=1.2)*	178
CO7: Wind suction – closed – full wind load	Storm belts Length direction		17.35 kN (y=1.2)*	180

\* 0 shows the acting forces and the verification of the anchoring for the ground points and stormbelts

#### Anchoring – dense cohesion less soil

<b>Guy ropes</b>	<b>1x</b>	<b>Ø35 x 1200 mm</b>	$F_d / F_{rd} = 7.24 / (1 \times 7.14) = 1.01 \approx 1$	<b>Acceptable</b>
<b>Ground point Corner front</b>	<b>2x</b>	<b>Ø35 x 1200 mm</b>	$F_d / F_{rd} = 14.53 / (2 \times 7.14) = 1.02 \approx 1$	<b>Acceptable</b>
<b>Ground point Corner back</b>	<b>1x</b>	<b>Ø35 x 1200 mm</b>	$F_d / F_{rd} = 6.57 / (1 \times 7.14) = 0.92 < 1$	<b>OK</b>
<b>Ground points Short side</b>	<b>1x</b>	<b>Ø35 x 1200 mm</b>	$F_d / F_{rd} = 5.81 / (1 \times 6.61) = 0.88 < 1$	<b>OK</b>
<b>Ground points Long side</b>	<b>1x</b>	<b>Ø35 x 1200 mm</b>	$F_d / F_{rd} = 4.15 / (1 \times 6.33) = 0.66 < 1$	<b>OK</b>
<b>Storm belts Width direction</b>	<b>2x</b>	<b>Ø35 x 1200 mm</b>	$F_d / F_{rd} = 14.69 / (2 \times 7.14) = 1.03 \approx 1$	<b>Acceptable</b>
<b>Storm belts Length direction</b>	<b>3x</b>	<b>Ø35 x 1200 mm</b>	$F_d / F_{rd} = 17.35 / (3 \times 7.01) = 0.82 < 1$	<b>OK</b>

### H.8.3 Anchor tests according to BS-EN 13782

It is advised to perform anchor test on location when there is a reason to doubt the “pull-out force” of the anchors, which could be when ground conditions differ from dense, non-cohesive soil.

Anchor tests should be carried out according to the following procedure:

Three anchors spread throughout the terrain should be put perpendicular into the ground. The anchors should be pulled out with the aid of a spring balance in the direction of the force acting on the anchor. The lowest of the three measured values should be used.

A safety factor of  $v = 1.6$  regarding ultimate limit load is to apply for the lowest test value in order to determine the load bearing capacity in subsequent calculation. The load bearing capacity determined in this manner shall not result in anchor movement which would result in stresses, deformations or instability inadmissible for the structure.

If the foundation conditions are comparable, test loadings carried out in another location may be adduced for substantiation purposes.

For example:

Force in belts:  $F_{rep} = 16.2 \text{ kN}$

$F_{sd_{belt}} = 1.2 \times F_{rep} = 1.2 \times 16.2 = 19.4 \text{ kN}$

The partial safety factor  $\gamma = 1.6$  is applied on the ultimate limit load:


$Z_{u,d,test} > 1.6 \times F_{sd} = 1.6 \times 19.4 = 31.1 \text{ kN}$

If for example the anchor test point out there has a minimal anchor capacity of 16 kN (1600 kg), then 2 anchors are needed:  $2 \times 16 = 32 \text{ kN} > Z_{u,d,test}$




## I. Material specifications

### Membrane – Contour 3xl FR– Technical data




# CONTOUR 3X AND 3XL

<b>DESCRIPTION</b>	<p>This is a three-ply laminated product that is designed for outdoor stretch tenting. This product exhibits very good stretch and recovery properties. The product is waterproofed by sandwiching a polyurethane membrane between two knitted fabrics. It is not intended to be a permanent structure, as lengthy exposure to UV and hydrolysis, will eventually compromise the membrane.</p> <p>By using a PU membrane instead of a PVC film, the elongation and recoverability is improved. However, the waterproofing performance of a PVC film is superior.</p>				
<b>INTENDED USE</b>	Outdoor Stretch Tenting, Bedouin tents				
<b>BENEFITS</b>	Water/Oil Repellent / UV Resistant / Waterproof / Anti-Fungal				

PROPERTY	UNITS	METHOD	3X	3XL	3XL FR
Composition	-	COA	Polyester/PU	Polyester/PU	Polyester/PU
Construction	-	Visual	Composite	Composite	Composite
Weight	gsm	Finlam	600 ±10%	440 ±10%	500 ±10%
Usable Width	cm	Finlam	170	170	150
Water Penetration	mm	SABS SM 99 (10 Min Hydrastatic Head)	> 2000	> 2000	> 2000
Tensile Strength	Machine Direction (Ave)	BS 13934	> 1300	> 1000	> 1000
	Cross Direction (Ave)		> 1000	> 700	> 700
Average Peelband	Machine Direction (Ave)	SANS 1439	> 10	> 10	> 10
	Cross Direction (Ave)		> 10	> 10	> 10
Anti-Fungal	-	DIN 53931	Pass	Pass	Pass
Colour Fastness to Light	Blue Wool	ISO 105-B02	≥ 6	≥ 6	≥ 6
Oil Repellency	Rating	AATCC 118	≥ 4	≥ 4	≥ 4
Water Repellency	Rating	AATCC 22	≥ 80	≥ 80	≥ 80
Flammability Performance	-	BS 7837:1996	-	-	Pass
	-	California Fire Marshall Title 19	-	-	Pass

This applies to the entire range of CONTOUR® fabrics. The fabric is guaranteed for a period of one year, during which it covers any abnormal deterioration of the waterproofing properties and delamination (when fabric is used for rental usage and not semi-permanent) as well as the general structural integrity of the fabric, given a normal exposure to weather and in the case that care and cleaning advices have been properly followed. The warranty includes the replacement of the fabric required to replace the product in question only, and does not include other expenses such as postage, transportation, sewing, labour and / or assembly. This warranty does not cover damages caused by accidents, mechanical action, rubbing, chemical aggressive agents, abuse and / or negligence. This warranty is applied from the date of the despatch from Finlam Technical, of the product and any claim must be accompanied with the installers receipt and relevant roll number stating that the fabric used was CONTOUR®. \*Please see colour specification chart for colourfastness results, supplied by Finlam Technical (Pty) Ltd. Please also note that darker colours will fade quicker than lighter colours.

Please note that the information shared in this document is for general information only, and it is the responsibility of the customer to ensure that this product meets their specific requirements. The data in this document is for indication only and the product performance properties may vary. This document is not to be treated as a Certificate of Conformance. Finlam Technical will not entertain any damages claimed against them as a result of data expressed in this document.





# CONTOUR 3XFR

<b>DESCRIPTION</b>	This is a three-ply laminated composite product that is designed for outdoor stretch tenting. This product exhibits very good stretch, recovery, UV and FR properties. The product is not intended to be a permanent structure, as lengthy exposure to the elements, will eventually compromise the membrane.
<b>INTENDED USE</b>	Outdoor FR Stretch Tenting, Bedouin tents
<b>BENEFITS</b>	Fire retardant (M2)/Water/Oil Repellent / UV Resistant / Waterproof / Anti-Fungal



PROPERTY	UNITS	METHOD	3X FR
Composition	-	COA	Polyester/PU
Construction	-	Visual	Composite
Weight	gsm	Finlam	820 ±10%
Usable Width	cm	Finlam	150
Tensile Strength	Machine Direction (Ave)	BS 13934+11999	≥ 1000
	Cross Direction (Ave)		≥ 700
Average Peelbond	Machine Direction (Ave)	SANS 1439	≥ 10
	Cross Direction (Ave)		≥ 10
Water Penetration	mm	SABS SM 99 (10 Min Hydrostatic Head)	≥ 1000
Water Repellency	Rating	AATCC 22	≥ 80
Oil Repellency	Rating	AATCC 118	≥ 4
Colour Fastness to Light	Rating	ISO 105-B02	≥ 6
Anti-Fungal	-	DIN EN 14119	Pass
Flammability Performance	-	BS 7837:1996;	Pass
	-	M2	Pass

This applies to the entire range of CONTOUR® fabrics. The fabric is guaranteed for a period of one year, during which it covers any abnormal deterioration of the waterproofing properties and delamination (when fabric is used for rental usage and not semi-permanent) as well as the general structural integrity of the fabric, given a normal exposure to weather and in the case that care and cleaning advice has been properly followed. The warranty includes the replacement of the fabric required to replace the product in question only, and does not include other expenses such as framework, postage, transportation, sewing, labour and / or assembly. This warranty does not cover damages caused by accidents, mechanical action, rubbing, chemical aggressive agents, abuse and / or negligence. This warranty is applied from the date of the despatch from Finlam Textiles, of the product and any claim must be accompanied with the installers receipt and relevant roll number stating that the fabric used was CONTOUR®. \*Please see colour specification chart for colourfastness results, supplied by Finlam Textiles (Pty) Ltd. Please also note that darker colours will fade quicker than lighter colours.

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**PROCES-VERBAL DE CLASSEMENT DE REACTION AU FEU  
D'UN MATERIAU PREVU A L'ARTICLE 5  
DE L'ARRÊTE DU 21 NOVEMBRE 2002**

Valable 5 ans à partir de la date de délivrance

**PROCES-VERBAL N°19-00151 L**

et 1 annexe de 5 pages

**MATERIAU** présenté par : FINLAM TECHNICAL (PTY) LTD  
19 MANCHESTER ROAD  
3610 PINETOWN  
AFRIQUE DU SUD

**MARQUE COMMERCIALE** : CONTOUR 3XL FR

**DESCRIPTION SOMMAIRE** : Complexe fait de 2 tricots polyester contrecollés sur chaque face d'un film polyuréthane avec une colle polyuréthane sur une face et une pâte polyuréthane pour l'autre face  
Masse surfacique mesurée : environ 600 g/m<sup>2</sup>  
Epaisseur nominale : 1.22 mm  
Coloris : marron

**RAPPORT D'ESSAI** : N°19-00151 Et - VI du 4 février 2019

**NATURE DES ESSAIS** : Brûleur électrique  
Essai pour matériaux fusibles

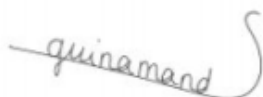
**CLASSEMENT** : **M 2**

**DURABILITE** du classement (Article 5 de l'annexe 2) : non limitée a priori

Compte tenu des critères résultant des essais décrits dans le rapport d'essai annexé.  
Ce document atteste uniquement des caractéristiques de l'échantillon soumis aux essais et ne préjuge pas des caractéristiques de produits similaires. Il ne constitue donc pas une certification de produits au sens de l'article L.433-3 l'Ordonnance n°2016-301 du 14 mars 2016 relative au code de la consommation

NOTA : Sont seules autorisées les reproductions intégrales et par photocopie du présent procès-verbal de classement ou de l'ensemble procès-verbal de classement et rapport d'essai annexé.

A Lyon, le 4 février 2019



Sophie GUINAMAND  
Ingénieur Tests et Essais



Jean-Marc ORAISON  
Responsable Réglementation Feu

**MATERIAL REACTION TO FIRE CLASSIFICATION  
REPORT PREPARED IN COMPLIANCE WITH AMENDED 5 OF THE FRENCH HOME OFFICE  
REGULATION DATED NOVEMBER 21<sup>ST</sup>, 2002 (OFFICIAL GAZETTE DATED DECEMBER 31, 2002)**

Valid five years from issue date

**CERTIFICATE N°19-00151 L**

And appendices of 5 pages

**MATERIAL presented by:** FINLAM TECHNICAL (PTY) LTD  
19 MANCHESTER ROAD  
3610 PINETOWN  
AFRIQUE DU SUD

**TRADE NAME:** CONTOUR 3XL FR

**BRIEF DESCRIPTION:** Fabric made of two polyester knitted fabric laminated on a polyurethane foil with fireproofed polyurethane glue for one side and fireproofed polyurethane paste for other side  
Measured surface weight: about 600 g/m<sup>2</sup>  
Nominal thickness: 1.22 mm  
Colours: brown

**TEST REPORT:** N°19-00151 E1 - V1 on the February 4<sup>th</sup>, 2019

**TESTS:** Electrical burner test  
Dripping test

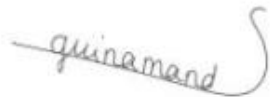
**CLASSIFICATION:** **M 2**

**CLASSIFICATION DURATION (article 5 of appendix 2):** unlimited unless otherwise specified

given the criteria resulting from the tests described in the enclosed test report.  
The classification indicated does not mean that materials marketed comply with the test samples and must not be considered as a qualification certificate as defined by French law dated March 14, 2016.

N.B.: Only integral copies of this document may be made by photocopying the classification report and/or the classification report and enclosed test report.

Issued in Lyon, France, on the February 4<sup>th</sup>, 2019



**Sophie GUINAMAND**  
Engineer Tests and Trials



**Jean-Marc ORAISON**  
In charge of Fire Regulation





CALIFORNIA DEPARTMENT OF FORESTRY and FIRE PROTECTION  
OFFICE OF THE STATE FIRE MARSHAL

## REGISTERED FLAME RESISTANT PRODUCT

Product:

CONTOUR 3XL FR

Registration No.


F-33201

Product Marketed By:

FINLAM TECHNICAL  
19-23 MANCHESTER ROAD  
PINETOWN, 3620, SOUTH AFRICA

This product meets the minimum requirements of flame resistance established by the California State Fire Marshal for products identified in Section 13115, California Health and Safety Code.

The scope of the approved use of this product is provided in the current edition of the **CALIFORNIA APPROVED LIST OF FLAME RETARDANT CHEMICALS AND FABRICS, GENERAL AND LIMITED APPLICATIONS CONCERNS** published by the California State Fire Marshal.

  
Deputy State Fire Marshal

Expire: 6/30/2020

FR-8



**THOR GMBH**

Landwehrstraße 1 · 67346 Speyer · GERMANY

Tel: +49 6232 636-0 · Fax: +49 6232 636-111

E-Mail: [info@thor.com](mailto:info@thor.com)

**TELEFAX**

---

**To:** FINLAM  
**Attn:** Janice Roberts  
**From:** Mrs. Schneider-Müller  
**Date:** 23.11.2020-bib  
**E-Mail:** [jroberts@finlamtechnical.com](mailto:jroberts@finlamtechnical.com)  
**Ref.:** SSM 27462/20  
**Pages:** 1

---

Dear Janice,

The samples were tested. Enclosed please find the results.

<b>NF-P92-503:</b>	<b>Classification</b>
Art. 750 gsm Contour Endurance – Chino – Sample 1	M2
Art. 900 gsm Contour Endurance – Grey – Sample 2	M2
Art. 810 gsm Contour Endurance – Grey – Sample 3	M2

The results in this report are based on the present samples under lab conditions. The results cannot be transferred to other samples without further testing.

For further information, please do not hesitate to contact us again.

Kind regards,  
Thor GmbH

i. V. Susanne Schneider-Müller  
SSM/CM

<b>Reference No.:</b>	SSM 27462/20
<b>Material:</b>	Art. 810 gsm Contour Endurance – Grey
<b>Subject:</b>	Test results

<b>Flammability Test acc. to NF-P92-503:</b>						
<b>(Burning time in sec)</b>						
<b>Sample</b>	<b>3</b>					
<b>Classification M</b>						
20 sec.	0					
45 sec.	6,8					
1 min. 15 sec.	0					
1 min. 45 sec.	0					
2 min. 15 sec.	0					
2 min. 45 sec.	0					
3 min. 15 sec.	0					
3 min. 45 sec.	0					
4 min. 15 sec.	0					
4 min. 45 sec.	0					
Hole formation	Yes					
Dripping / not burning	Yes					
Dripping / burning parts	Yes					
Smoke development (visual)	Very strong					
Burning length (in cm)	12,5					
Burning width (in cm)	5,5					
Test acc. to NFP92-503	M2					
Test acc. to NFP92-504	M2					
Classification NFP92-507	M2					

The results in this report are based on the tests carried out with the present samples under laboratory conditions. The results are not transferrable to other samples without doing further tests.

Membrane – Contour 3xl FR – Biaxial test

UNIVERSITÄT  
DUISBURG  
ESSEN

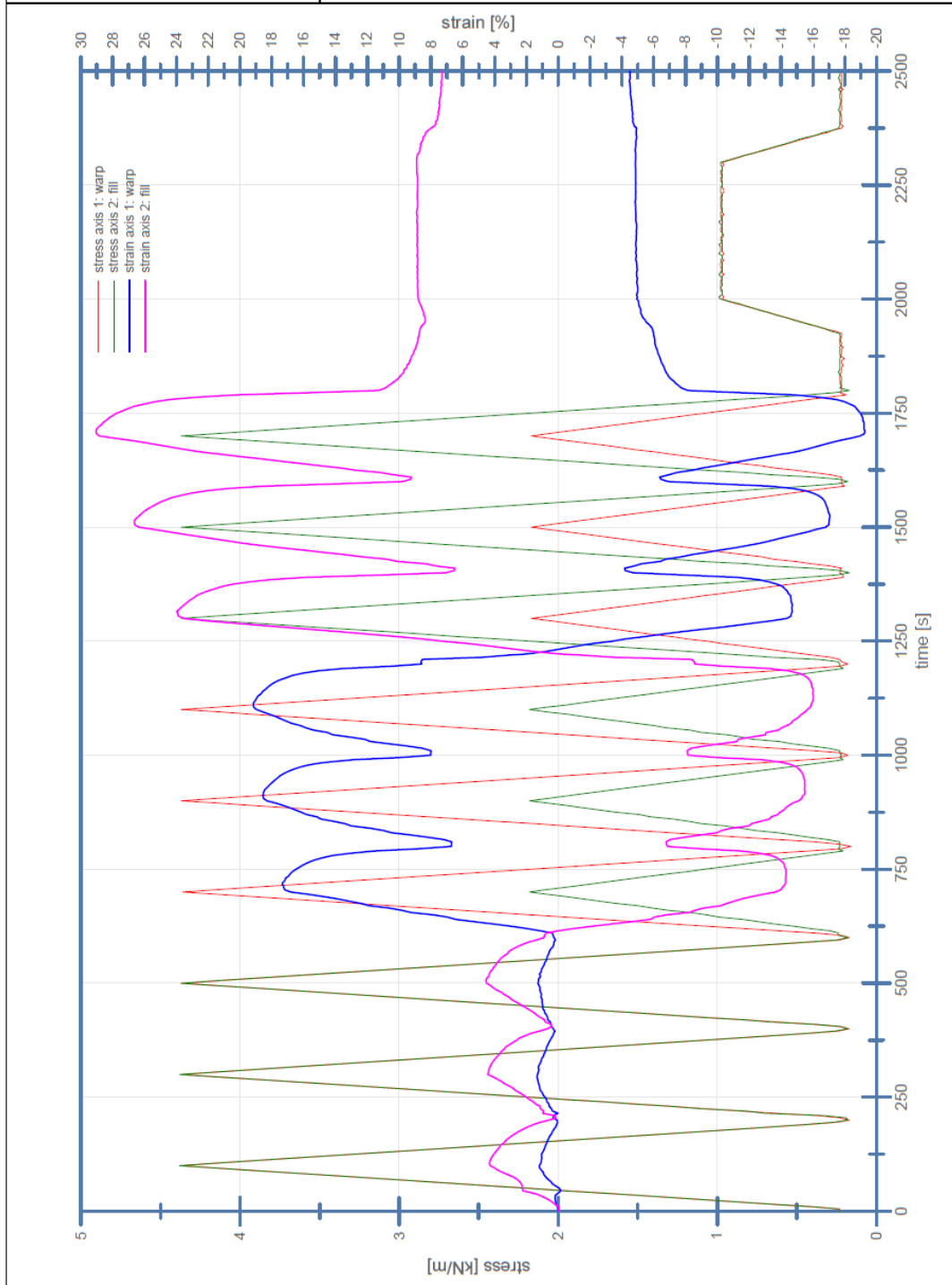
Open-Minded

Fakultät für Ingenieurwissenschaften - Abteilung Bauwissenschaften  
 Institut für Metall- und Leichtbau  
 Essener Labor für Leichte Flächentragwerke - ELLF  
 D-45141 Essen, Universitätsstr. 15, Tel.: +49 201 183-4223, Fax: -4276

**Stress-strain-time diagram to biaxial tensile test**

Test: DTS010618

Material: DTS 3ply fabric



Membrane – Contour X FR – Flex matte coated – Technical data



# DIAMENSIONS

TENTS + STRUCTURES

## Product Technical Data

**Product Reference:** Contour X FR,800gm/mt Stretch Fabric, Vinyl Prim for Semi-Permanent Outdoor Structures

**Product Description:** Coated HT Polyester Knitted Fabric

**Product Characteristics:** Coated high tenacity polyester - Waterproof, Fire Retarded and UV protected. The base knitted fabric is Teflon coated for water and dirt repellence

**End Use:** Permanent and Semi permanent outdoor structures.

**Fabric Supplier ISO compliant - ISO 9001, ISO TS and ISO 14001**

### Technical Data and Product Dimensions:

	Value	Tolerances	Unit	Test Method
Width	152	+/-2	cm	
Fabric Weight Overall Mass	340	0.95	g/m <sup>2</sup>	BS 3424.5 BS 3424.5
	770	+/-5%	g/m <sup>2</sup>	
<b>Product Properties:</b>				
	Value	Tolerances	Unit	Test Method
Color fastness for wet and dry rubbing	4	min	grey scale grey scale	BS 3424 p14method 16
	3	min		A BS 3424 p14 method 16 A
Tensile WMD	950	min.	N/50mm	BS 3424 P4
AMD	750	min.	N/50mm	BS 3424 P4
Elongation under constant load WMD	25	min.	%	10kg/10min.
Elongation under constant load AMD	30	min.	%	10kg/10min.
Bursting resistance	20	min.	kg/sq.cm	Mullen
Fusion of PVC	Good	0	Disintegration	BS 3424.83 P22
Adhesion of Coating	80	min.	N/50mm	BS 3424 P7
Resistance to Flexions	ok	No Cracks	250000Cycles	BS 3424 P9 Annex NA
Loss of volatiles	2.5	max.	%	BS 3424 P12 Method A
Flame Resistance (British)	5	max.	sec	BS 7837 (method BS 54382B)
Electrical Burner Test M2 (French)	20	max	sec	M2
Flame Resistance test ISO 6940	20	max	sec	ISO 6940
Flame Resistance test ISO 6941	20	max	sec	ISO 6941
Coating Pell strength	25	min	N/50mm	BS 3424:9B
Water column	2m		No drops	ASTM D 751

# DIAMENSIONS

TENTS + STRUCTURES

## Contour XFR – OutDoor Stretch PVC Coated Vinyl

- Premium PVC coated stretch material designed for outdoor use
- High Frequency adhesion ("high weld properties")
- Field experience : waterproof in excess of 3 years & exposure 5+ years
- A premium coated product specifically designed for a durable, long lasting stretch tent product
- Fire resistant to British Standards 7837, French M2, CSFM, CPAI 18, ISO 6940, ISO 6941
- Water / dirt repellent: Teflon coated on the reverse base fabric
- Anti peel: strong adhesion between the PVC coat and base fabric
- Premium Elasticity: Stretch, strength, burst and stretch recovery of the pole pressure points

Note:

*Cleaning of the product should be with a mild soapy solution and rinsed thoroughly. The product should not be stored in a wet/damp state for extended periods of time.*

*The use of abrasive chemicals for cleaning purposes as well as cleaning agents containing solvents should be avoided. The product is not recommended for use in temperatures below -10 degrees Celsius.*

*Longevity: When used for the designed purpose, the material should not show brittleness of film, delamination from the fabric or disintegration of the film for a period of 24 months from invoice date. Customers are required to ensure that the product is suitable for the end use application.*



**INSTITUT TECHNOLOGIQUE  
FCBA**

**Fire Laboratory**  
10 rue Galilée - 77420 Champs-sur-Marne

Laboratory approved by the Ministry for the Interior  
JO of February 10, 2007  
**OFFICIAL REPORT OF CLASSIFICATION OF FIRE PERFORMANCE  
OF A MATERIAL ENVISAGED IN ARTICLE 5  
DECREE OF November 21, 2002**

**N° CM-16-P-076**

Page 1

**et 2 annexes de 4 pages.**

Valid 5 years as from mercredi 18 mai 2016

**Material presents by :** **SIOEN FABRICS SA**

Avenue Urbino 6  
7700 MOUSCRON

**Trade mark:** **F5639**

**Summary description**

**Kind of textile** support textile polyester enduit de PVC/ polyester fabric with PVC coating

**Composition** support textile/ fabric 100% polyester (280 g/m<sup>2</sup>) + enduction/ coating PVC ignifugé (450 g/m<sup>2</sup>)

**Mass per unit area** 730 g/m<sup>2</sup>

**Color** tous coloris/ all colors

**Nature of the test :** Electrical burner test  
Flame persistence flame  
Dripping test

**Classification :** **M2**

**Observations :** /

**Durability of the classification (Article 5 de l'annexe 2)** Not limited

**Usual term** Walls hung covering, sheers, curtains,...

**Cleaning conditions** Not washable with water and not cleanable

**Taking into account the criteria resulting from the tests annexed described in the test report N°367160335 et 367160687**

This official report attests only characteristics of the sample submitted for testing and does not prejudice similar characteristics of products. It thus does not constitute a certification of products within the meaning of the L.115-27 article of the code of the consumption and the law of June 3rd, 1994.

NOTE: The integral reproductions and by photocopy of this official report of classification are only authorized on the official report unit of classification and report of annexed test.

**"Not legally acceptable for any use under the pretence of the article AM 18 (Decree of March 6th, 2006)"**

Champs-sur-Marne, le **lundi 28 novembre 2016**

**Technicienne d'essais  
du Laboratoire Feu**

**Clotilde ETIEMVRE**

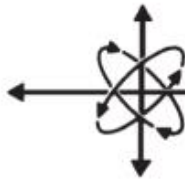
Siège social  
10 rue Galilée  
77420 Champs-sur-Marne  
Tél + 33 (0)1 72 84 97 84  
www.fcba.fr  
Siret 775 680 903 00132  
APE 7219 Z  
Code TVA CEE : FR 14 775 680 903



Only the french version remains valid



Institut technologique FCBA : Forêt, Cellulose, Bois - construction, Ameublement



**DIVERSIFIED**  
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"We Test Per Your Request"

336 WEST FRONT STREET  
 P.O. BOX 4004  
 BURLINGTON, NORTH CAROLINA 27215  
 PHONE (336) 227-7710 • FAX (336) 227-1175  
 www.diversifiedtestinglabs.com

June 24, 2019

Ms. Karen Provost  
 SIOEN FABRICS S.A.  
 Avenue Urbino 6  
 B-7700 Mouscron  
 Belgium

Reference: Flammability Test Report  
 Lab Identification No. 36497  
 Invoice No. 67757

Dear Ms. Provost:

One (1) sample, identified as **RP5639/RP5678**, was received and tested in accordance with the California Administrative Code Title 19--Public Safety, Section 1237. Flame Resistance, Small Scale Test. The results are as follows:

<u>Specimen Number</u>	<u>Test Results</u>			
	<u>After Flame (sec)</u>		<u>Char Length (in)</u>	
	<u>Length</u>	<u>Width</u>	<u>Length</u>	<u>Width</u>
1	0.0	3.0	4.0	4.9
2	13.0	0.0	4.1	3.8
3	0.0	0.0	4.1	3.6
4	0.0	3.0	4.3	3.9
5	<u>0.0</u>	<u>0.0</u>	4.0	4.1
Avg.	2.6	1.2		

The sample submitted, in its original state, **meets** the minimum requirements of the above standard. The char length may not exceed 6.0 inches for any individual specimen and the average afterflame time may not exceed 4.0 seconds in the length or width directions.

If there are any questions or when we can be of further assistance, please let us know.

Sincerely,



Brian S. Dement

BSD/mr



OUR LETTERS AND REPORTS ARE FOR THE EXCLUSIVE USE OF THE CLIENT TO WHOM THEY ARE ADDRESSED. ANY COMMUNICATION TO OTHERS OR THE USE OF OUR COMPANY NAME MUST RECEIVE PRIOR APPROVAL. OUR TEST RESULTS APPLY ONLY TO THE SAMPLE TESTED AND ARE NOT NECESSARILY INDICATIVE OF THE QUALITIES OF APPARENTLY IDENTICAL OR SIMILAR PRODUCTS. SAMPLES NOT DESTROYED IN TESTING ARE RETAINED A MAXIMUM OF THIRTY DAYS. THE LETTERS, REPORTS OR NAME OF DIVERSIFIED TESTING LABORATORIES, INC. MAY NOT BE USED IN ADVERTISING TO THE GENERAL PUBLIC.



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June 24, 2019

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SIOEN FABRICS S.A.  
Avenue Urbino 6  
B-7700 Mouscron  
Belgium

Reference: Flammability Test Report  
Lab Identification No. 36497  
Invoice No. 67757

Dear Ms. Provost:

One (1) sample, identified as **RP5639/RP5678**, was received and tested in accordance with the California Administrative Code Title 19--Public Safety, Section 1237. Flame Resistance, Small Scale Test. **The sample was tested after 72 hours of leaching.** The results are as follows:

<u>Specimen Number</u>	<u>After Flame (sec)</u>		<u>Char Length (in)</u>	
	<u>Length</u>	<u>Width</u>	<u>Length</u>	<u>Width</u>
1	5.0	0.0	3.7	4.2
2	0.0	0.0	3.6	3.4
3	7.0	4.0	5.6	2.7
4	0.0	0.0	4.0	3.3
5	0.0	0.0	4.1	4.0
Avg.	2.4	0.8		

The sample submitted, when tested after 72 hours of leaching, **meets** minimum requirements of the above standard. The char length may not exceed 6.0 inches for any individual specimen and the average afterflame time may not exceed 4.0 seconds in the length or width directions.

If there are any questions or when we can be of further assistance, please let us know.

Sincerely,



Brian S. Dement

BSD/mr



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Avenue Urbino 6  
B-7700 Mouscron  
Belgium

Reference: Flammability Test Report  
Lab Identification No. 36497  
Invoice No. 67757

Dear Ms. Provost:

One (1) sample, identified as **RP5639/RP5678**, was received and tested in accordance with the California Administrative Code Title 19--Public Safety, Section 1237. Flame Resistance, Small Scale Test. **The sample was tested after 100 hours of accelerated weathering.** The results are as follows:

<u>Specimen Number</u>	<u>Test Results</u>			
	<u>After Flame (sec)</u>		<u>Char Length (in)</u>	
	<u>Length</u>	<u>Width</u>	<u>Length</u>	<u>Width</u>
1	0.0	0.0	4.1	4.6
2	0.0	0.0	4.4	4.1
3	0.0	0.0	4.0	3.2
4	0.0	0.0	3.4	3.5
5	<u>0.0</u>	<u>0.0</u>	4.2	3.7
Avg.	0.0	0.0		

The sample submitted, when tested after 100 hours accelerated weathering, **meets** the minimum requirements of the above standard. The char length may not exceed 6.0 inches for any individual specimen and the average afterflame time may not exceed 4.0 seconds in the length or width directions.

If there are any questions or when we can be of further assistance, please let us know.

Sincerely,



Brian S. Dement

BSD/mr



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Testing. Advising. Assuring.

## Test report No. 2018-1124

for applying of a required "Verwendbarkeitsnachweis"  
issued 26.02.2018

**Applicant:** SIOEN FABRICS SA  
Avenue Urbino 6  
  
B – 7700 Mouscron

**Date of order:** 25.01.2018  
**Date of sampling:** *no official sampling of the specimen by a representative of Exova Warringtonfire, Frankfurt*  
**Date of arrival:** 26.01.2018  
**Date of test:** 12.02.2018 + 14.02.2018

### Order

Testing of the flammability (building class B1) according to DIN 4102-1 (May 1998)

### Description / designation of the test object

Product name: RP5639

### Description of the relevant test procedure

DIN 4102 part 1 (Mai 1998)

This test report does not replace the required „Verwendbarkeitsnachweis“. It is only used for issuing the "Verwendbarkeitsnachweis".



## 1. Description of the test material

### 1.1 Details of the customer:

Product name: RP5639

Product description:

280 g/m<sup>2</sup> PES-Textile with 450 g/m<sup>2</sup> PVC coating  
Thickness: approx. 0.90mm  
Colour: taupe

Intended end use of product: Flexible stretch tents.

### 1.2 By Exova Warringtonfire, Frankfurt determined values:

Fabric pattern

Colour: light grey

Thickness: 0,85 mm

Square weight: 750 g/m<sup>2</sup>

Testing after storing 14- days under climatic conditions (23°C / 50 % rel. humidity).



## 2. Test results

### 2.1.1 Brandschachtprüfung according to DIN 4102-1

Sample A: Material tested in production direction  
 Sample B: Material tested crosswise to the production direction  
 Sample C: Material tested crosswise to the production direction  
 Sample D: Material tested crosswise to the production direction

Test results of the Brandschacht tests part 1						
line no.		Measurements test sample				
		A	B	C	D	
1	<u>no. test arrangement according to DIN 4102 part 15, table 1</u>		1	1	1	1
2	<u>flame height max. over lower sample edge</u> time <sup>1)</sup>	cm	70	90	90	90
		min : s	0:18	0:22	0:18	0:21
3	<u>ascertainties on the front side</u> Flaming/glowing time <sup>1)</sup>	min : s	0:03	0:03	0:03	0:08
4	<u>melting / burning through</u> time <sup>1)</sup>	min : s	0:15	0:15	0:16	0:15
5	<u>ascertainties on the back side</u> Flaming/glowing time <sup>1)</sup>	min : s	no	no	no	no
6	discolouring time <sup>1)</sup>	min : s	no	no	no	no
		min : s	no	no	no	no
7	<u>burning droplets</u> begin <sup>1)</sup>	min : s	not occurred	not occurred	not occurred	not occurred
8	extent					
9	occasional dropping of material constant dropping of material					
10	<u>separating from burning sample parts</u> begin <sup>1)</sup>	min : s	no	no	no	no
11	occasional separating parts					
12	constant separating parts					
13	duration of burning on the sieve tray (max.)	min : s	not occurred	not occurred	not occurred	not occurred
14	influence on the burner flame by dropping of / separating material time <sup>1)</sup>	min : s	yes	yes	yes	yes
15	<u>earlier end of test</u> end of the fire scenario on the sample <sup>1)</sup>	min : s	no	no	no	no
16	time of a possible resulted test stop <sup>1)</sup>	min : s				

<sup>1)</sup> time from start of test

Test results of the Brandschacht tests part 2						
line no.		Measurements test sample				
		A	B	C		
17	<u>flaming after end of test</u> duration	min : s	--	--	--	--
18	number of sample	min : s	--	--	--	--
19	front side of sample	min : s	--	--	--	--
20	backside of sample	min : s	--	--	--	--
21	flame length	cm	--	--	--	--
22	<u>glowing after end of test</u> duration	min : s	not occured	not occured	not occured	not occured
23	number of sample	min : s	--	--	--	--
24	place of occurrence	min : s	--	--	--	--
25	lower sample part	min : s	--	--	--	--
26	upper sample part	min : s	--	--	--	--
27	front side of sample	min : s	--	--	--	--
27	backside of sample	min : s	--	--	--	--
28	<u>smoke density</u> < 400 % x min		78	75	68	99
29	> 440 % x min		--	--	--	--
30	diagram in annex no.		1	2	3	4
31	<u>residual length</u> single results	cm	39 / 38 47 / 49	42 / 45 44 / 42	45 / 46 44 / 43	42 / 47 44 / 40
32	average of the single results	cm	43	43	44	43
33	photo of the sample on page	cm	5	5	5	5
34	<u>smoke temperature</u> max. of the average results	°C	124	133	131	123
35	time <sup>1)</sup>	min : s	0:20	0:25	0:22	0:26
36	diagram in annex no.	min : s	1	2	3	4

<sup>1)</sup> time from start of test

Remarks: none

2.1.2 Appearance of the specimen after the test:



Sample A



Sample B



Sample C



Sample D

3/28/2018

2.2.1 Normal flammability test according to DIN 4102-1

Test with edge ignition without deposit  
 Flame application on: lower sample edge  
 Edge ignition

Length direction

Sample-no.					
Time from start of test					
Ignition point [s]	1	1	1	1	1
Reaching the measuring mark within 20 seconds	no	no	no	no	no
Self-extinguishing of the flame [s]	15	15	15	15	15
Max. flame height [mm]	70	70	60	70	70
Time [s]	10	10	10	10	10
End of afterflaming [s]	-	-	-	-	-
End of afterglowing [s]	-	-	-	-	-
Flames extinguished after [s]	-	-	-	-	-
Smoke development (visual impression) <sup>low / moderate / strong</sup>	strong smoke development				
Separating from burning material	no	no	no	no	no
Time [s]	-	-	-	-	-

Remarks: none

Test with edge ignition without deposit  
 Flame application on: lower sample edge  
 Edge ignition

Cross direction

Sample-no.					
Time from start of test					
Ignition point [s]	1	1	1	1	1
Reaching the measuring mark within 20 seconds	no	no	no	no	no
Self-extinguishing of the flame [s]	15	15	15	15	15
Max. flame height [mm]	70	70	60	60	60
Time [s]	10	10	10	10	10
End of afterflaming [s]	-	-	-	-	-
End of afterglowing [s]	-	-	-	-	-
Flames extinguished after [s]	-	-	-	-	-
Smoke development (visual impression) <sup>low / moderate / strong</sup>	strong smoke development				
Separating from burning material	no	no	no	no	no
Time [s]	-	-	-	-	-

Remarks: none

Testing. Advising. Assuring.

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2.2.2 Appearance of the sample after the small burner test:



2/28/2018

### Assessment

The material described in chapter one fulfils the requirements of the building class B2 according to DIN 4102-1 (Mai 1998).

The determined test results show that the material also fulfils the requirements

#### of the building class B1

according to DIN 4102-1 (Mai 1998).

### Special note

The fire test result is only valid for the material described in chapter one in the tested colour and square weight.

The test was carried out in free hanging configuration.

The distance to other plane material must be more or equal then 40 mm.

The material wasn't tested after an outside storage.

In combination with other materials (for example coatings, deposits) the burning behaviour could be influenced unfavourable so that the classification above is not valid any longer. According to DIN 4102-1 the burning behaviour in combination with other materials has to be tested separately.

This test report does not replace the required „Verwendbarkeitsnachweis“. It is only used for issuing the „Verwendbarkeitsnachweis“.

Frankfurt, the 26.02.2018



H. Anders  
Tester in Charge



i. V.  
Dipl.-Ing. T. Zachäus  
Head of the business



This Test report is valid until 11.02.2023.

The results of the tests relate only to the behaviour of the test specimen which is designated on the top.

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This test report is a translation of the German version 2018-1124 (issued 26.02.2018). In case of doubt only the German version is valid

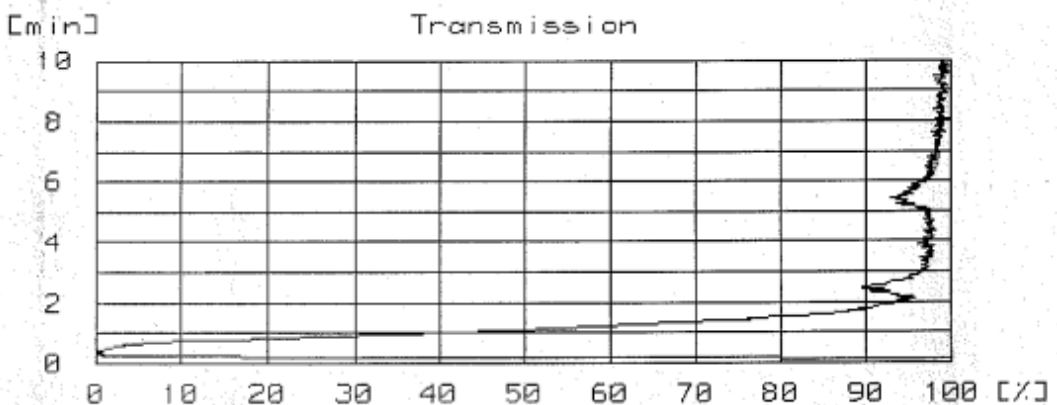
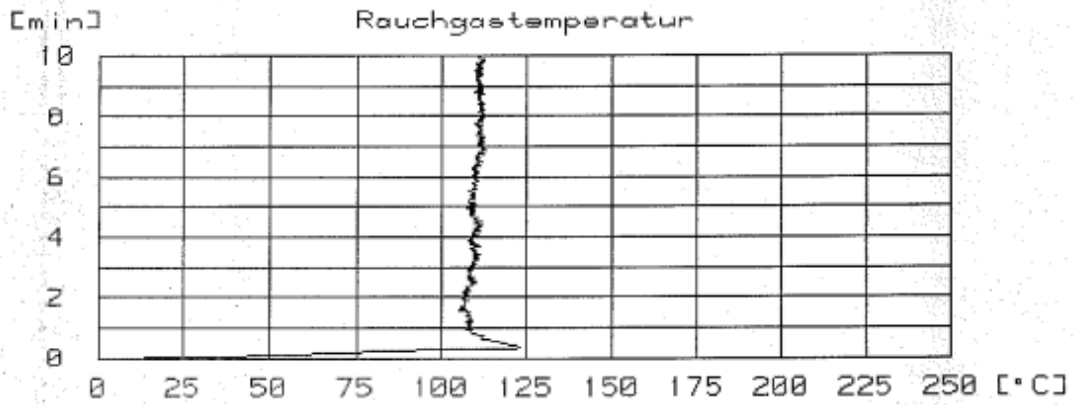
This test report contains 8 pages and 4 annexes.

2/28/2018

Testing. Advising. Assuring.

Annex 1 to the Test report No. 2018-1124 issued 26.02.2018

Sample A:

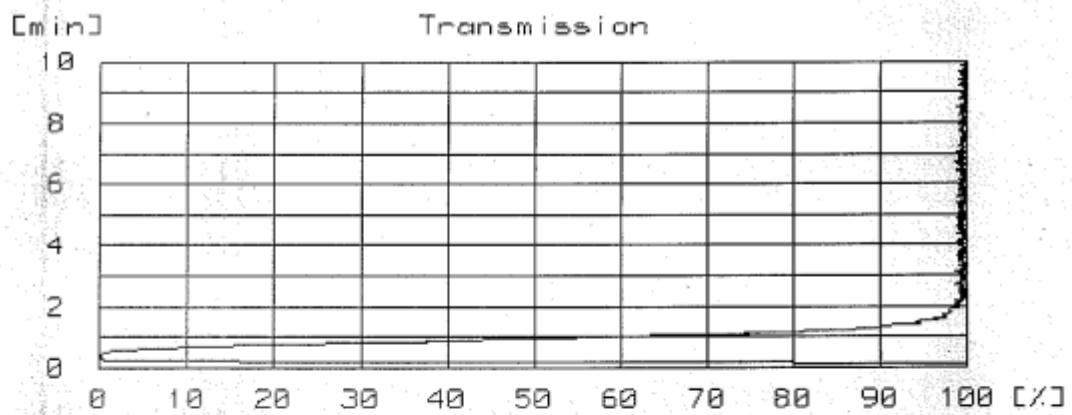
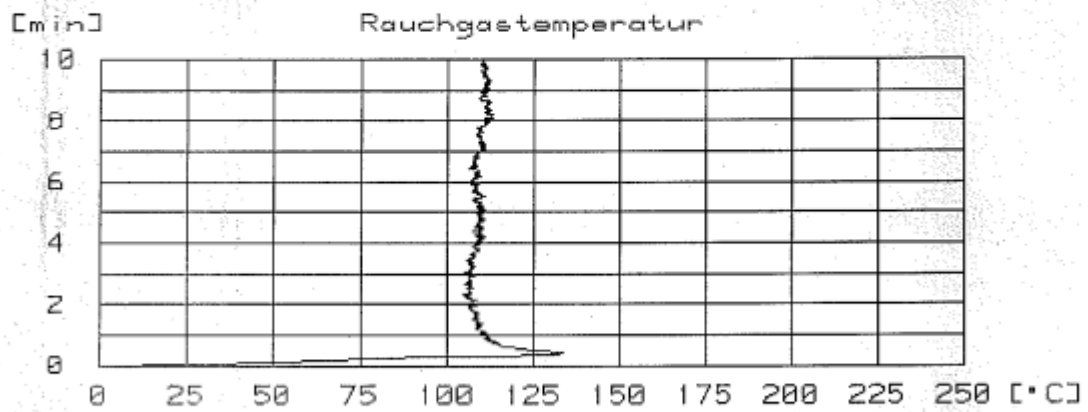




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Annex 2 to the Test report No. 2018-1124 issued 26.02.2018

Sample B:



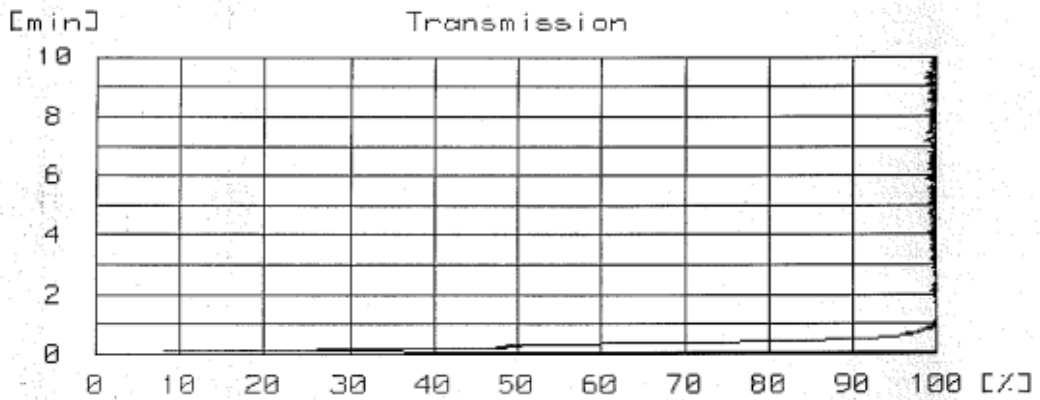
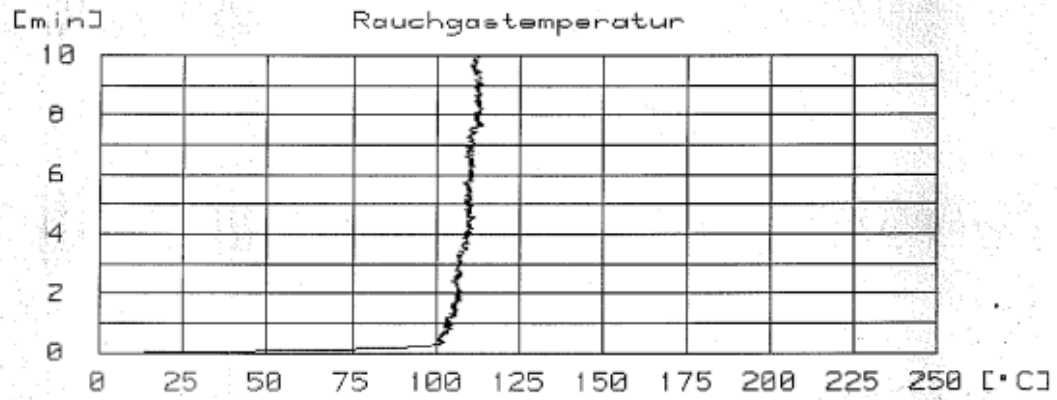
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Annex 2 to the Test report No. 2018-1124 issued 26.02.2018

Sample C:

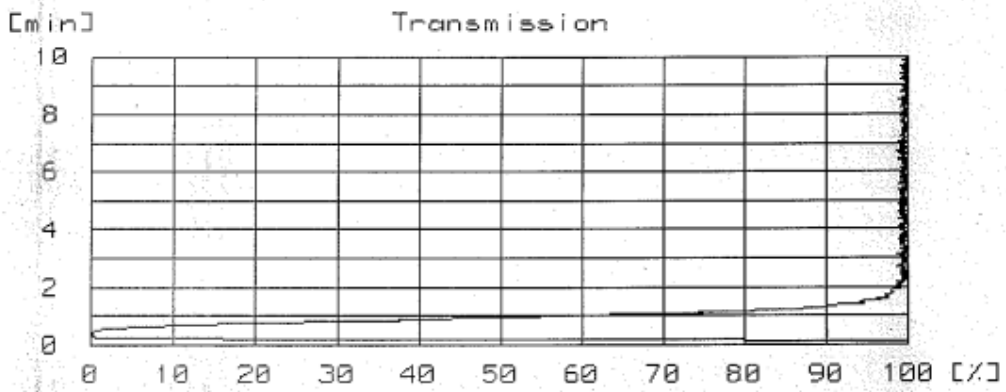
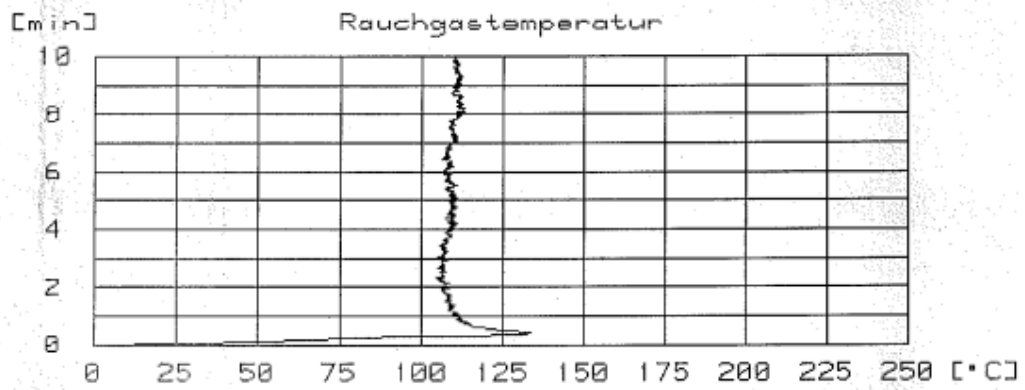


2/28/2018

Testing. Advising. Assuring.

Annex 4 to the Test report No. 2018-1124 issued 26.02.2018

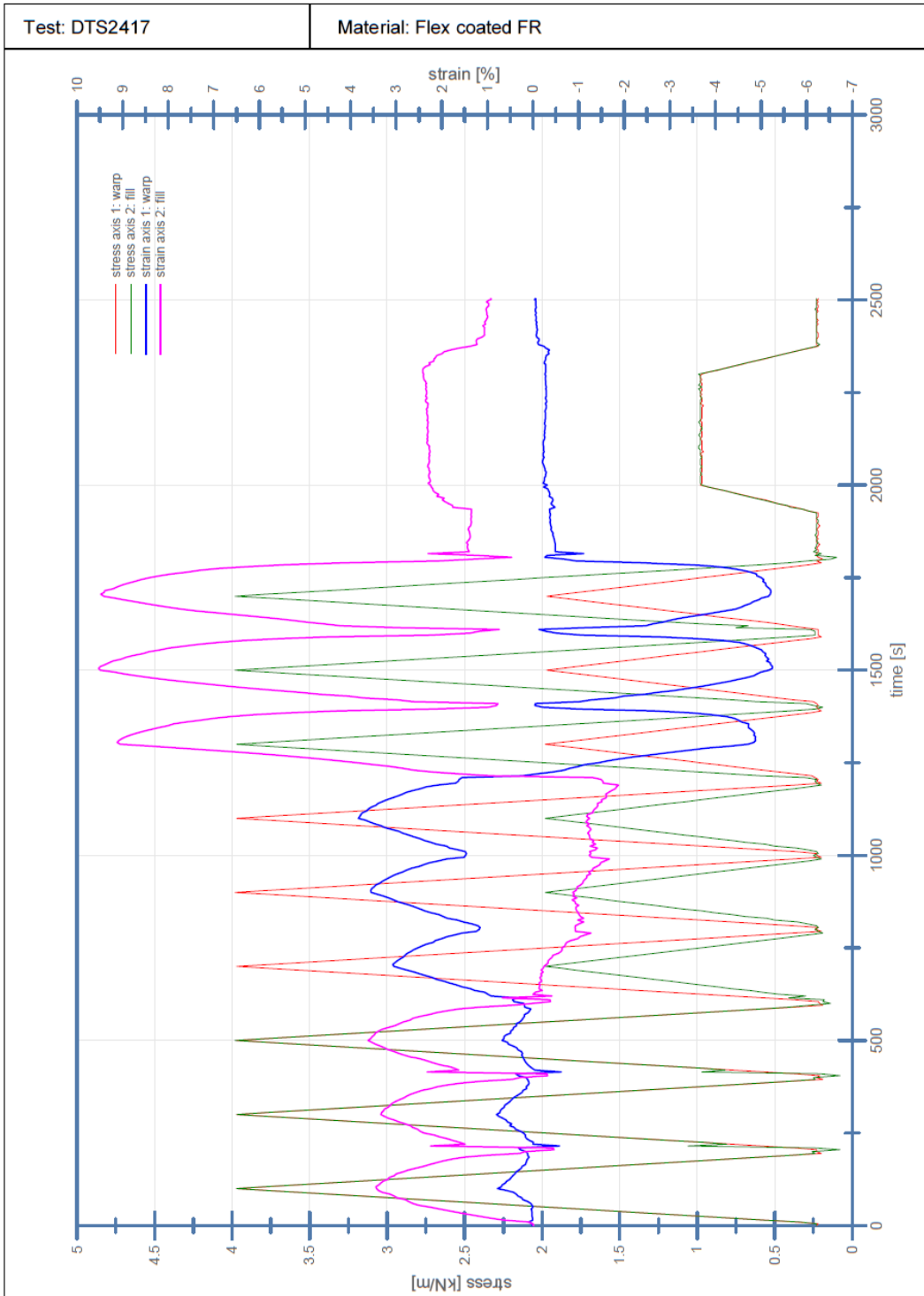
Sample D:



2/28/2018

Membrane – Contour X FR – Flex matte coated – Biaxial test

Stress-strain-time diagram to biaxial tensile test



Material certificate – Ropes 14mm and 7mm



Date: 17/08/2015

DESCRIPTION: Polyester Braid Rope

DIAMETER	14mm
COLOUR	Black
SHEETH COMPOSITION	High tenacity PET
CORE COMPOSITION	Twisted Polyamide
LINEAR DENSITY	130g/m
LOAD AT BREAK	3640kg

DIAMETER	7mm
COLOUR	Black
SHEETH COMPOSITION	High tenacity PET
CORE COMPOSITION	Twisted Polyamide
LINEAR DENSITY	32g/m
LOAD AT BREAK	910kg

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## Annex A. Software input (load cases)

### Annex A.1. 20x15m - floating

#### Annex A.1.1. Own weight + pretension

EXTERNAL LOADS (AUTOMATIC SELFWEIGHT)  
ORDERED BY LOADGROUPS

LOADGROUP	LOADMODE	LOADFACTOR	VOLUME/AREA (P.U)	SUM_X	SUM_Y	SUM_Z	VOLUME/AREA
STRUTS	SELFWEIGHT	1.00	78.500000	0.0000	0.0000	-1.0099	0.012865
CABLES	SELFWEIGHT	1.00	78.500000	0.0000	0.0000	-0.0727	0.000926
MEM-LINKS1000	SELFWEIGHT	1.00	0.003750	0.0000	0.0000	-1.0973	292.624216
MEM-LINKS2000	SELFWEIGHT	1.00	0.003750	0.0000	0.0000	-1.0972	292.592308

EXTERNAL LOADS (AREA-DEPENDENT)  
ORDERED BY LOADGROUPS

LOADGROUP	LOADMODE	LOAD	FACTOR	SUM_X	SUM_Y	SUM_Z	LOADED AREA
SUM				0.0000	0.0000	0.0000	

EXTERNAL LOADS (AREA-DEPENDENT)  
ORDERED BY LOADMODES

LOADMODE	SUM_X	SUM_Y	SUM_Z
SUM AREA-LOADS	0.0000	0.0000	0.0000

EXTERNAL LOADS: SUM OF ALL EXTERNAL LOADS

SUM_X	SUM_Y	SUM_Z
0.0000	0.0000	-3.2771

### Annex A.1.2. Own weight + pretension + conventional / snow

EXTERNAL LOADS (AUTOMATIC SELFWEIGHT)  
ORDERED BY LOADGROUPS

LOADGROUP	LOADMODE	LOADFACTOR	VOLUME/AREA (P.U)	SUM_X	SUM_Y	SUM_Z	VOLUME/AREA
STRUTS	SELFWEIGHT	1.00	78.500000	0.0000	0.0000	-1.0099	0.012865
CABLES	SELFWEIGHT	1.00	78.500000	0.0000	0.0000	-0.0727	0.000926
MEM-LINKS1000	SELFWEIGHT	1.00	0.003750	0.0000	0.0000	-1.0973	292.624216
MEM-LINKS2000	SELFWEIGHT	1.00	0.003750	0.0000	0.0000	-1.0972	292.592308

EXTERNAL LOADS (AREA-DEPENDENT)  
ORDERED BY LOADGROUPS

LOADGROUP	LOADMODE	LOAD	FACTOR	SUM_X	SUM_Y	SUM_Z	LOADED AREA
1	SCHNEE	0.1000	1.00	0.0000	0.0000	-27.3639	273.64
SUM				0.0000	0.0000	-27.3639	

EXTERNAL LOADS (AREA-DEPENDENT)  
ORDERED BY LOADMODES

LOADMODE	SUM_X	SUM_Y	SUM_Z
SUM SCHNEE	0.0000	0.0000	-27.3639
SUM AREA-LOADS	0.0000	0.0000	-27.3639

EXTERNAL LOADS: SUM OF ALL EXTERNAL LOADS

SUM_X	SUM_Y	SUM_Z
0.0000	0.0000	-30.6410

### Annex A.1.3. Own weight + pretension + wind pressure

EXTERNAL LOADS (AUTOMATIC SELFWEIGHT)  
ORDERED BY LOADGROUPS

LOADGROUP	LOADMODE	LOADFACTOR	VOLUME/AREA (P.U)	SUM_X	SUM_Y	SUM_Z	VOLUME/AREA
STRUTS	SELFWEIGHT	1.00	78.500000	0.0000	0.0000	-1.0099	0.012865
CABLES	SELFWEIGHT	1.00	78.500000	0.0000	0.0000	-0.0727	0.000926
MEM-LINKS1000	SELFWEIGHT	1.00	0.003750	0.0000	0.0000	-1.0973	292.624216
MEM-LINKS2000	SELFWEIGHT	1.00	0.003750	0.0000	0.0000	-1.0972	292.592308

EXTERNAL LOADS (AREA-DEPENDENT)  
ORDERED BY LOADGROUPS

LOADGROUP	LOADMODE	LOAD	FACTOR	SUM_X	SUM_Y	SUM_Z	LOADED AREA
1	WIND	-0.1500	1.00	0.0000	0.0000	-41.0458	292.80
SUM				0.0000	0.0000	-41.0458	

EXTERNAL LOADS (AREA-DEPENDENT)  
ORDERED BY LOADMODES

LOADMODE	SUM_X	SUM_Y	SUM_Z
SUM WIND	0.0000	0.0000	-41.0458
SUM AREA-LOADS	0.0000	0.0000	-41.0458

EXTERNAL LOADS: SUM OF ALL EXTERNAL LOADS

SUM_X	SUM_Y	SUM_Z
0.0000	0.0000	-44.3230



### Annex A.1.4. Own weight + pretension + wind suction – floating – reduction 0.53

EXTERNAL LOADS (AUTOMATIC SELFWEIGHT)  
ORDERED BY LOADGROUPS

LOADGROUP	LOADMODE	LOADFACTOR	VOLUME/AREA (P.U)	SUM_X	SUM_Y	SUM_Z	VOLUME/AREA
STRUTS	SELFWEIGHT	1.00	78.500000	0.0000	0.0000	-1.0099	0.012865
CABLES	SELFWEIGHT	1.00	78.500000	0.0000	0.0000	-0.0727	0.000926
MEM-LINKS1000	SELFWEIGHT	1.00	0.003750	0.0000	0.0000	-1.0973	292.624216
MEM-LINKS2000	SELFWEIGHT	1.00	0.003750	0.0000	0.0000	-1.0972	292.592308

EXTERNAL LOADS (AREA-DEPENDENT)  
ORDERED BY LOADGROUPS

LOADGROUP	LOADMODE	LOAD	FACTOR	SUM_X	SUM_Y	SUM_Z	LOADED AREA
1	WIND	0.3500	0.53	0.0000	0.0000	50.7600	292.80
SUM				0.0000	0.0000	50.7600	

EXTERNAL LOADS (AREA-DEPENDENT)  
ORDERED BY LOADMODES

LOADMODE	SUM_X	SUM_Y	SUM_Z
SUM WIND	0.0000	0.0000	50.7600
SUM AREA-LOADS	0.0000	0.0000	50.7600

EXTERNAL LOADS: SUM OF ALL EXTERNAL LOADS

SUM_X	SUM_Y	SUM_Z
0.0000	0.0000	47.4829

### Annex A.1.5. Own weight + pretension + wind suction – floating – full wind load

EXTERNAL LOADS (AUTOMATIC SELFWEIGHT)  
ORDERED BY LOADGROUPS

LOADGROUP	LOADMODE	LOADFACTOR	VOLUME/AREA (P.U)	SUM_X	SUM_Y	SUM_Z	VOLUME/AREA
STRUTS	SELFWEIGHT	1.00	78.500000	0.0000	0.0000	-1.0099	0.012865
CABLES	SELFWEIGHT	1.00	78.500000	0.0000	0.0000	-0.0897	0.001142
MEM-LINKS1000	SELFWEIGHT	1.00	0.003750	0.0000	0.0000	-1.0974	292.628759
MEM-LINKS2000	SELFWEIGHT	1.00	0.003750	0.0000	0.0000	-1.0971	292.564733

EXTERNAL LOADS (AREA-DEPENDENT)  
ORDERED BY LOADGROUPS

LOADGROUP	LOADMODE	LOAD	FACTOR	SUM_X	SUM_Y	SUM_Z	LOADED AREA
1	WIND	0.3500	1.00	0.0006	-0.0003	95.7406	292.75
SUM				0.0006	-0.0003	95.7406	

EXTERNAL LOADS (AREA-DEPENDENT)  
ORDERED BY LOADMODES

LOADMODE	SUM_X	SUM_Y	SUM_Z
SUM WIND	0.0006	-0.0003	95.7406
SUM AREA-LOADS	0.0006	-0.0003	95.7406

EXTERNAL LOADS: SUM OF ALL EXTERNAL LOADS

SUM_X	SUM_Y	SUM_Z
0.0006	-0.0003	92.4466

## Annex A.2. 20x15m - closed

### Annex A.2.1. Own weight + pretension

EXTERNAL LOADS (AUTOMATIC SELFWEIGHT)  
ORDERED BY LOADGROUPS

LOADGROUP	LOADMODE	LOADFACTOR	VOLUME/AREA (P.U)	SUM_X	SUM_Y	SUM_Z	VOLUME/AREA
STRUTS	SELFWEIGHT	1.00	78.500000	0.0000	0.0000	-1.1430	0.014561
CABLES	SELFWEIGHT	1.00	78.500000	0.0000	0.0000	-0.0848	0.001081
MEM-LINKS1000	SELFWEIGHT	1.00	0.003750	0.0000	0.0000	-1.2874	343.302092
MEM-LINKS2000	SELFWEIGHT	1.00	0.003750	0.0000	0.0000	-1.2873	343.267288

EXTERNAL LOADS (AREA-DEPENDENT)  
ORDERED BY LOADGROUPS

LOADGROUP	LOADMODE	LOAD	FACTOR	SUM_X	SUM_Y	SUM_Z	LOADED AREA
SUM				0.0000	0.0000	0.0000	

EXTERNAL LOADS (AREA-DEPENDENT)  
ORDERED BY LOADMODES

LOADMODE	SUM_X	SUM_Y	SUM_Z
SUM AREA-LOADS	0.0000	0.0000	0.0000

EXTERNAL LOADS: SUM OF ALL EXTERNAL LOADS

SUM_X	SUM_Y	SUM_Z
0.0000	0.0000	-3.8025

## Annex A.2.2. Own weight + pretension + conventional / snow

EXTERNAL LOADS (AUTOMATIC SELFWEIGHT)  
ORDERED BY LOADGROUPS

LOADGROUP	LOADMODE	LOADFACTOR	VOLUME/AREA (P.U)	SUM_X	SUM_Y	SUM_Z	VOLUME/AREA
STRUTS	SELFWEIGHT	1.00	78.500000	0.0000	0.0000	-1.1430	0.014561
CABLES	SELFWEIGHT	1.00	78.500000	0.0000	0.0000	-0.0848	0.001081
MEM-LINKS1000	SELFWEIGHT	1.00	0.003750	0.0000	0.0000	-1.2874	343.302092
MEM-LINKS2000	SELFWEIGHT	1.00	0.003750	0.0000	0.0000	-1.2873	343.267288

EXTERNAL LOADS (AREA-DEPENDENT)  
ORDERED BY LOADGROUPS

LOADGROUP	LOADMODE	LOAD	FACTOR	SUM_X	SUM_Y	SUM_Z	LOADED AREA
1	SCHNEE	0.1000	1.00	0.0000	0.0000	-28.7573	287.57
SUM				0.0000	0.0000	-28.7573	

EXTERNAL LOADS (AREA-DEPENDENT)  
ORDERED BY LOADMODES

LOADMODE	SUM_X	SUM_Y	SUM_Z
SUM SCHNEE	0.0000	0.0000	-28.7573
SUM AREA-LOADS	0.0000	0.0000	-28.7573

EXTERNAL LOADS: SUM OF ALL EXTERNAL LOADS

SUM_X	SUM_Y	SUM_Z
0.0000	0.0000	-32.5599

### Annex A.2.3. Own weight + pretension + wind pressure

EXTERNAL LOADS (AUTOMATIC SELFWEIGHT)  
ORDERED BY LOADGROUPS

LOADGROUP	LOADMODE	LOADFACTOR	VOLUME/AREA (P.U)	SUM_X	SUM_Y	SUM_Z	VOLUME/AREA
STRUTS	SELFWEIGHT	1.00	78.500000	0.0000	0.0000	-1.1430	0.014561
CABLES	SELFWEIGHT	1.00	78.500000	0.0000	0.0000	-0.0848	0.001081
MEM-LINKS1000	SELFWEIGHT	1.00	0.003750	0.0000	0.0000	-1.2874	343.302092
MEM-LINKS2000	SELFWEIGHT	1.00	0.003750	0.0000	0.0000	-1.2873	343.267288

EXTERNAL LOADS (AREA-DEPENDENT)  
ORDERED BY LOADGROUPS

LOADGROUP	LOADMODE	LOAD	FACTOR	SUM_X	SUM_Y	SUM_Z	LOADED AREA
1	WIND	-0.1500	1.00	0.0000	-6.2523	-43.1360	344.27
SUM				0.0000	-6.2523	-43.1360	

EXTERNAL LOADS (AREA-DEPENDENT)  
ORDERED BY LOADMODES

LOADMODE	SUM_X	SUM_Y	SUM_Z
SUM WIND	0.0000	-6.2523	-43.1360
SUM AREA-LOADS	0.0000	-6.2523	-43.1360

EXTERNAL LOADS: SUM OF ALL EXTERNAL LOADS

SUM_X	SUM_Y	SUM_Z
0.0000	-6.2523	-46.9385

**Annex A.2.4. Own weight + pretension + wind suction – closed – reduction 0.53**

EXTERNAL LOADS (AUTOMATIC SELFWEIGHT)  
ORDERED BY LOADGROUPS

LOADGROUP	LOADMODE	LOADFACTOR	VOLUME/AREA (P.U)	SUM_X	SUM_Y	SUM_Z	VOLUME/AREA
STRUTS	SELFWEIGHT	1.00	78.500000	0.0000	0.0000	-1.1430	0.014561
CABLES	SELFWEIGHT	1.00	78.500000	0.0000	0.0000	-0.0848	0.001081
MEM-LINKS1000	SELFWEIGHT	1.00	0.003750	0.0000	0.0000	-1.2874	343.302092
MEM-LINKS2000	SELFWEIGHT	1.00	0.003750	0.0000	0.0000	-1.2873	343.267288

EXTERNAL LOADS (AREA-DEPENDENT)  
ORDERED BY LOADGROUPS

LOADGROUP	LOADMODE	LOAD	FACTOR	SUM_X	SUM_Y	SUM_Z	LOADED AREA
1	WIND	0.6500	0.53	-0.0001	14.3595	99.0691	344.27
SUM				-0.0001	14.3595	99.0691	

EXTERNAL LOADS (AREA-DEPENDENT)  
ORDERED BY LOADMODES

LOADMODE	SUM_X	SUM_Y	SUM_Z
SUM WIND	-0.0001	14.3595	99.0691
SUM AREA-LOADS	-0.0001	14.3595	99.0691

EXTERNAL LOADS: SUM OF ALL EXTERNAL LOADS

SUM_X	SUM_Y	SUM_Z
-0.0001	14.3595	95.2665

### Annex A.2.5. Own weight + pretension + wind suction – closed – full wind load

EXTERNAL LOADS (AUTOMATIC SELFWEIGHT)  
ORDERED BY LOADGROUPS

LOADGROUP	LOADMODE	LOADFACTOR	VOLUME/AREA (P.U)	SUM_X	SUM_Y	SUM_Z	VOLUME/AREA
STRUTS	SELFWEIGHT	1.00	78.500000	0.0000	0.0000	-1.1430	0.014561
CABLES	SELFWEIGHT	1.00	78.500000	0.0000	0.0000	-0.1468	0.001870
MEM-LINKS1000	SELFWEIGHT	1.00	0.003750	0.0000	0.0000	-1.2873	343.287623
MEM-LINKS2000	SELFWEIGHT	1.00	0.003750	0.0000	0.0000	-1.2873	343.271683

EXTERNAL LOADS (AREA-DEPENDENT)  
ORDERED BY LOADGROUPS

LOADGROUP	LOADMODE	LOAD	FACTOR	SUM_X	SUM_Y	SUM_Z	LOADED AREA
1	WIND	0.6500	1.00	-0.0003	27.0704	186.8839	344.18
SUM				-0.0003	27.0704	186.8839	

EXTERNAL LOADS (AREA-DEPENDENT)  
ORDERED BY LOADMODES

LOADMODE	SUM_X	SUM_Y	SUM_Z
SUM WIND	-0.0003	27.0704	186.8839
SUM AREA-LOADS	-0.0003	27.0704	186.8839

EXTERNAL LOADS: SUM OF ALL EXTERNAL LOADS

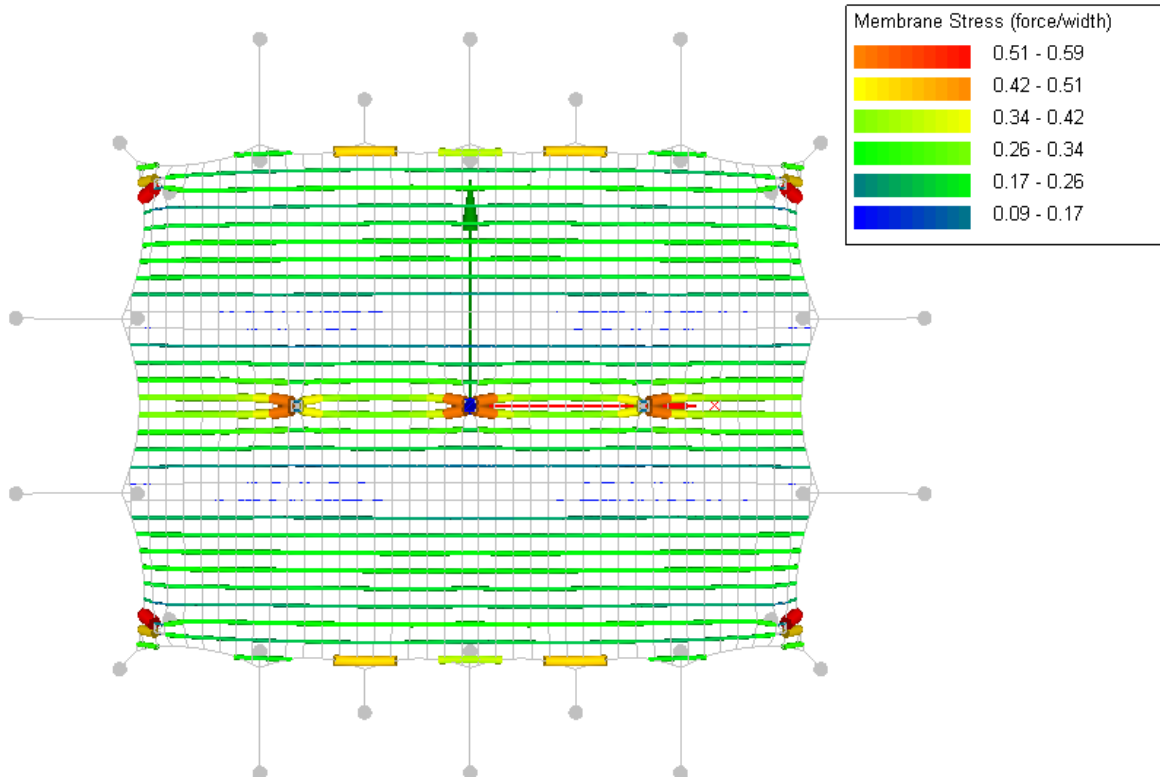
SUM_X	SUM_Y	SUM_Z
-0.0003	27.0704	183.0195

**Annex B. Results per load combination**

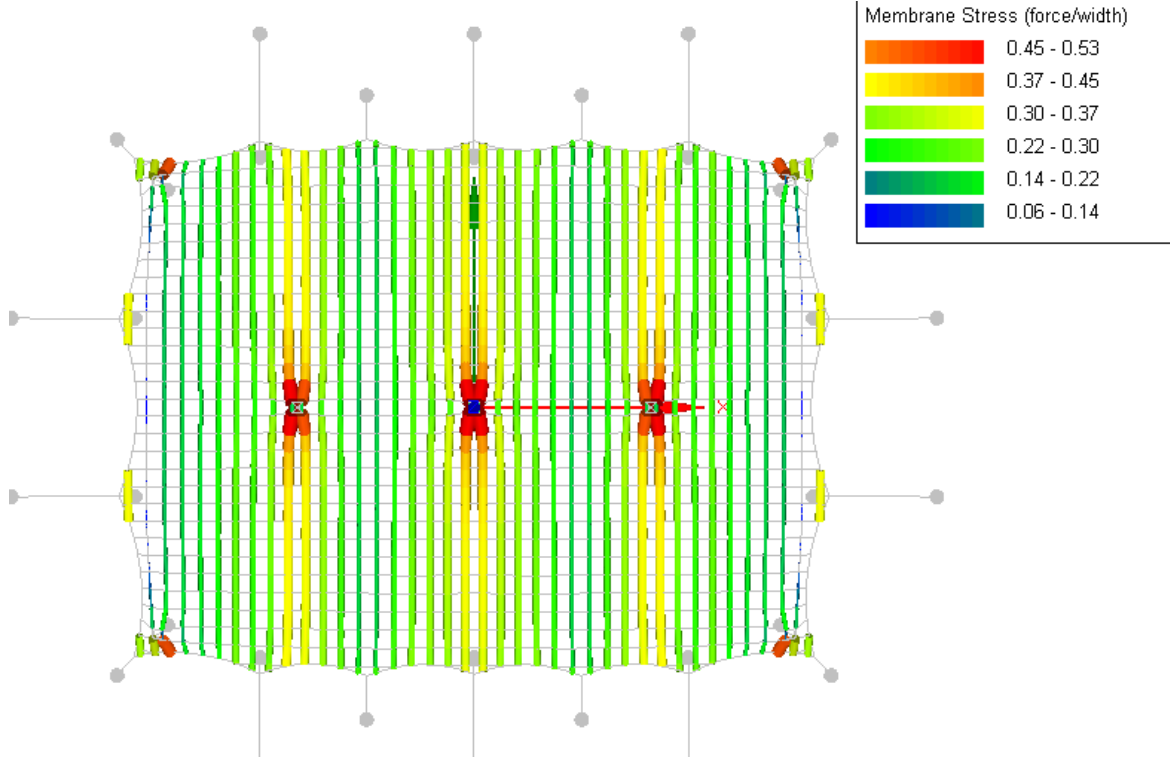
**Annex B.1. 20x15m - floating**

**Annex B.1.1. CO1 Own weight + Pretension**

**Annex B.1.1.1. Membrane stress (warp)**

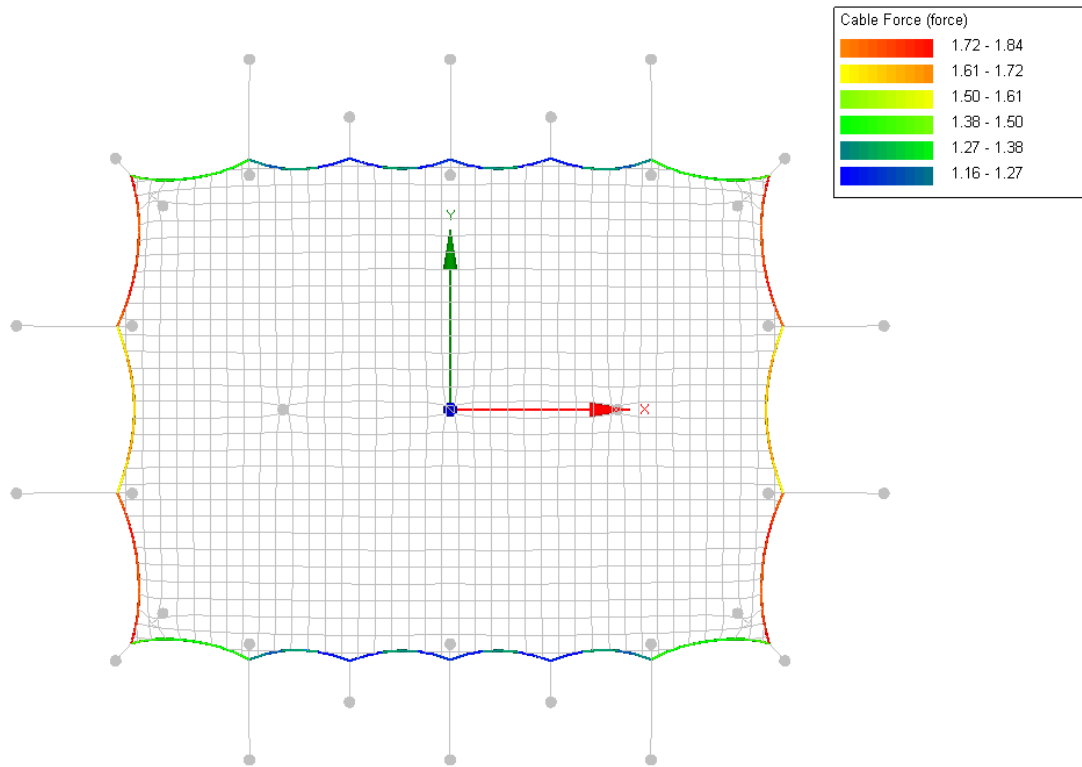


**Annex B.1.1.2. Membrane stress (weft)**

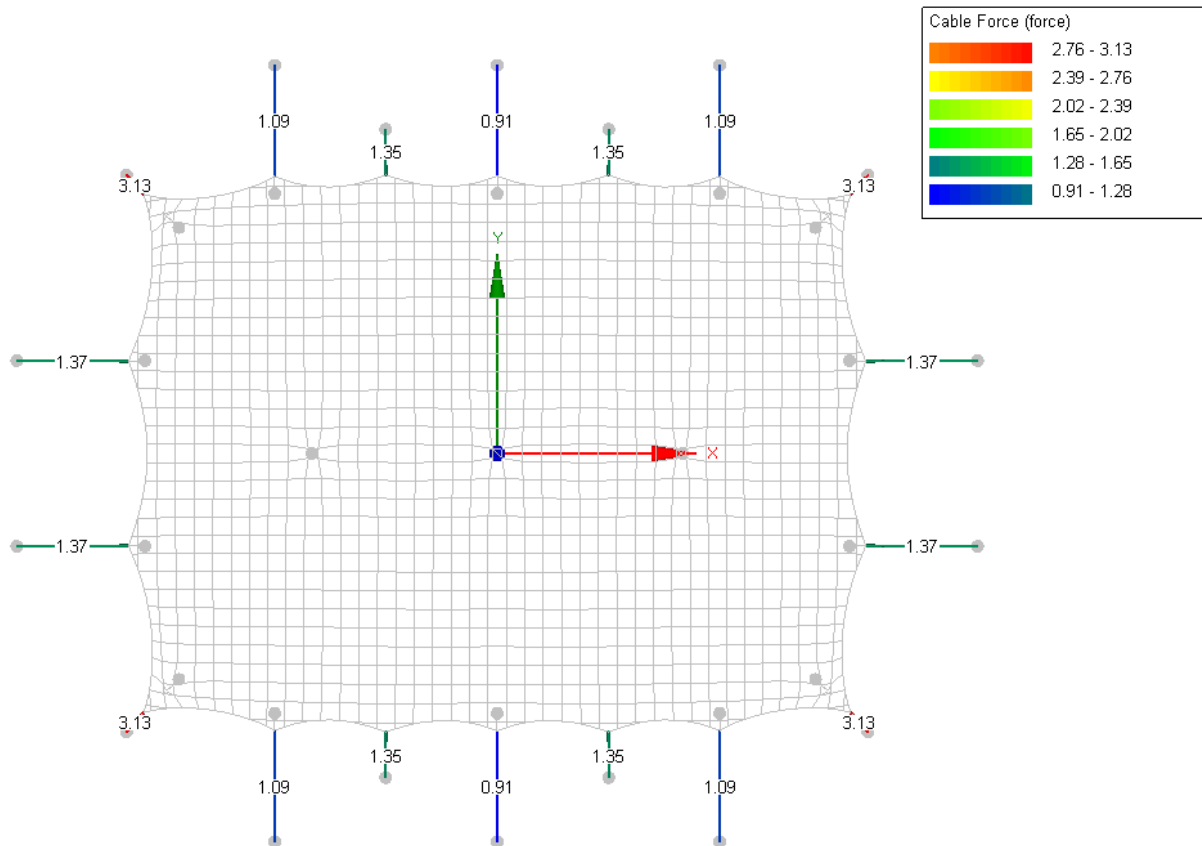




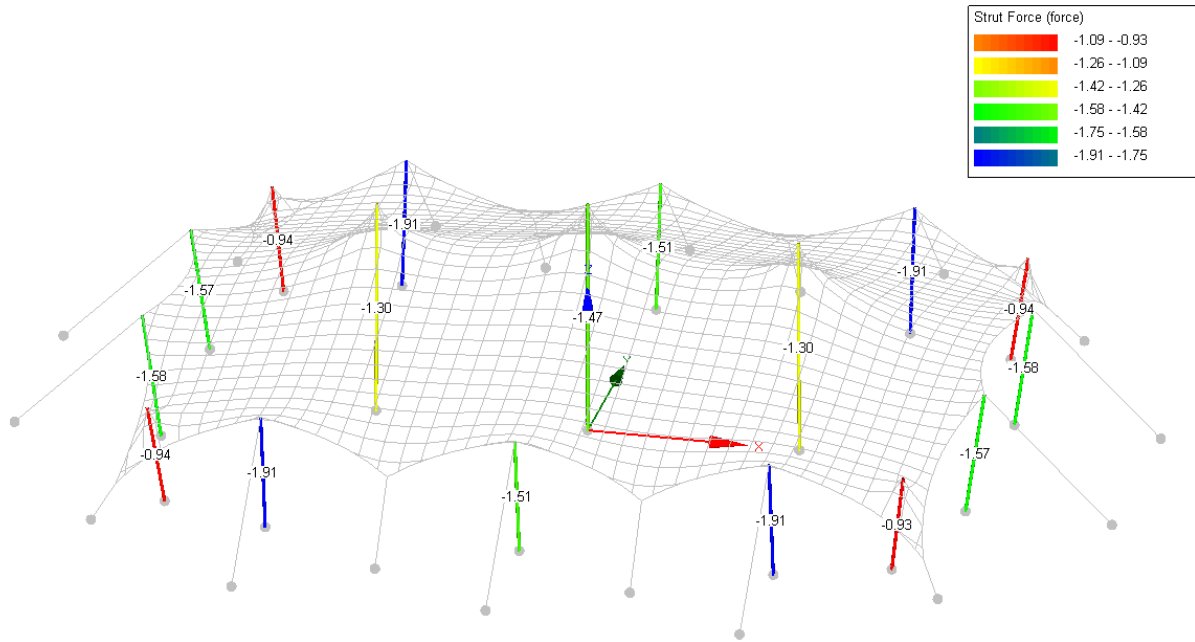
Annex B.1.1.3. Membrane edge



Annex B.1.1.4. Cable forces

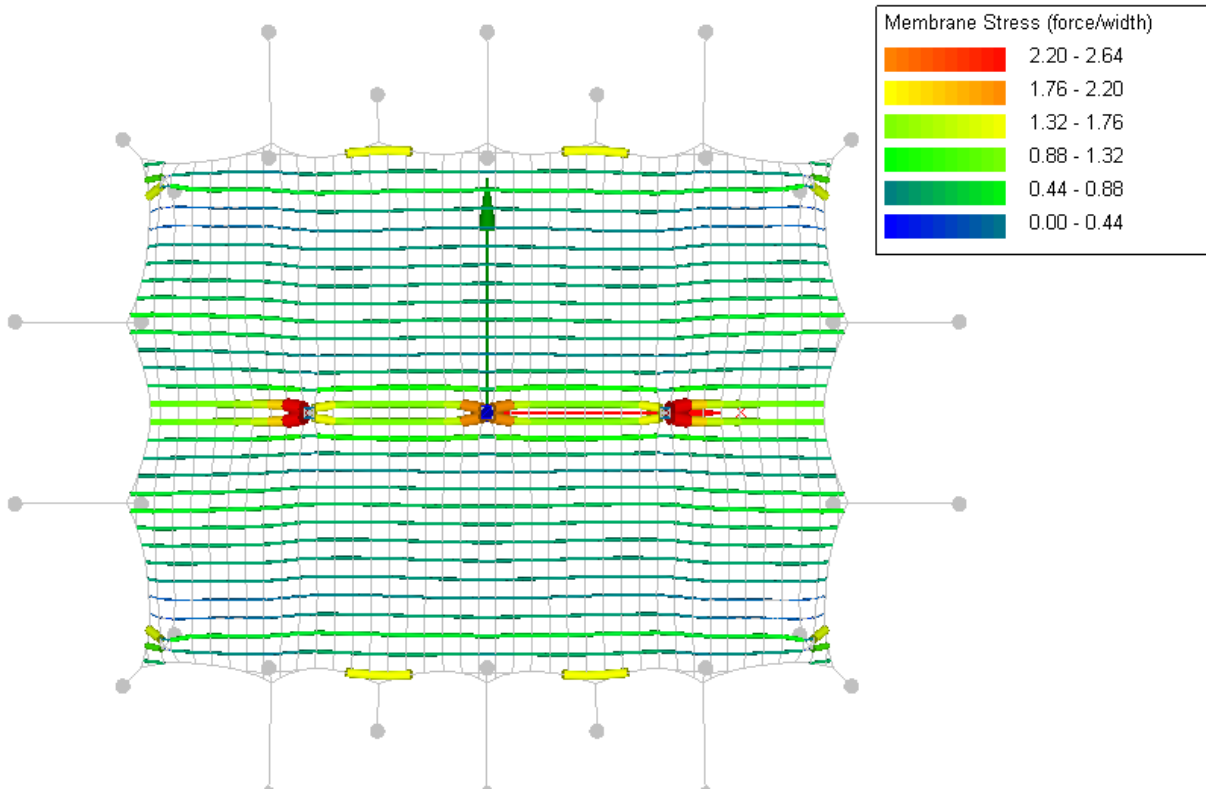


Annex B.1.1.5. Strut forces

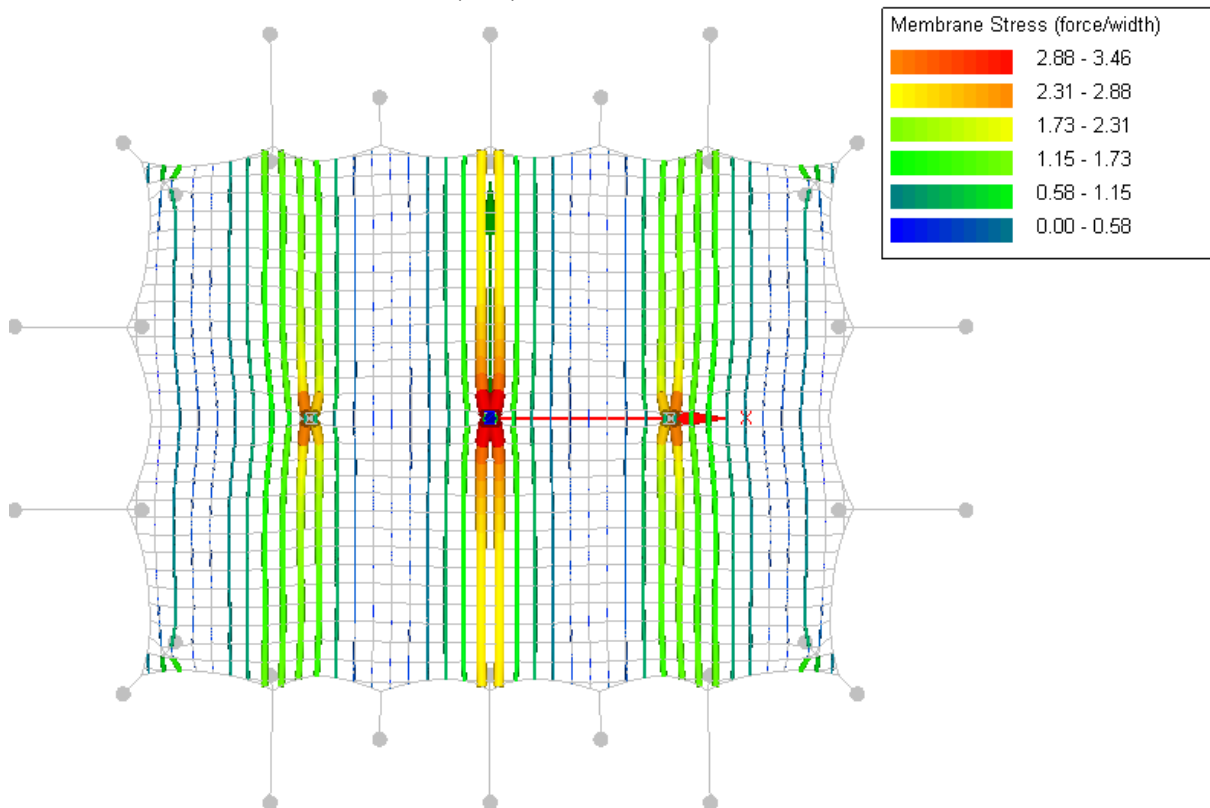


**Annex B.1.2. CO2 Own weight + Pretension + Conventional / snow**

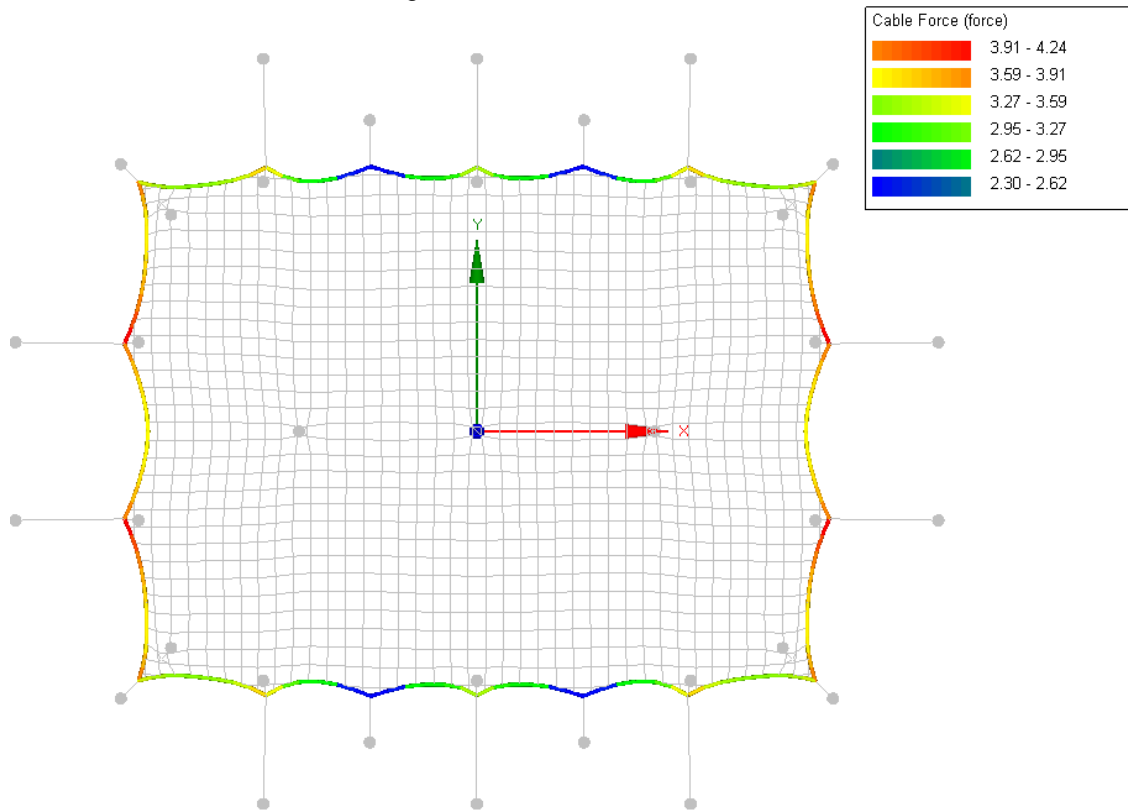
**Annex B.1.2.1. Membrane stress (warp)**



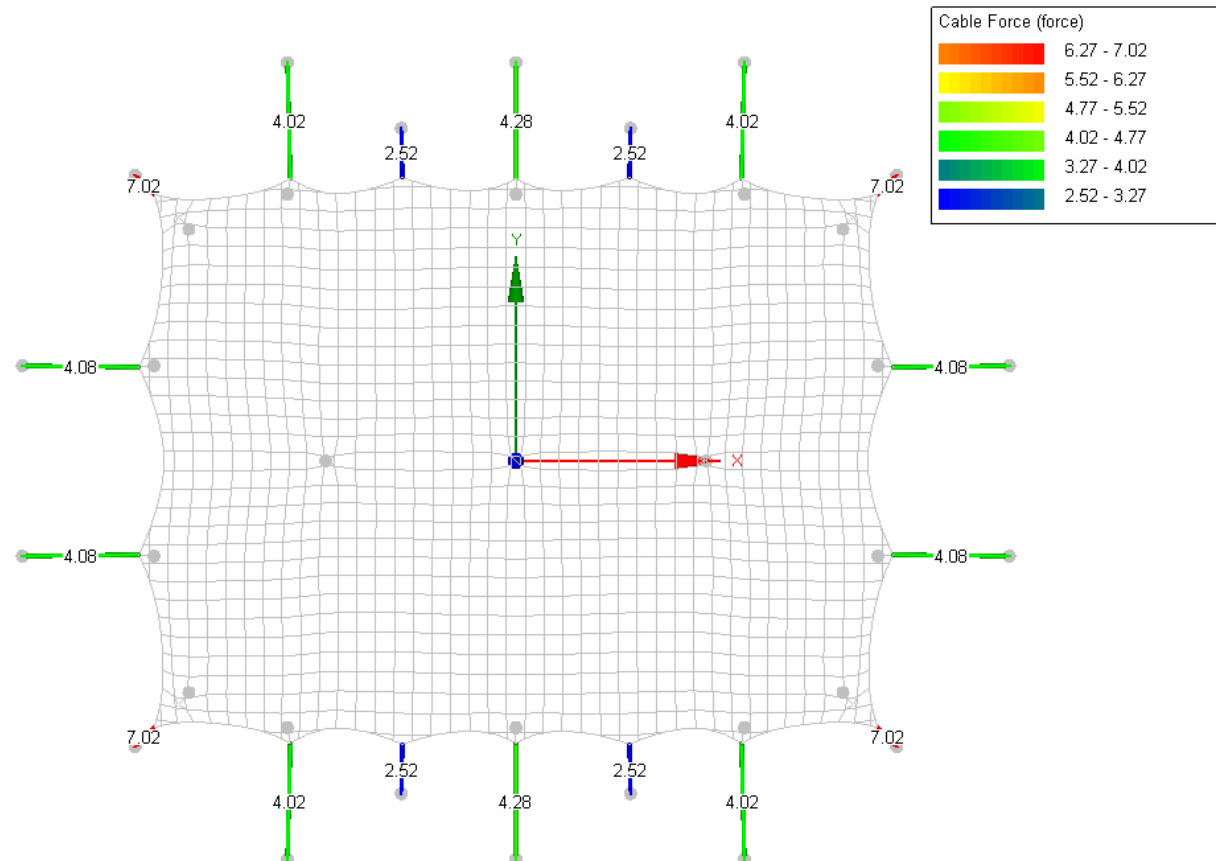
**Annex B.1.2.2. Membrane stress (weft)**



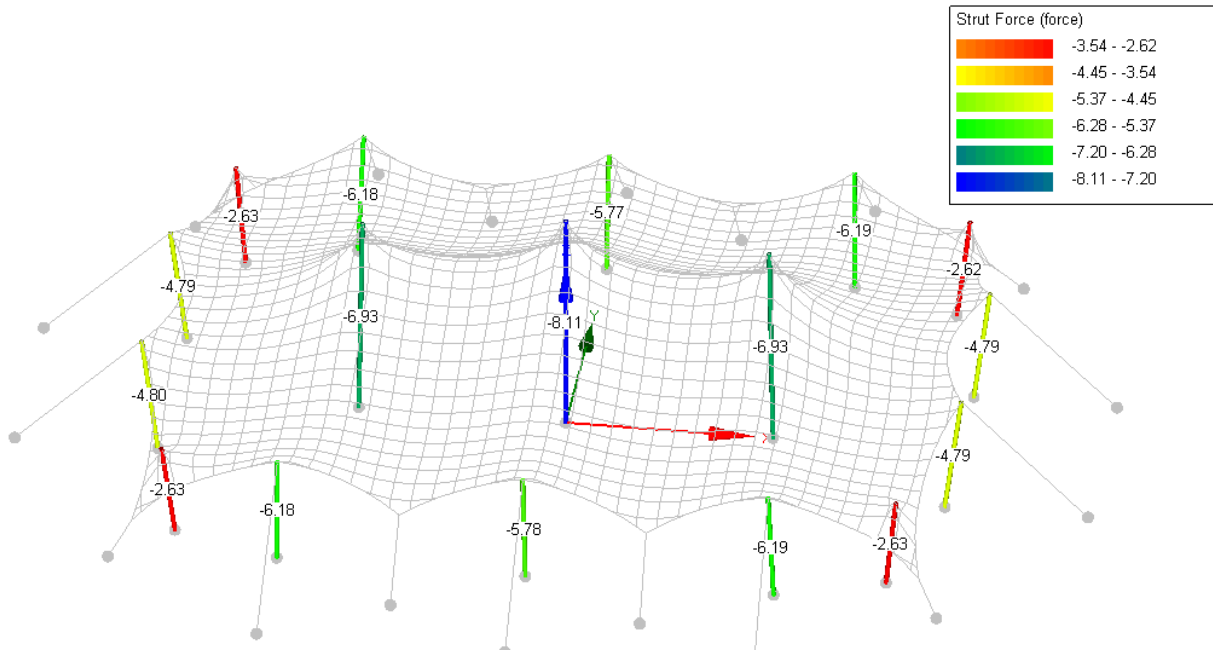
Annex B.1.2.3. Membrane edge



Annex B.1.2.4. Cable forces

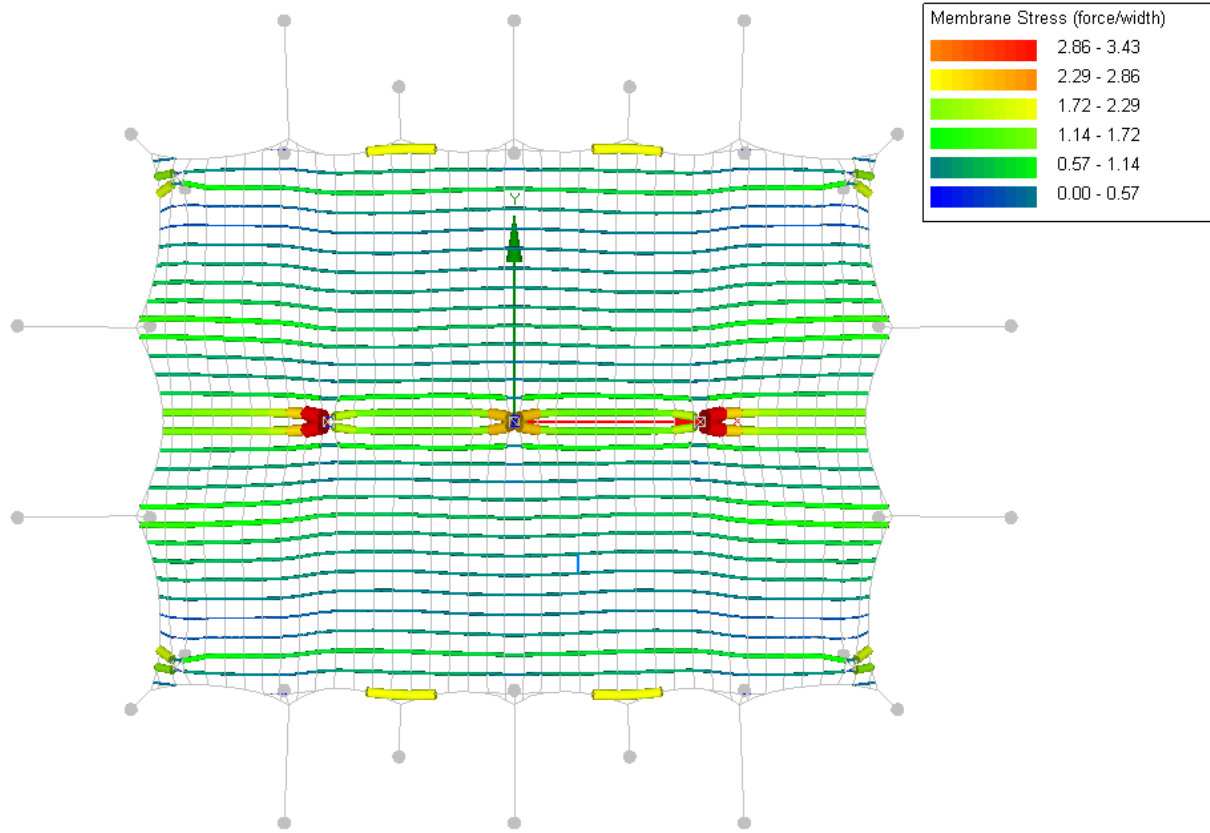


Annex B.1.2.5. Strut forces

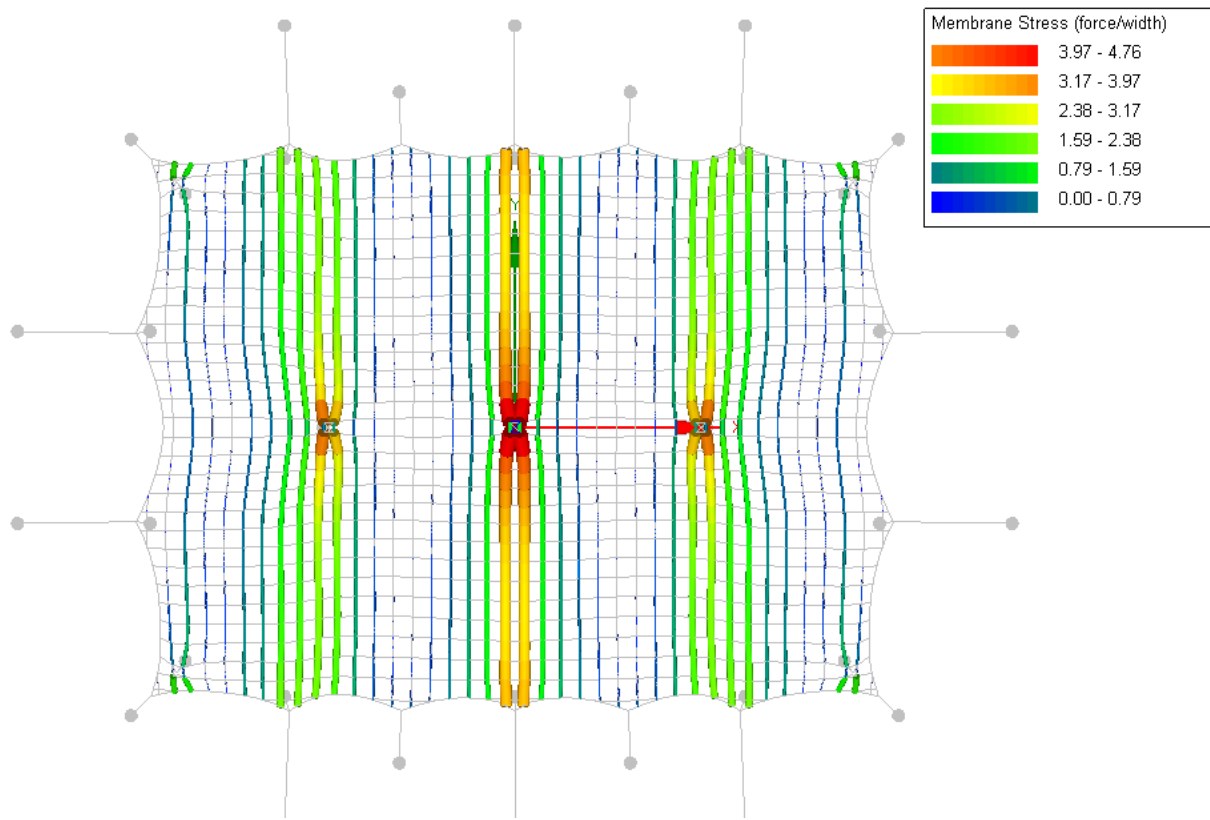


**Annex B.1.3. CO3 Own weight + Pretension + Wind pressure**

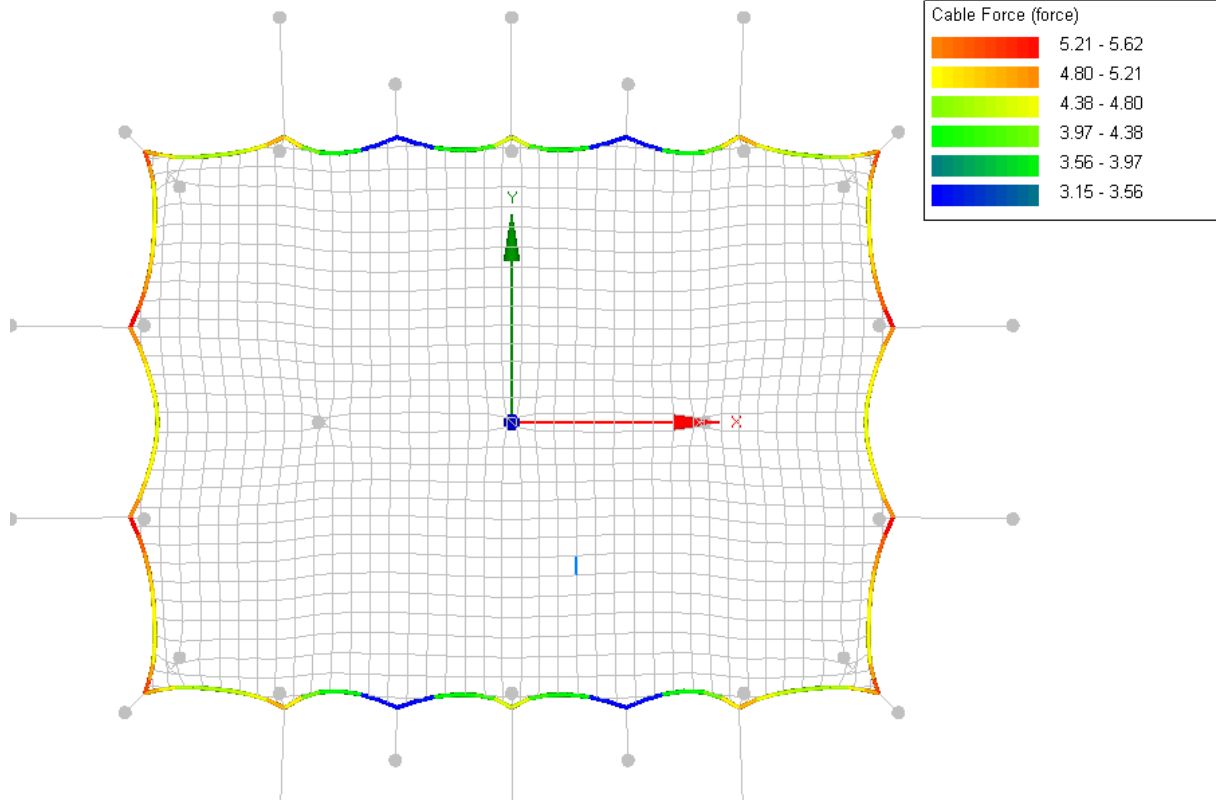
**Annex B.1.3.1. Membrane stress (warp)**



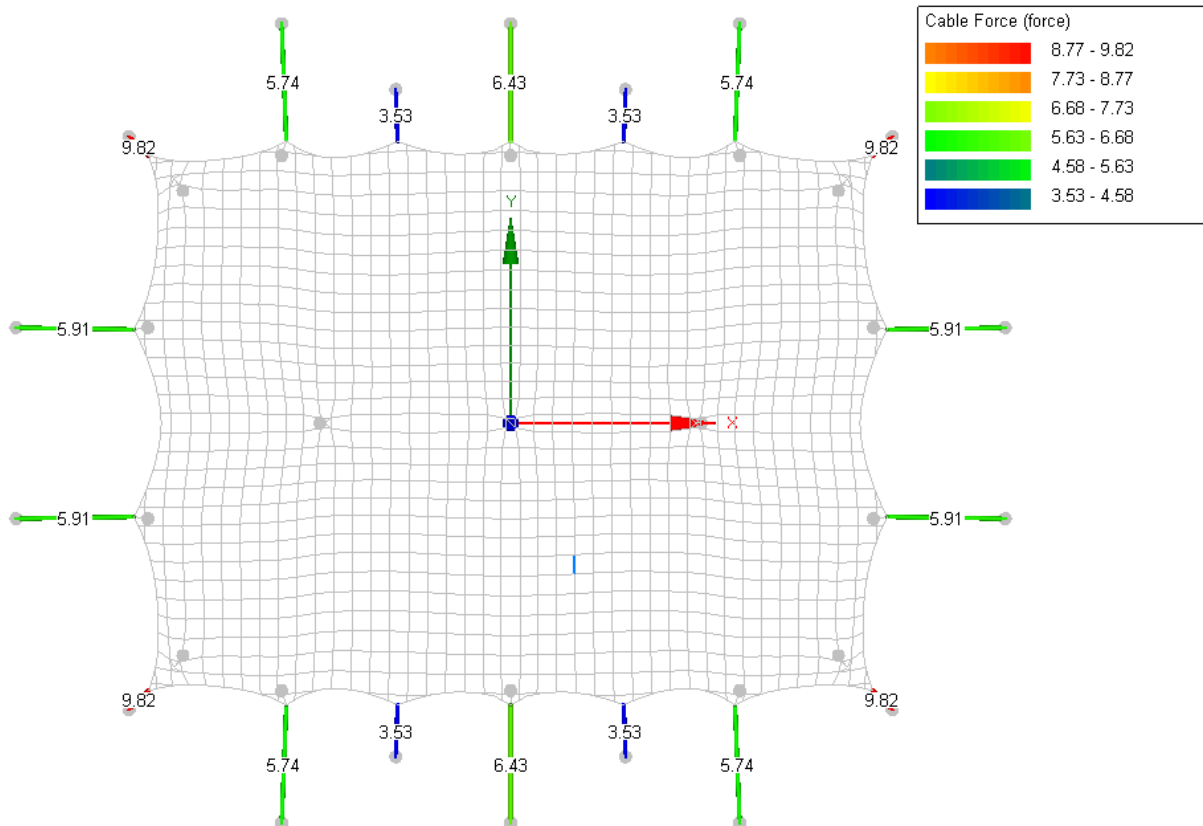
**Annex B.1.3.2. Membrane stress (weft)**



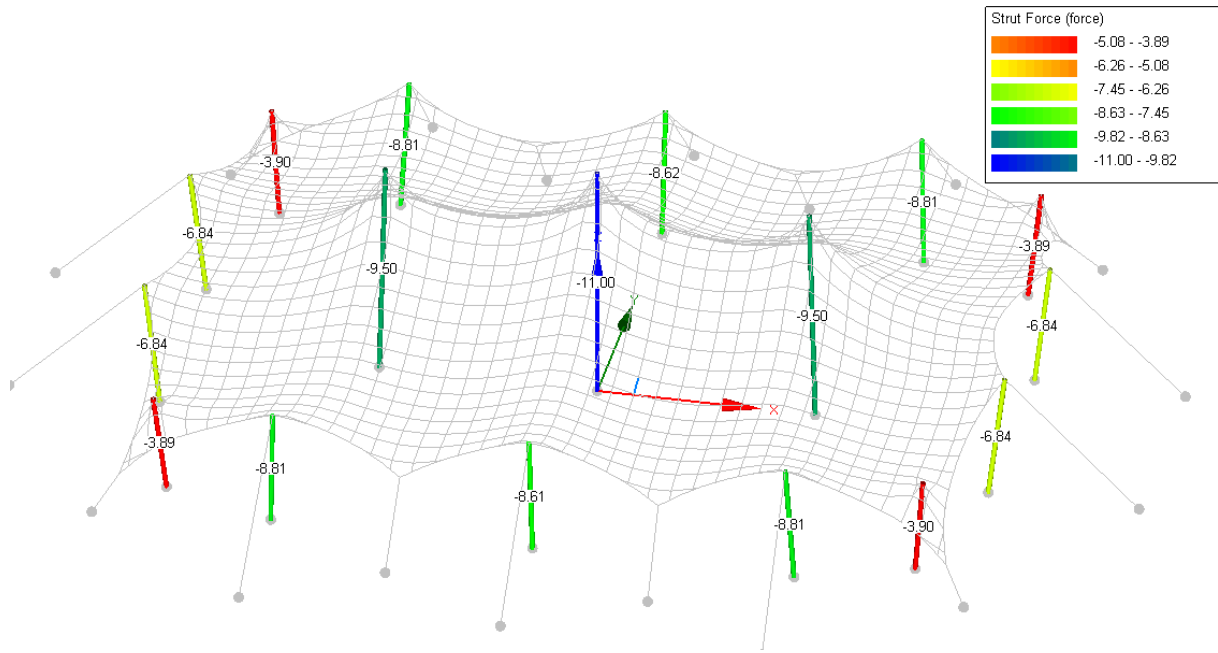
Annex B.1.3.3. Membrane edge



Annex B.1.3.4. Cable forces



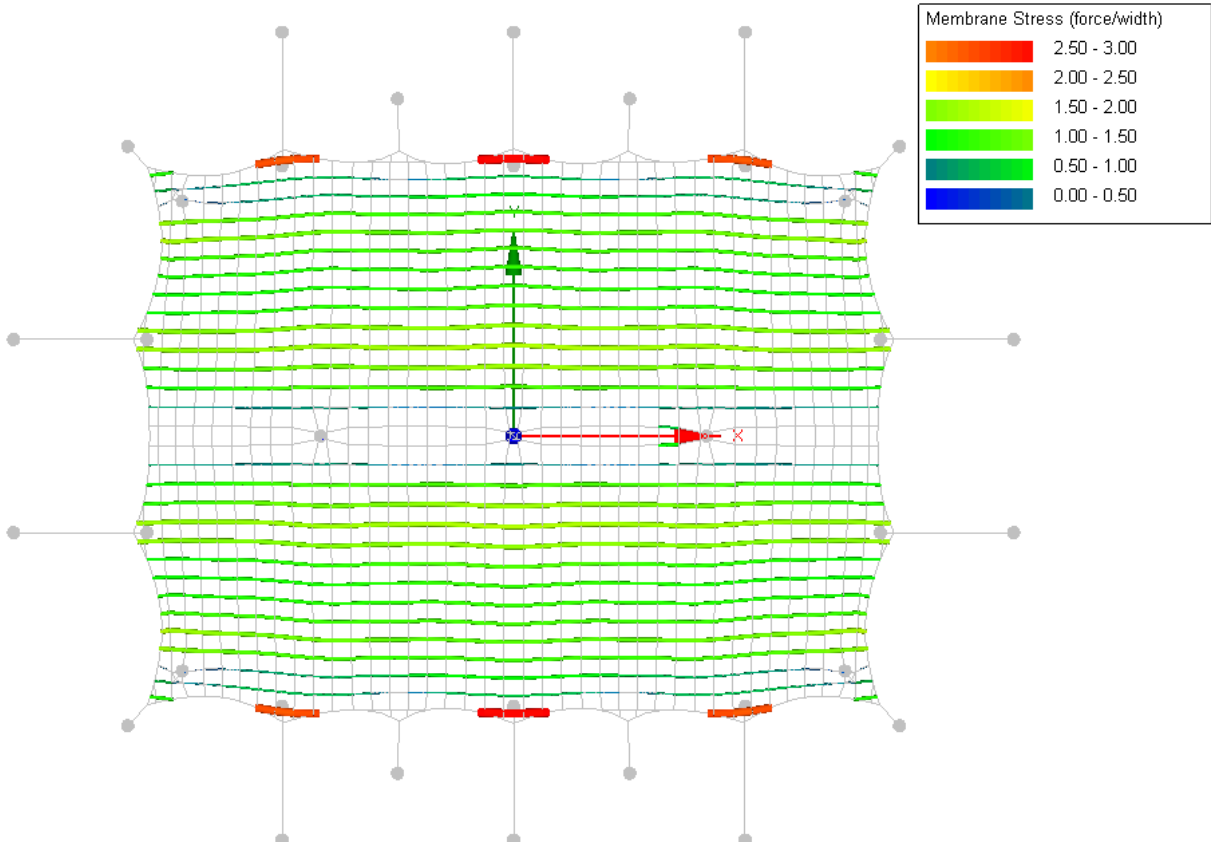
Annex B.1.3.5. Strut forces



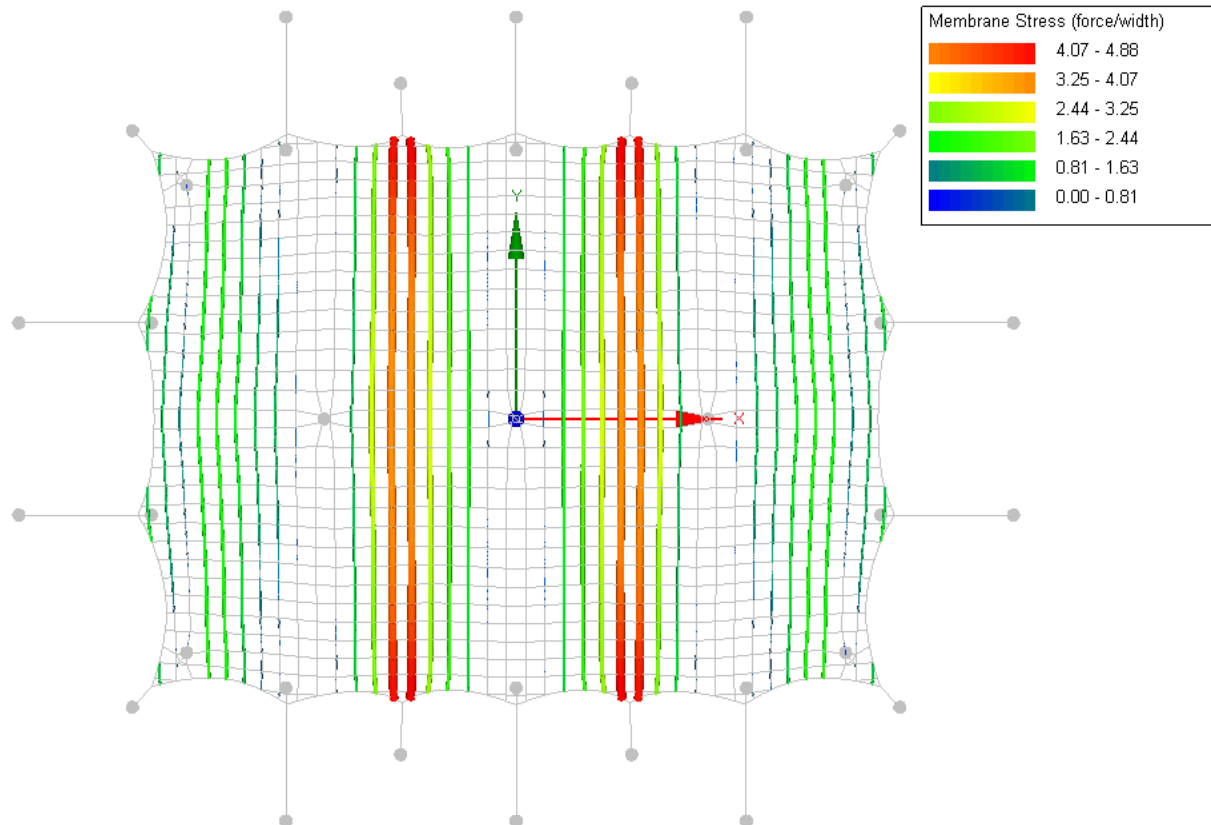


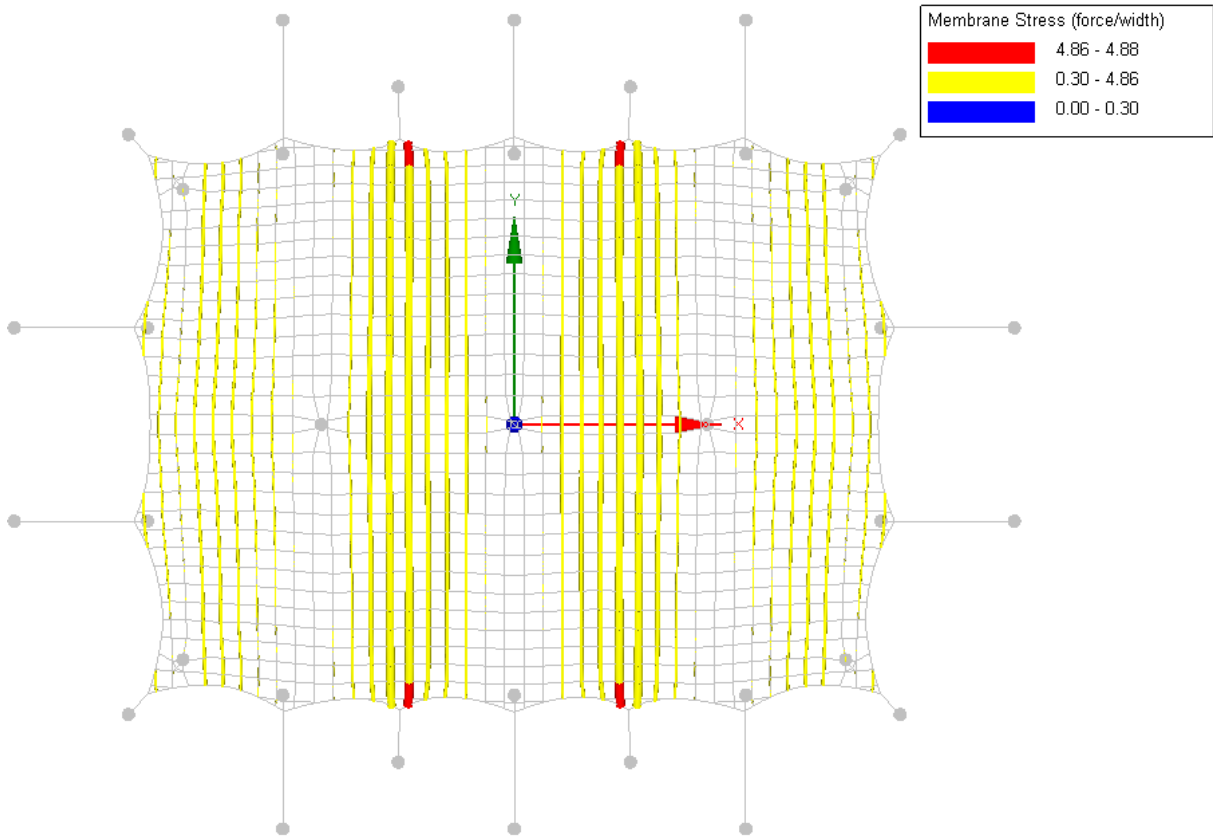
**Annex B.1.4. CO4 Own weight + Pretension + Wind suction – floating – reduction 0.53**

**Annex B.1.4.1. Membrane stress (warp)**



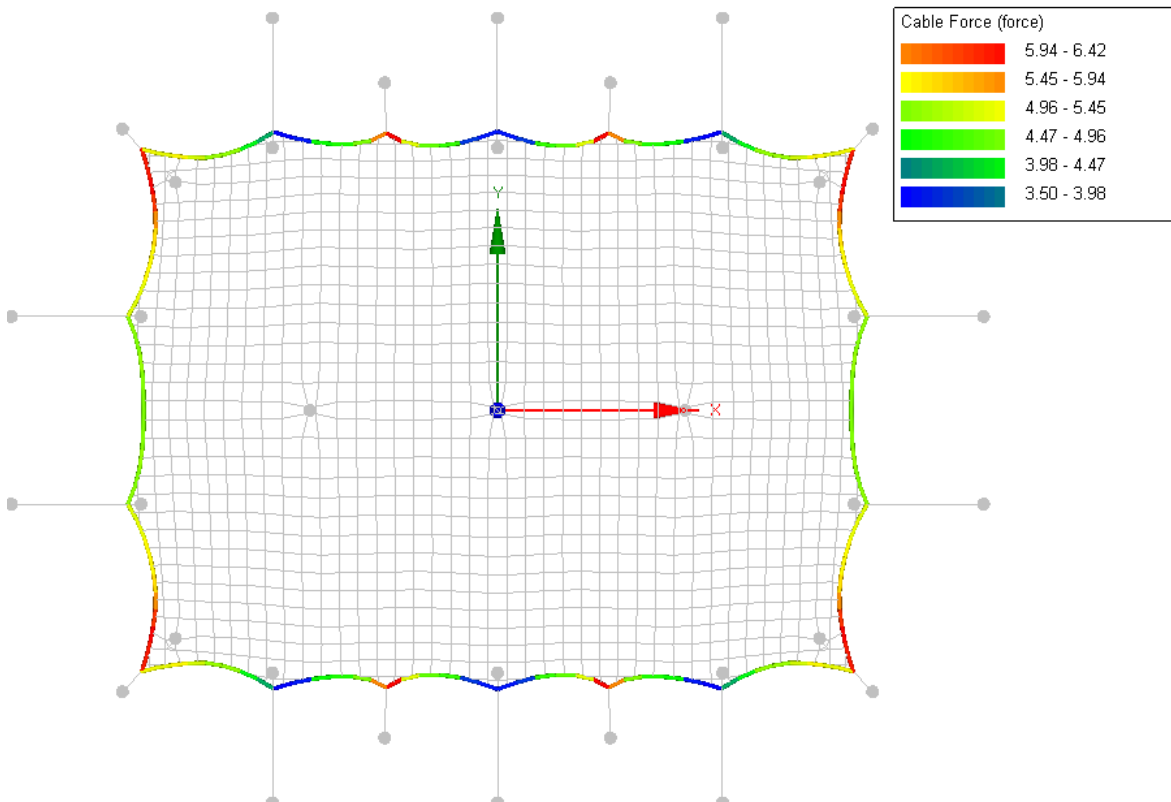
**Annex B.1.4.2. Membrane stress (weft)**



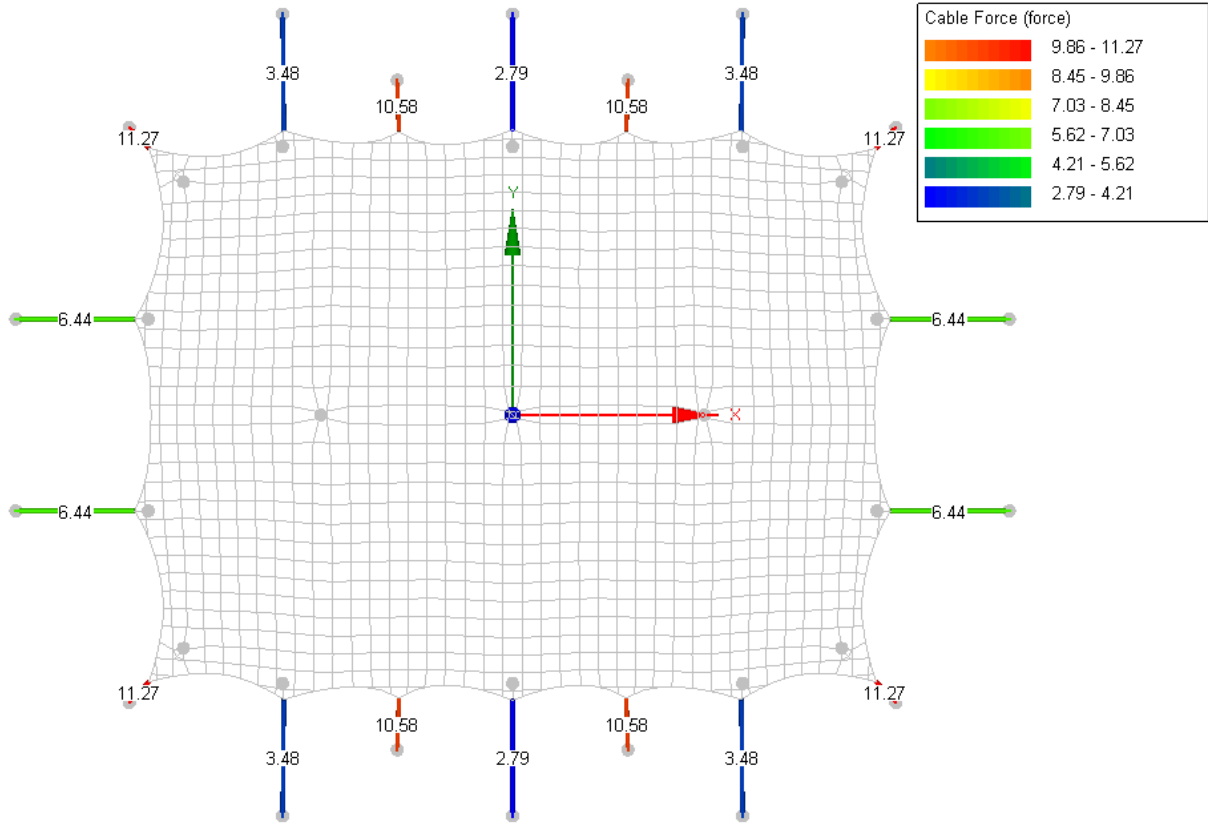


If the weft direction of the fabric runs along the width of the tent (vertical in the picture above), the capacity of the membrane is exceeded in the red colored links. However, these are local stresses near the edge, where multiple layers of fabric are present. Therefore, this is acceptable.

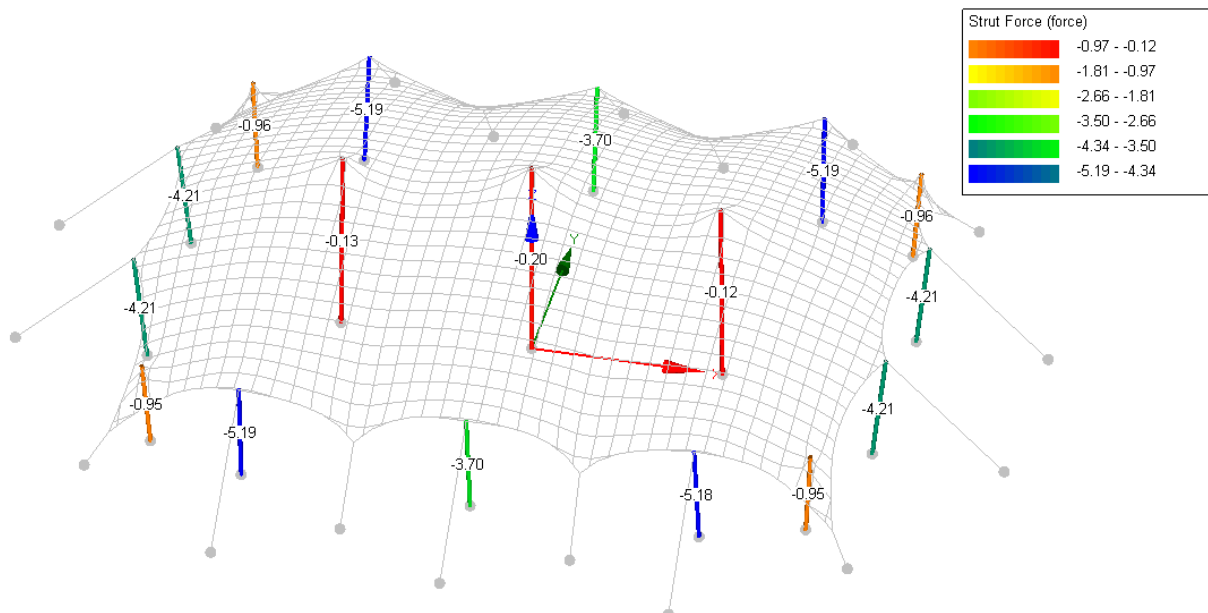
#### Annex B.1.4.3. Membrane edge



Annex B.1.4.4. Cable forces

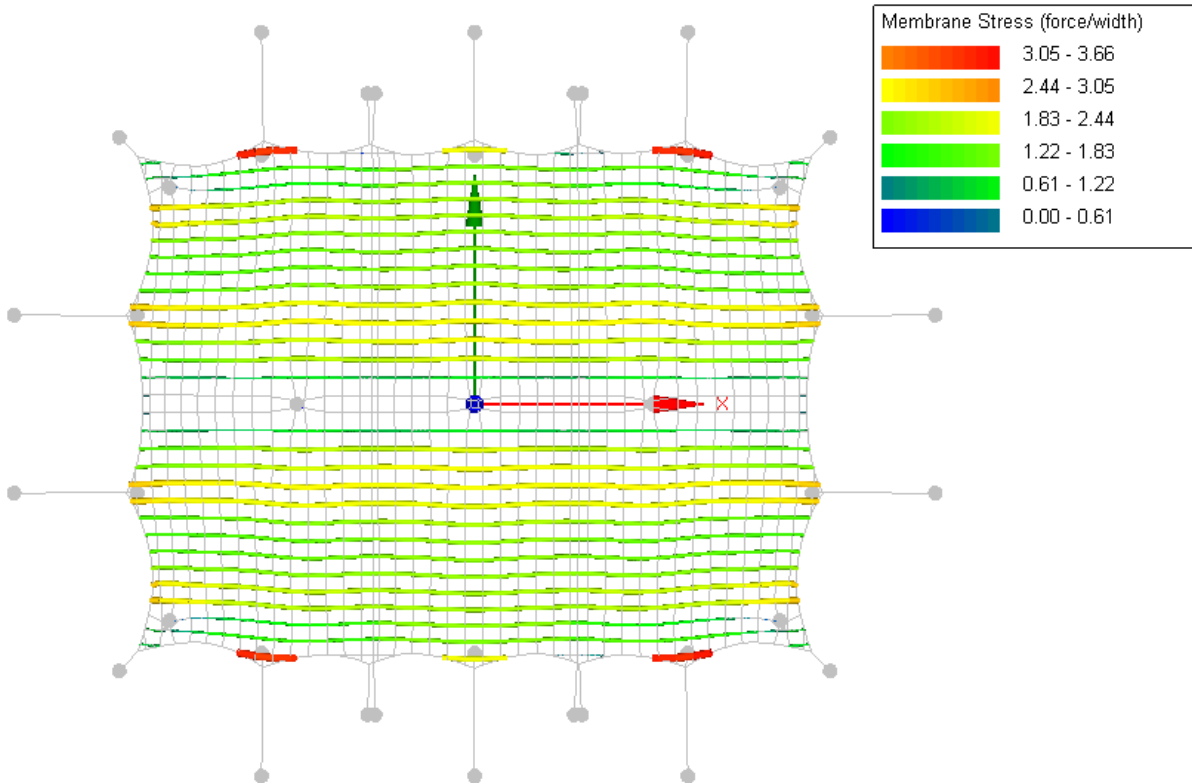


Annex B.1.4.5. Strut forces

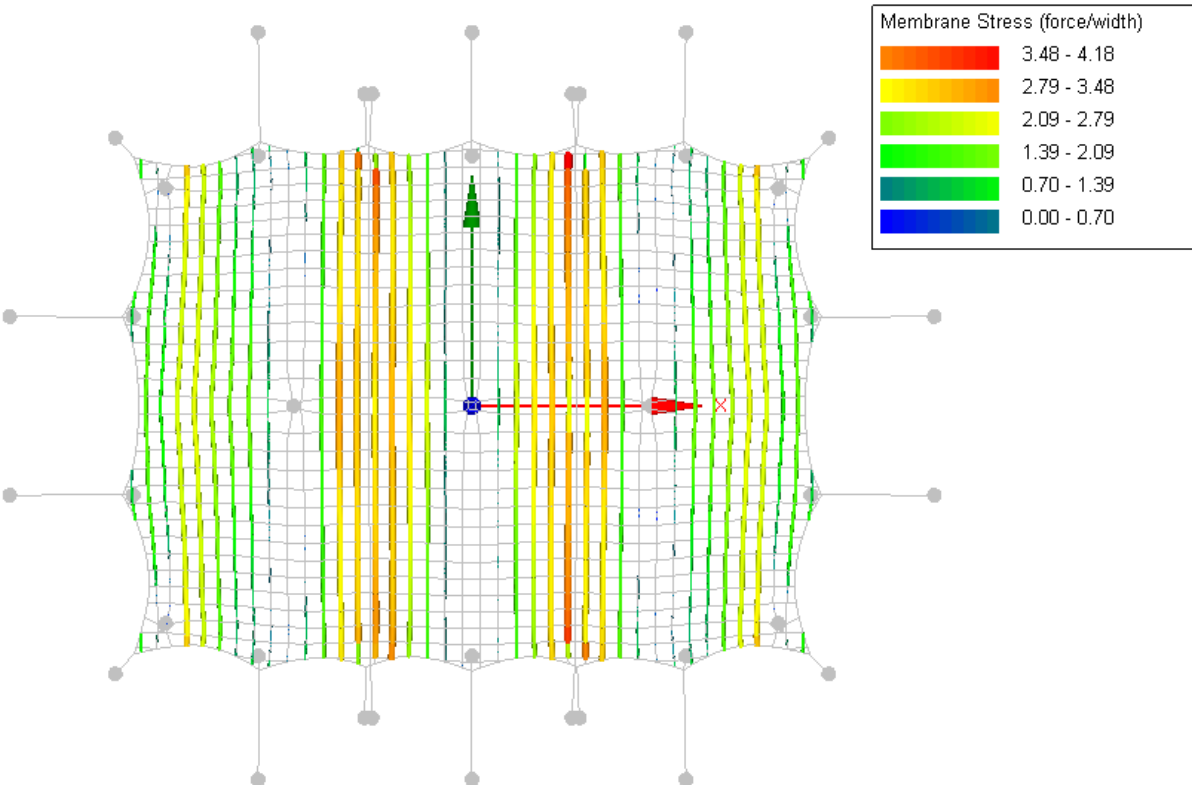


**Annex B.1.5. CO5 Own weight + Pretension + Wind suction – floating – full wind load**

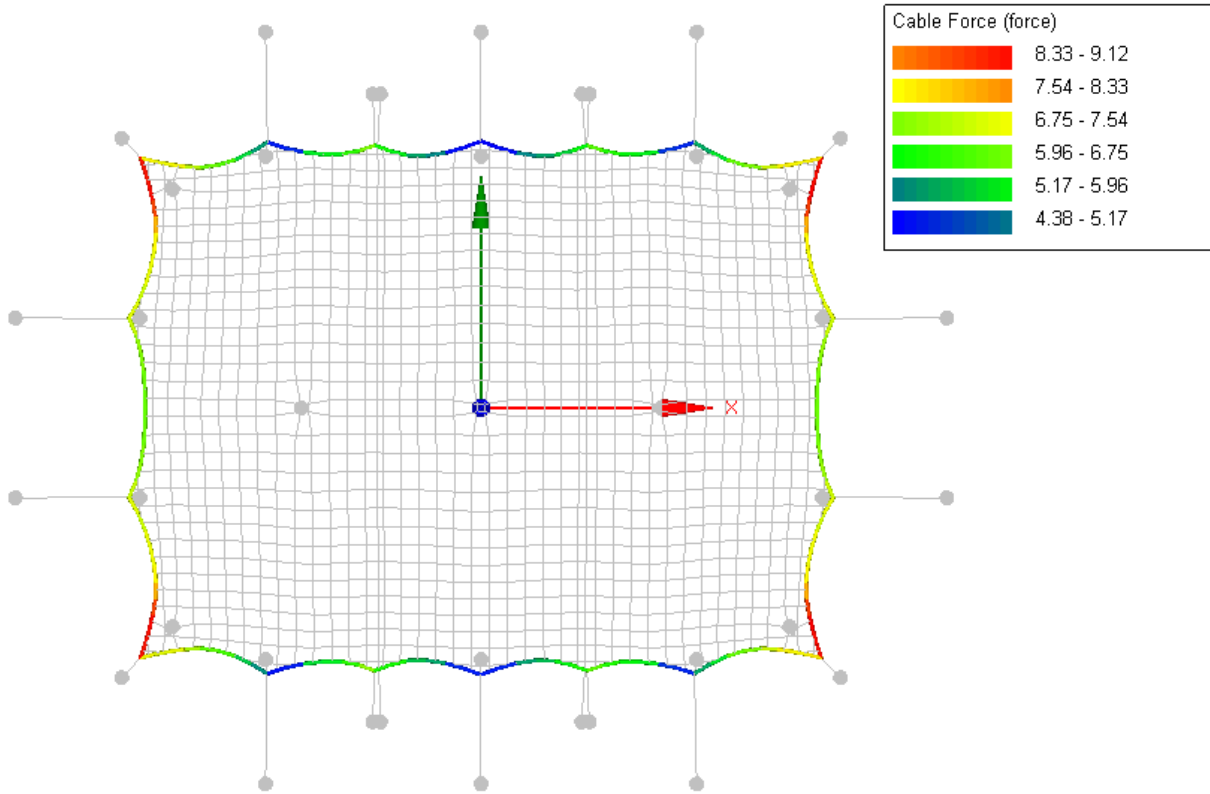
**Annex B.1.5.1. Membrane stress (warp)**



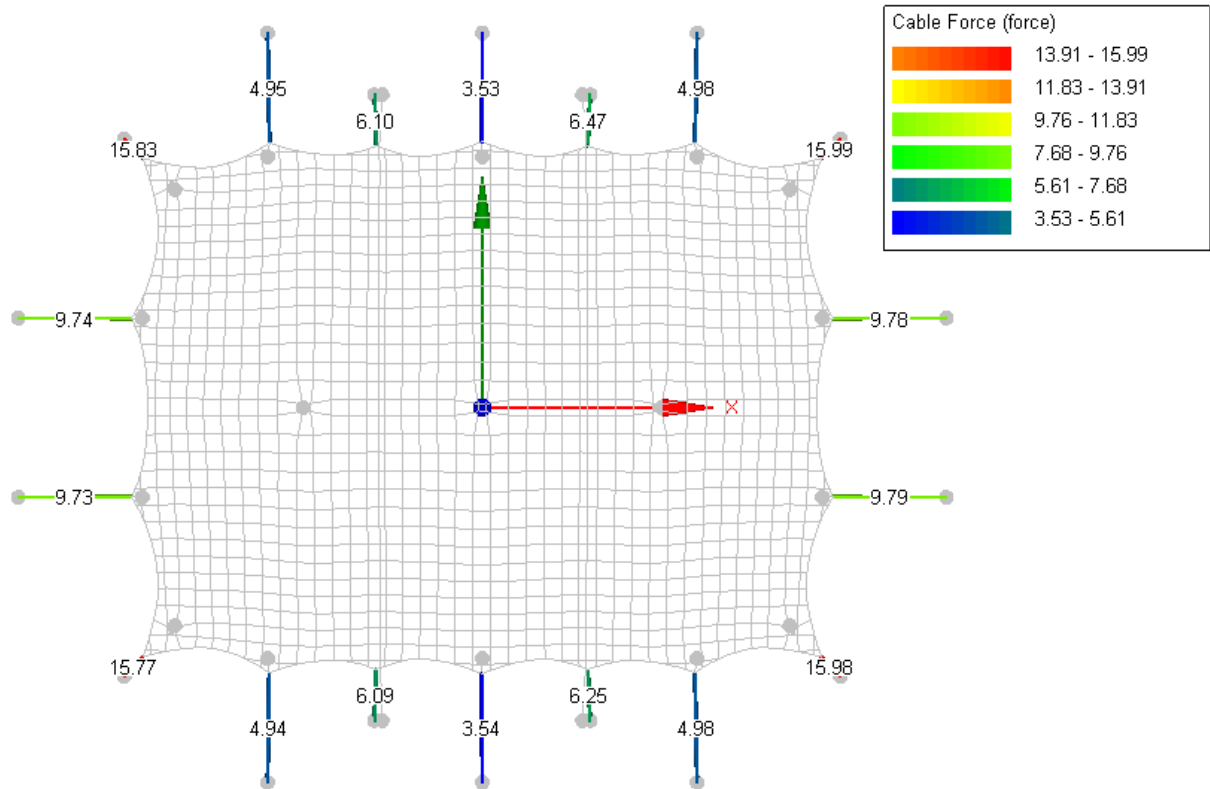
**Annex B.1.5.2. Membrane stress (weft)**



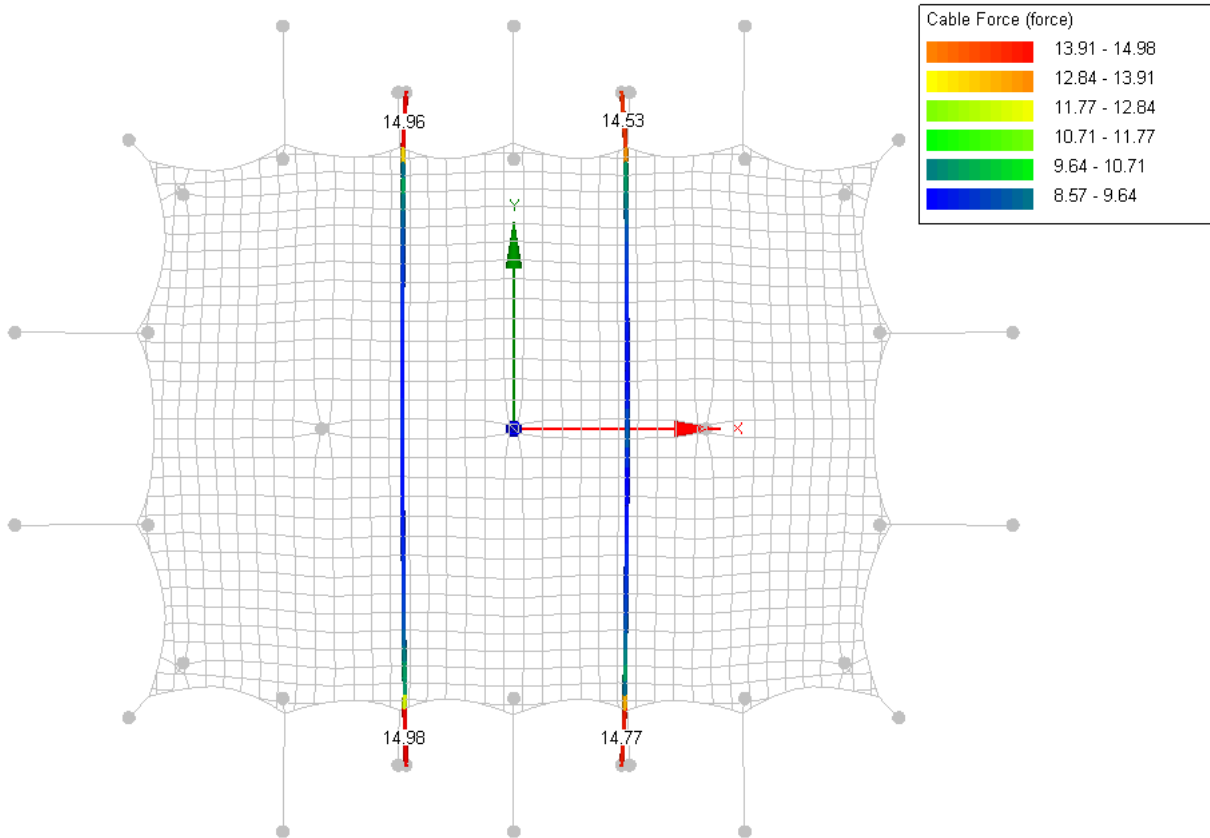
Annex B.1.5.3. Membrane edge



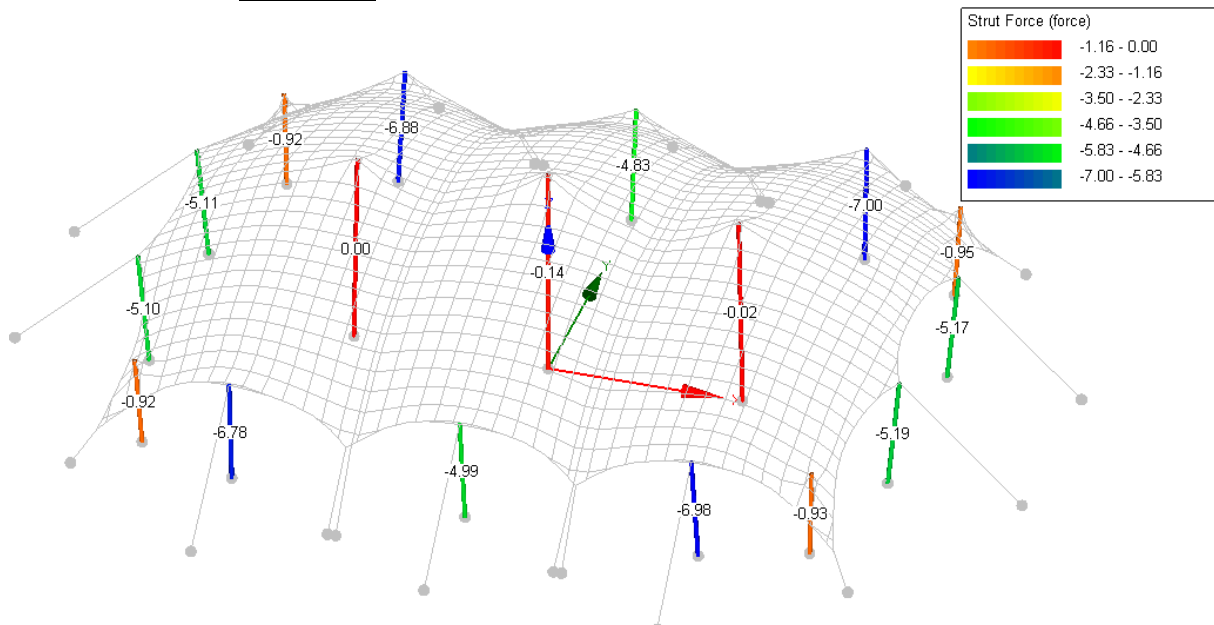
Annex B.1.5.4. Cable forces



Annex B.1.5.5. Storm belts



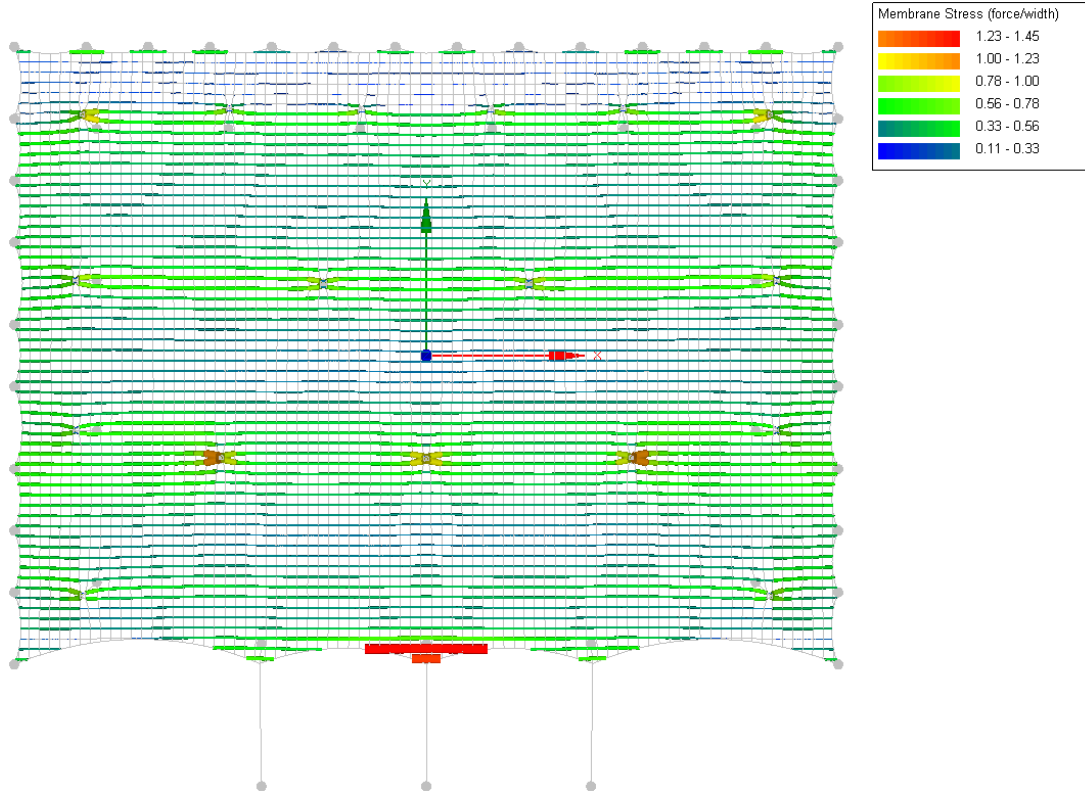
Annex B.1.5.6. Strut forces



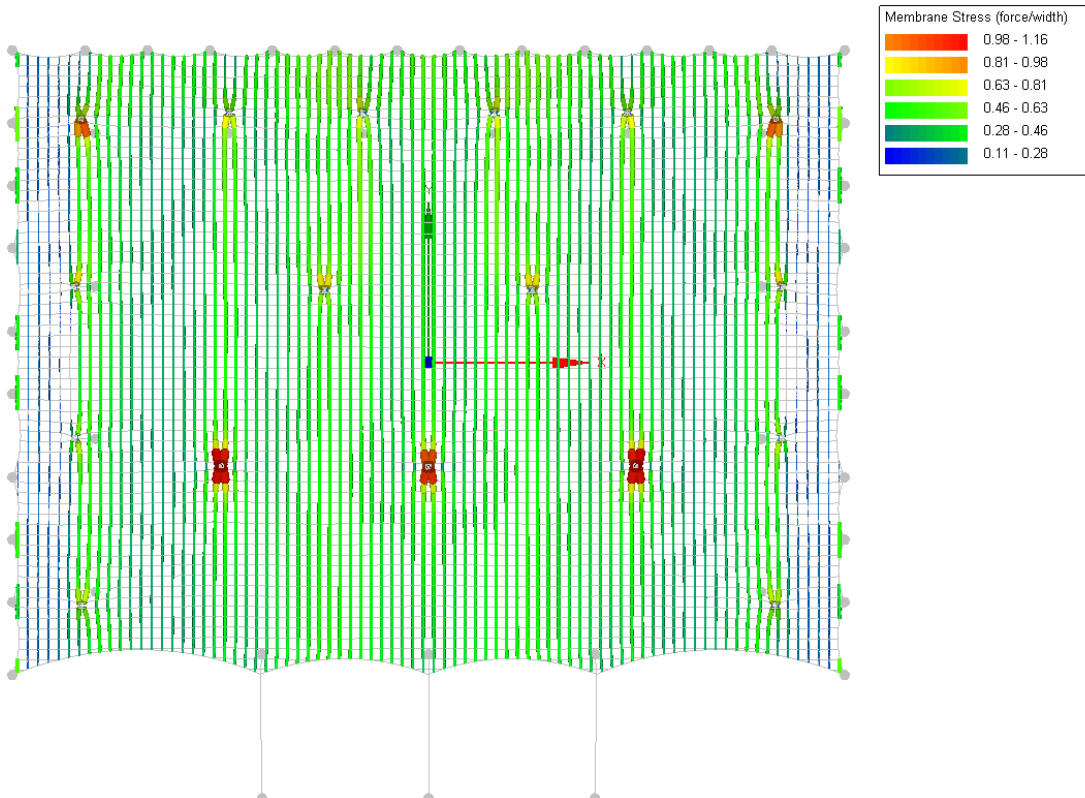
## Annex B.2. 20x15m - closed

### Annex B.2.1. CO1 Own weight + Pretension

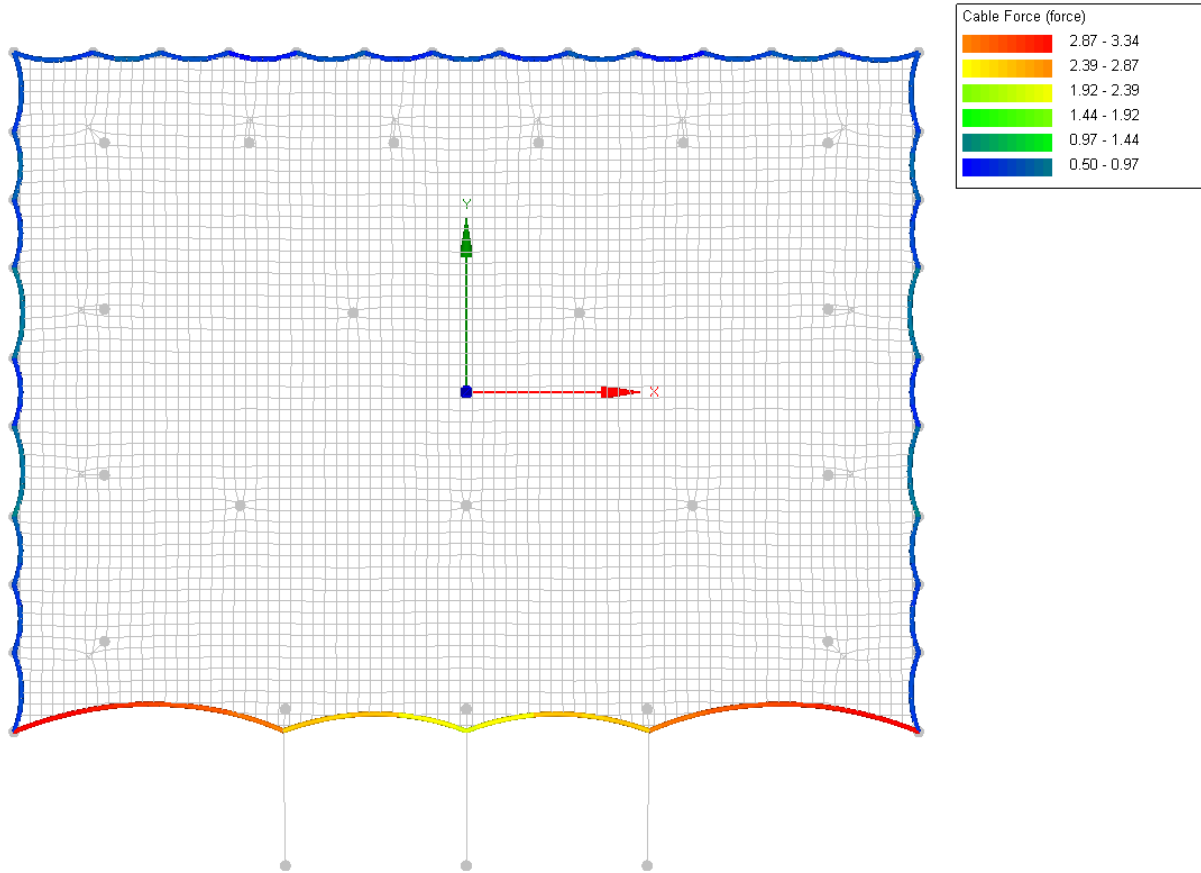
#### Annex B.2.1.1. Membrane stress (warp)



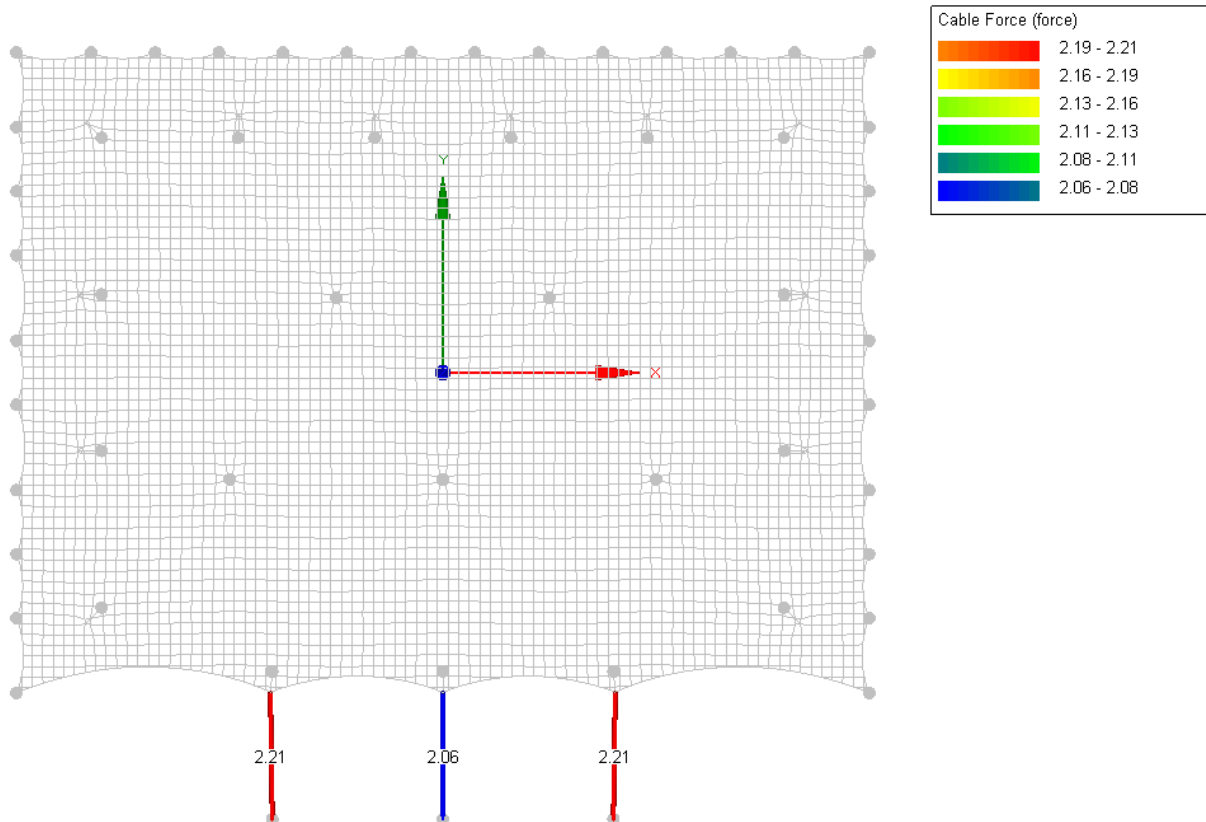
#### Annex B.2.1.2. Membrane stress (weft)



Annex B.2.1.3. Membrane edge

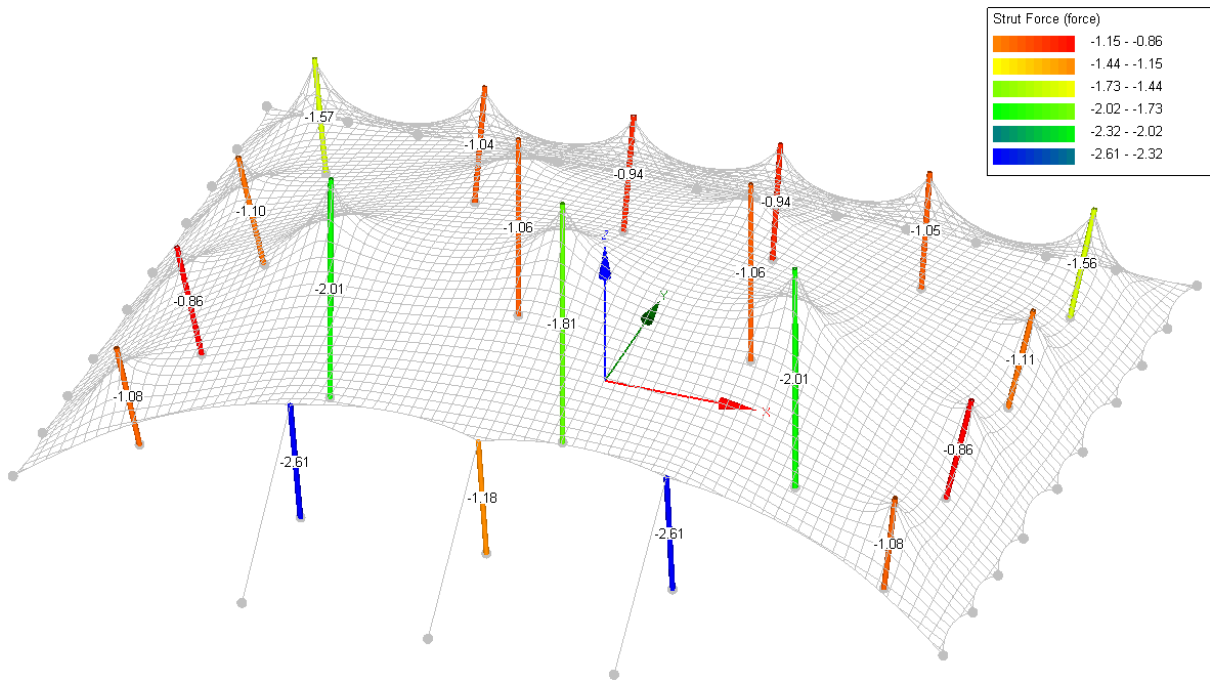


Annex B.2.1.4. Cable forces



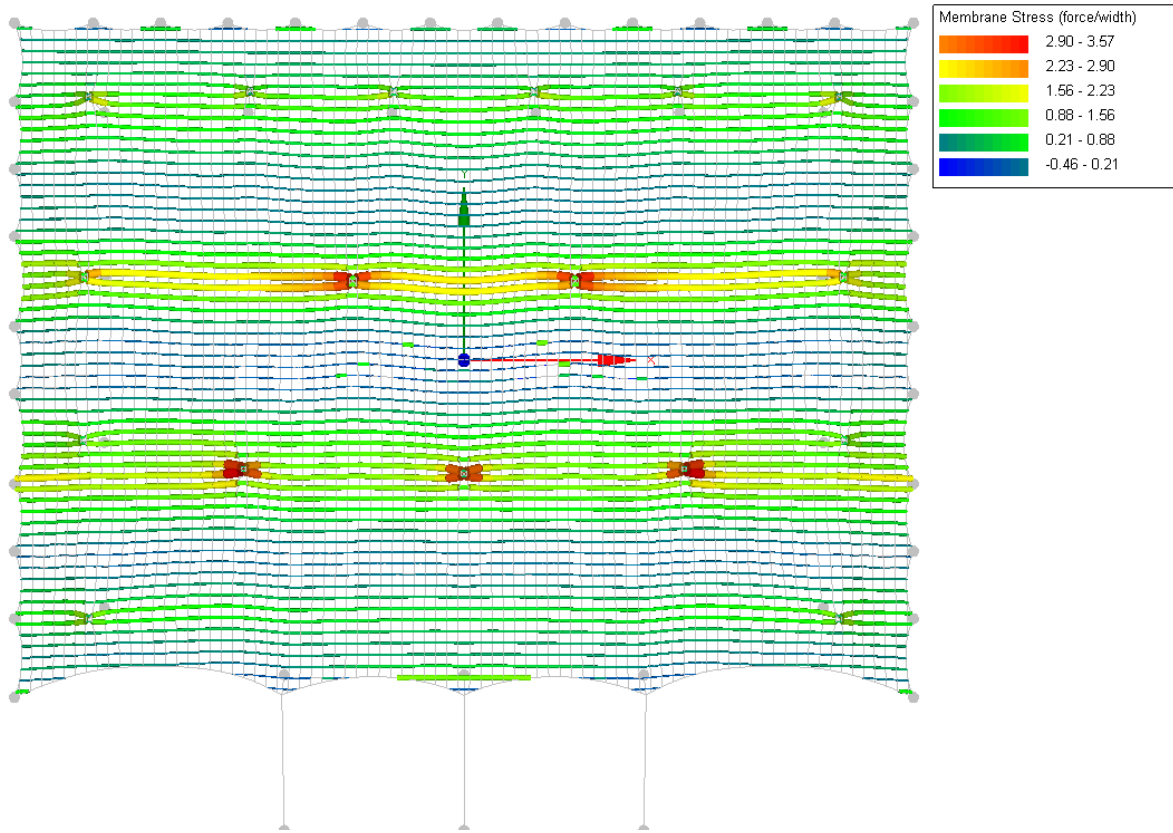


Annex B.2.1.5. Strut forces

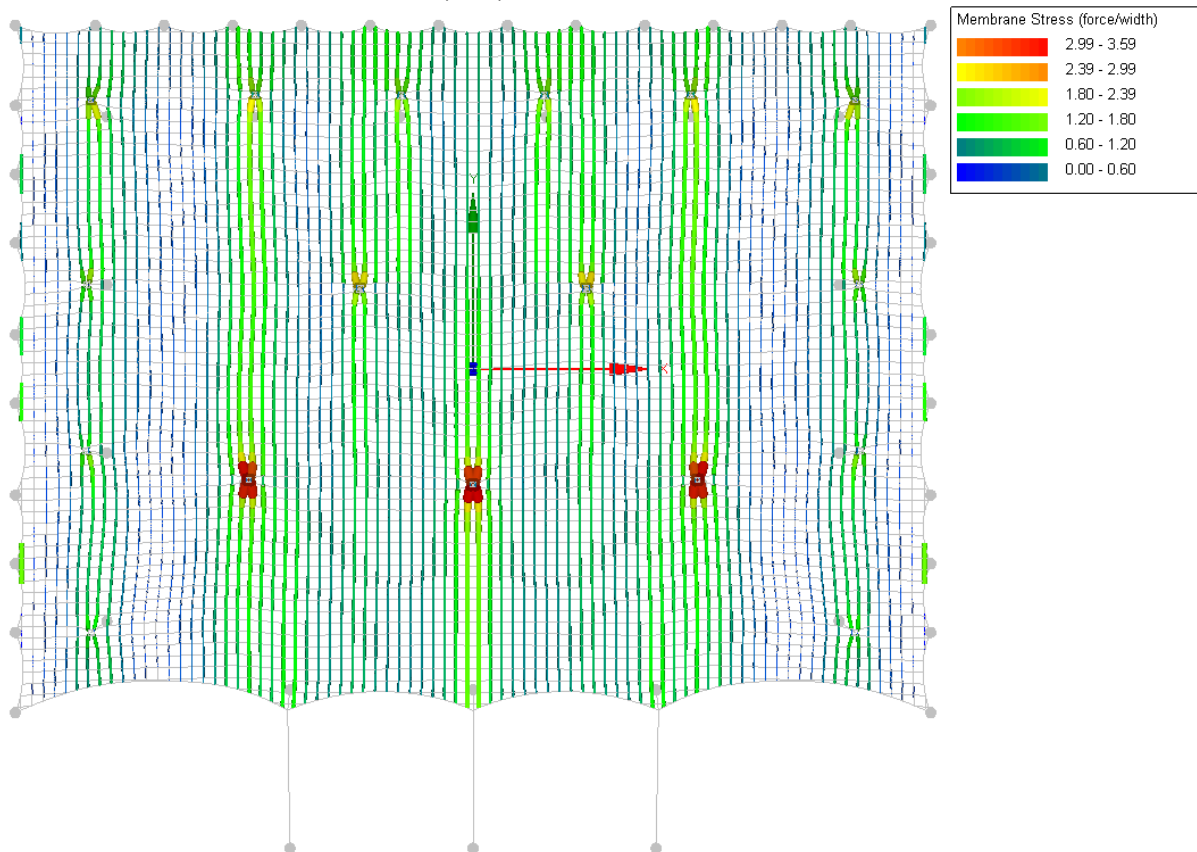


**Annex B.2.2. CO2 Own weight + Pretension + Conventional / snow**

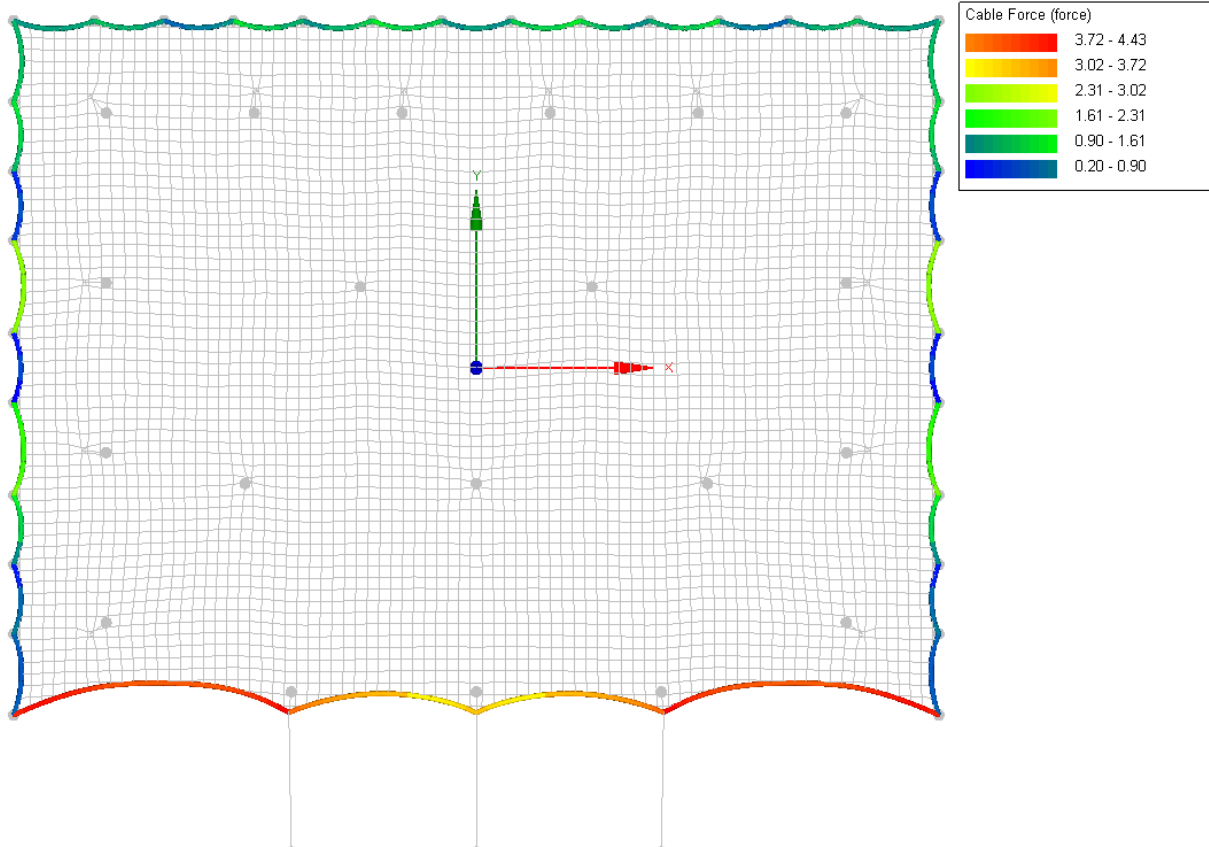
**Annex B.2.2.1. Membrane stress (warp)**



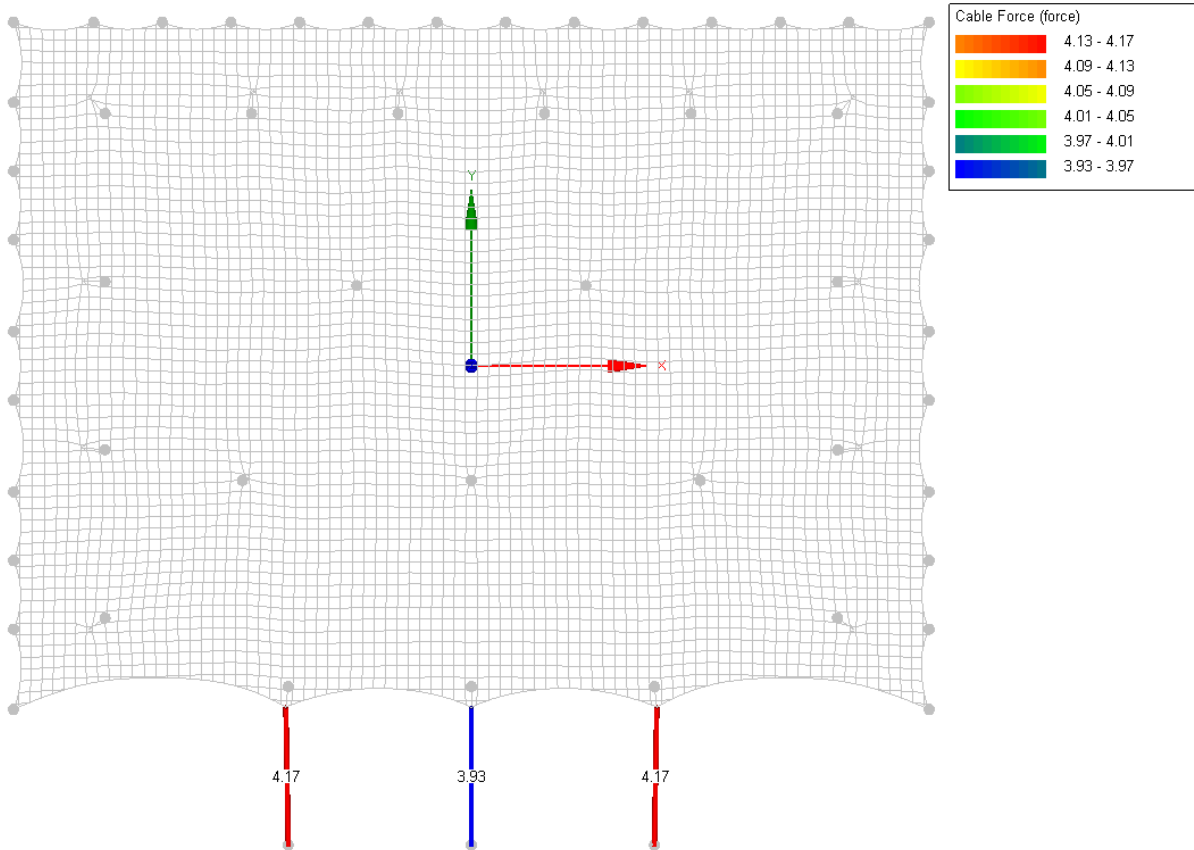
**Annex B.2.2.2. Membrane stress (weft)**



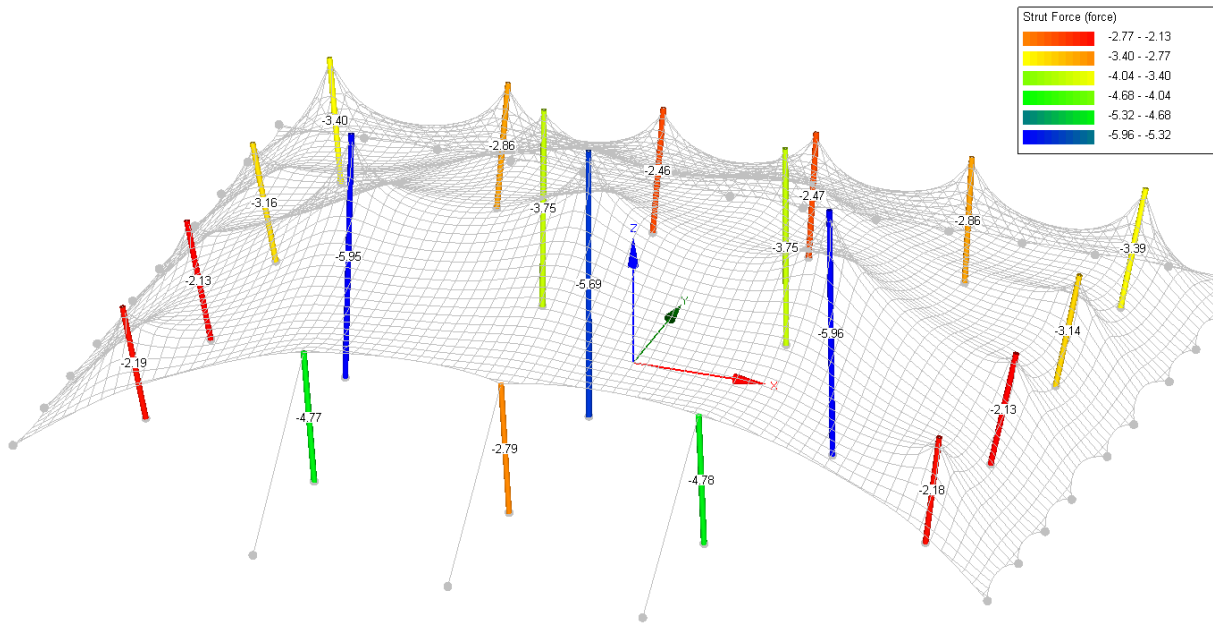
Annex B.2.2.3. Membrane edge



Annex B.2.2.4. Cable forces

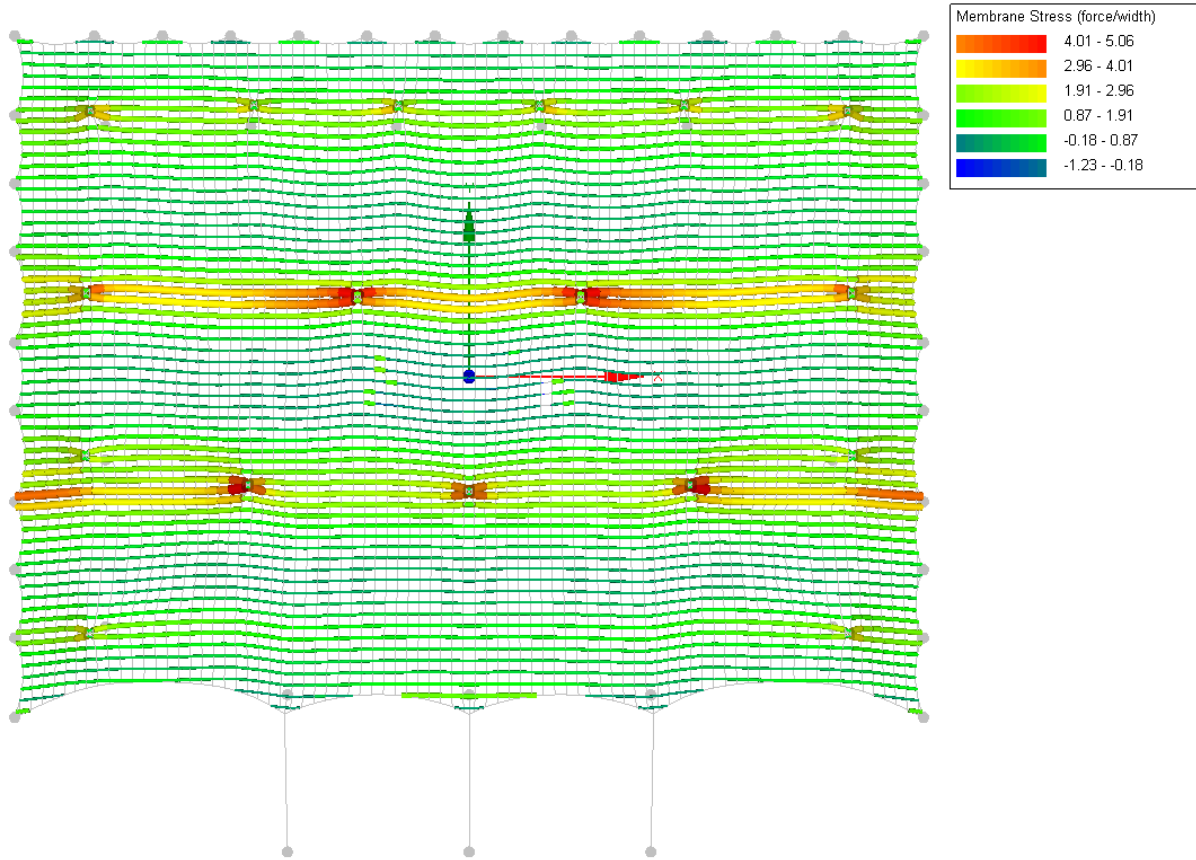


Annex B.2.2.5. Strut forces

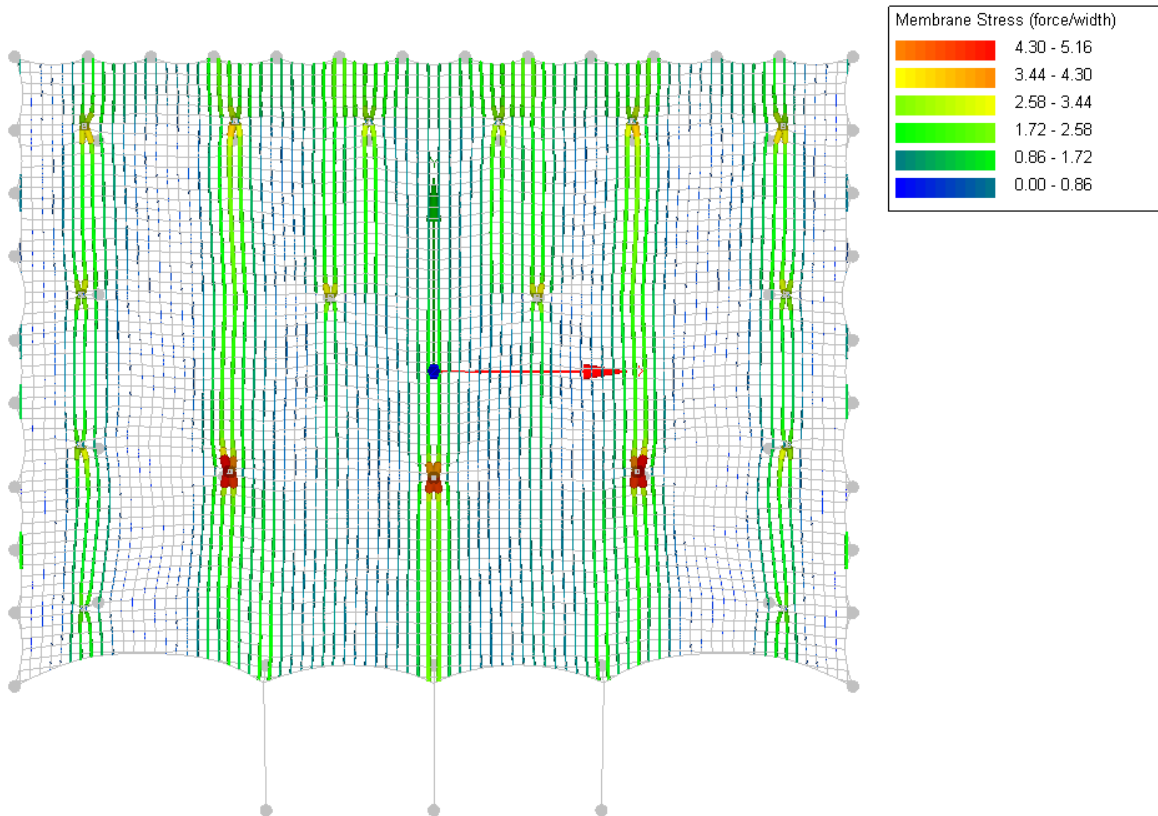


**Annex B.2.3. CO3 Own weight + Pretension + Wind pressure**

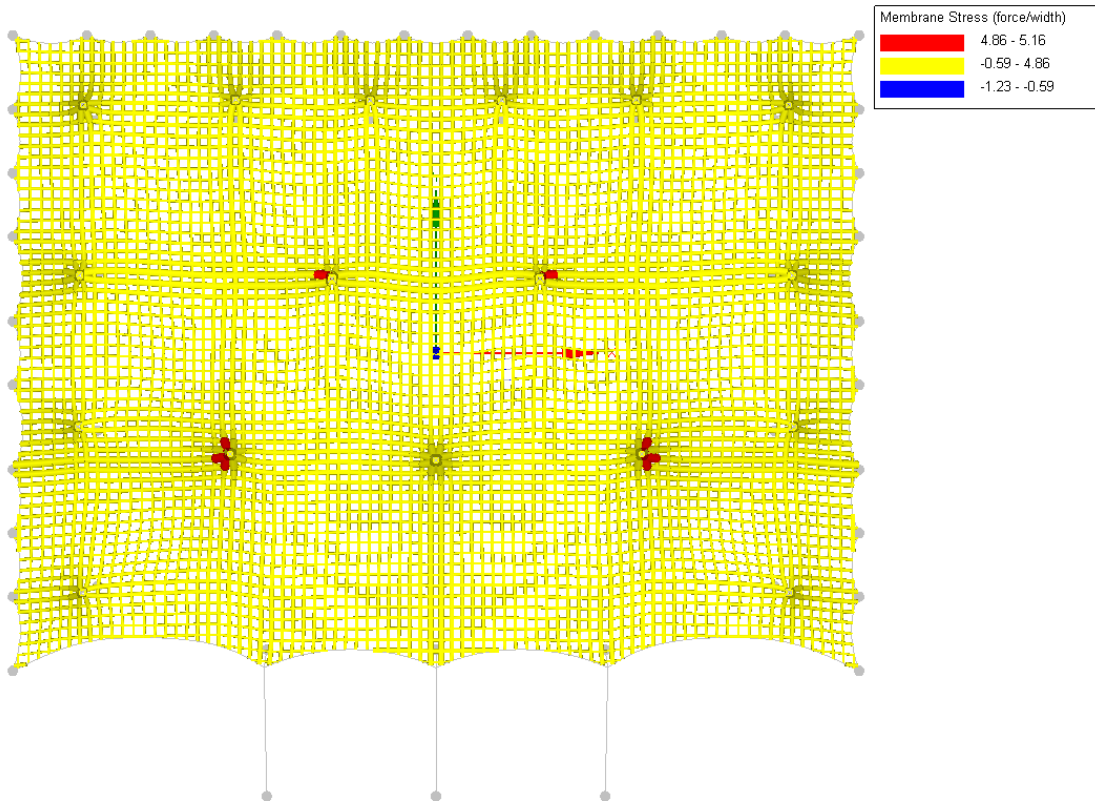
**Annex B.2.3.1. Membrane stress (warp)**



**Annex B.2.3.2. Membrane stress (weft)**

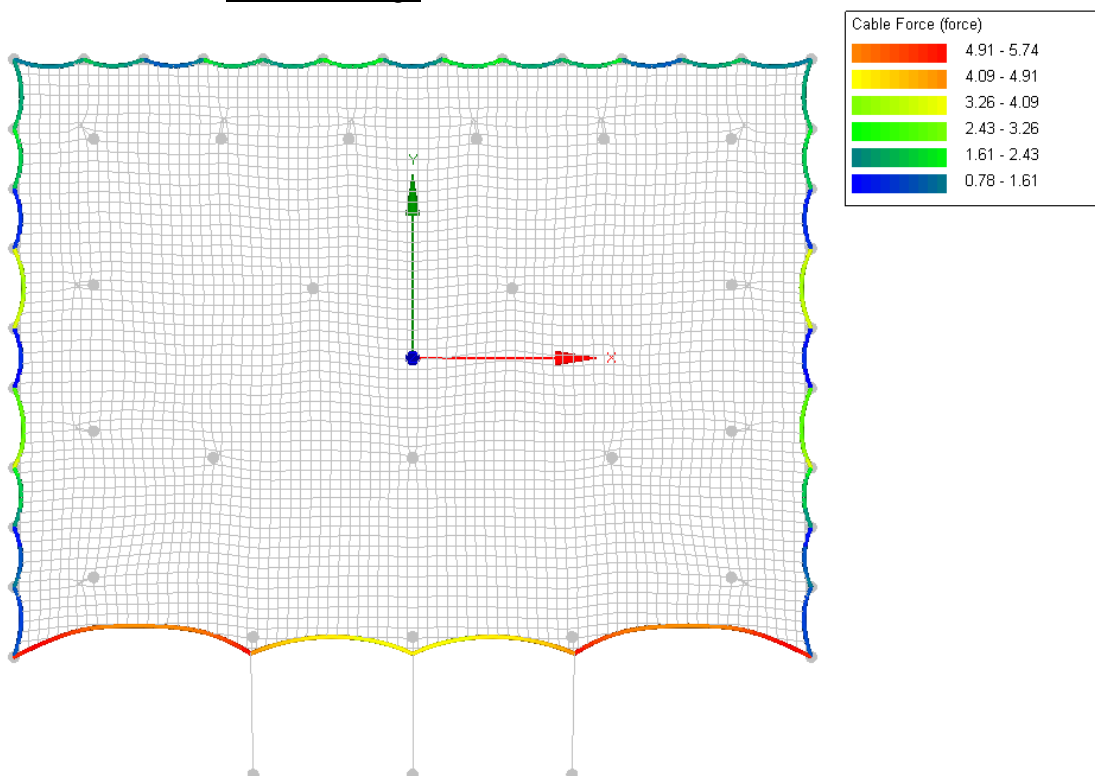




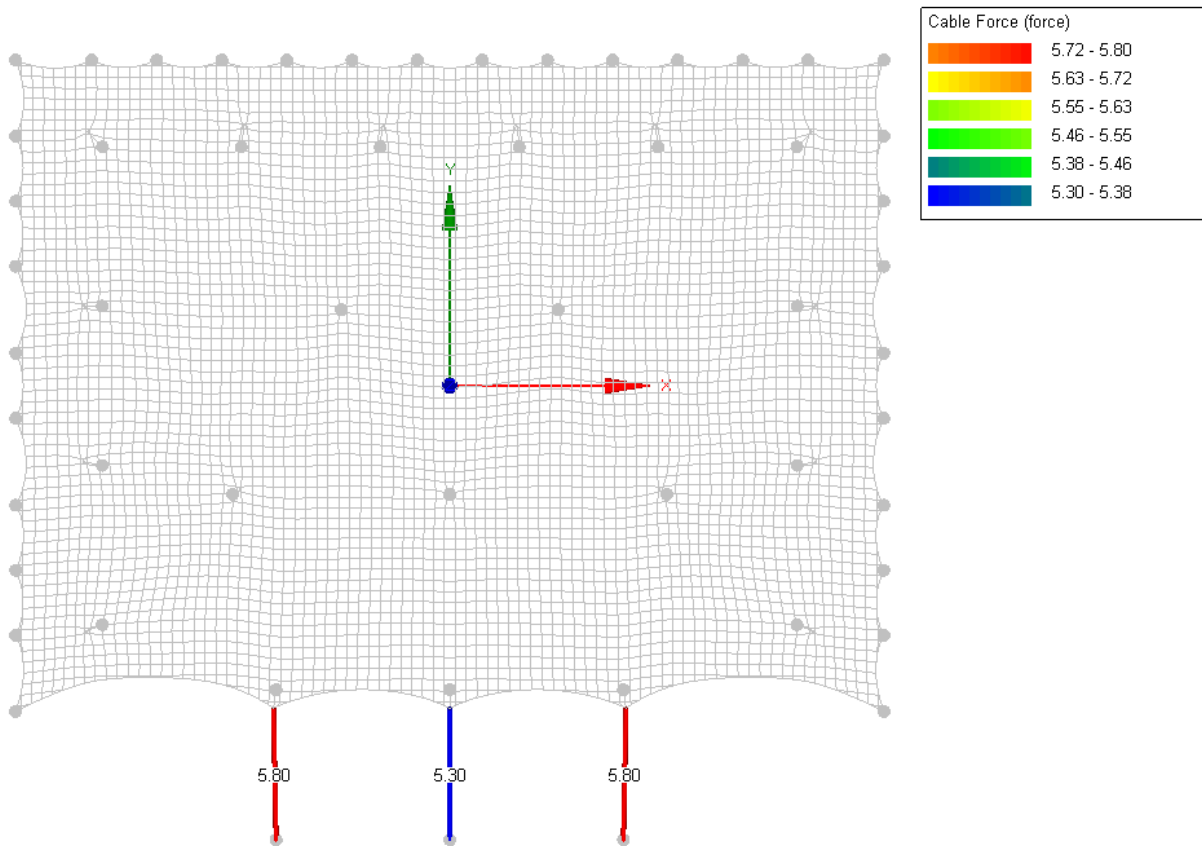


The capacity of the membrane is theoretically exceeded in the red colored links. However these are local stresses that occur due to the way the poletops are modelled. The stresses in these links may be averaged with the other poletop links to compensate for the modelling. With the averaged stresses the membrane will not burst.

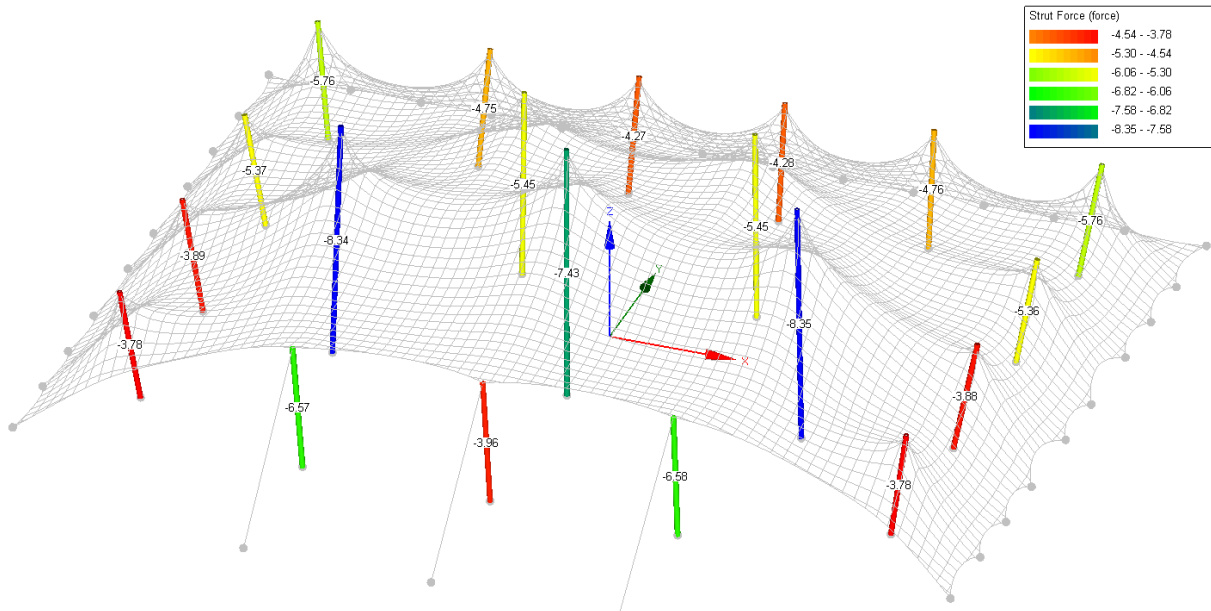
Annex B.2.3.3. Membrane edge



Annex B.2.3.4. Cable forces

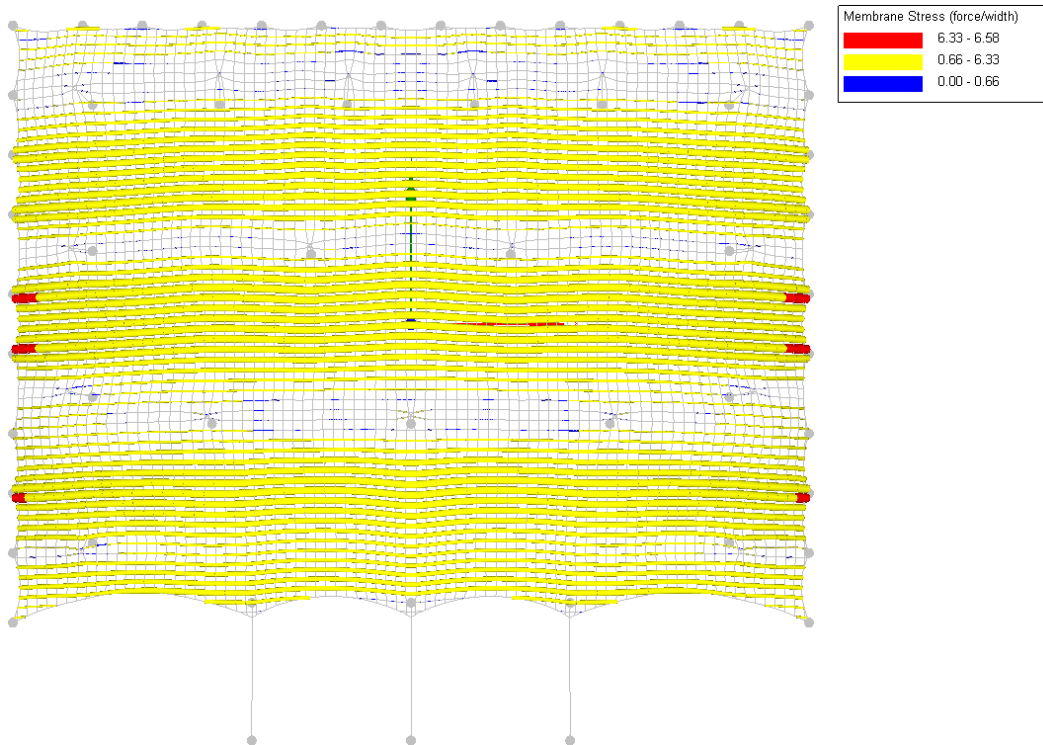


Annex B.2.3.5. Strut forces



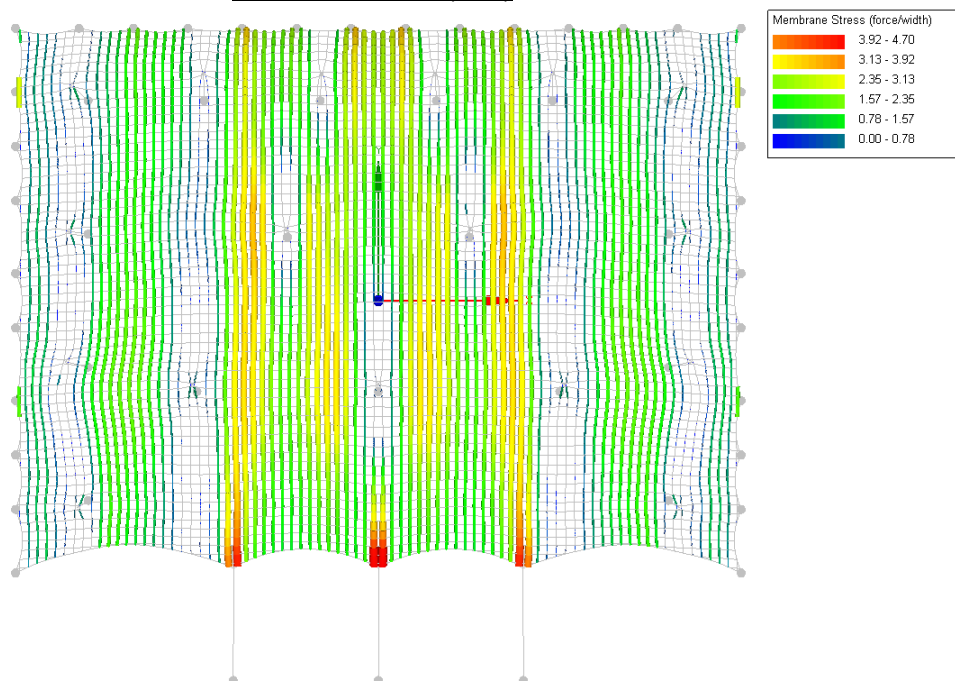
**Annex B.2.4. CO6 Own weight + Pretension + Wind suction – closed – reduction 0.53**

**Annex B.2.4.1. Membrane stress (warp)**



The warp and weft direction of the membrane must be fixed (warp direction along the length of the tent). The membrane stresses are exceeded in the red colored links. However, these are local stresses near the edge, where multiple layers of fabric are present. Therefore, this is acceptable.

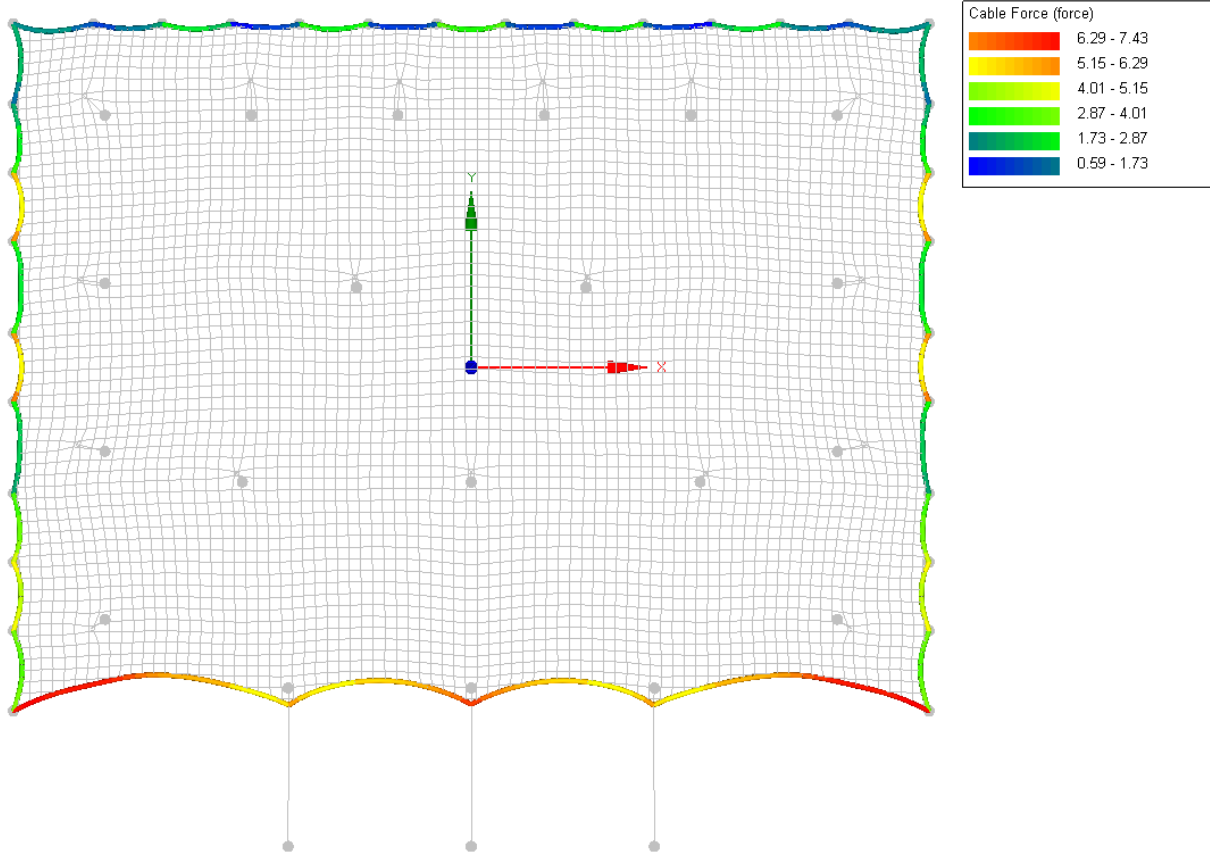
**Annex B.2.4.2. Membrane stress (weft)**



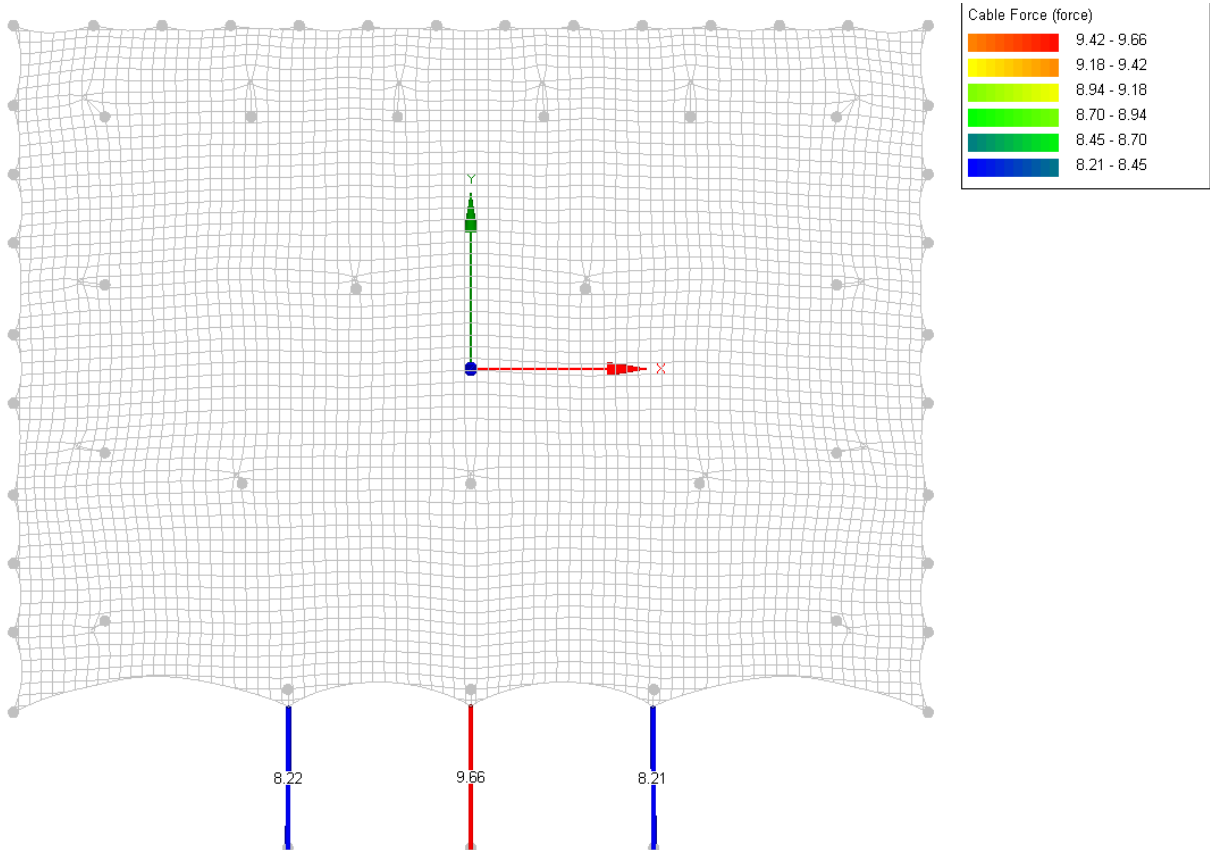
The membrane capacity in weft direction is locally exceeded near the edge. Because multiple layers of fabric are present here, this is acceptable.



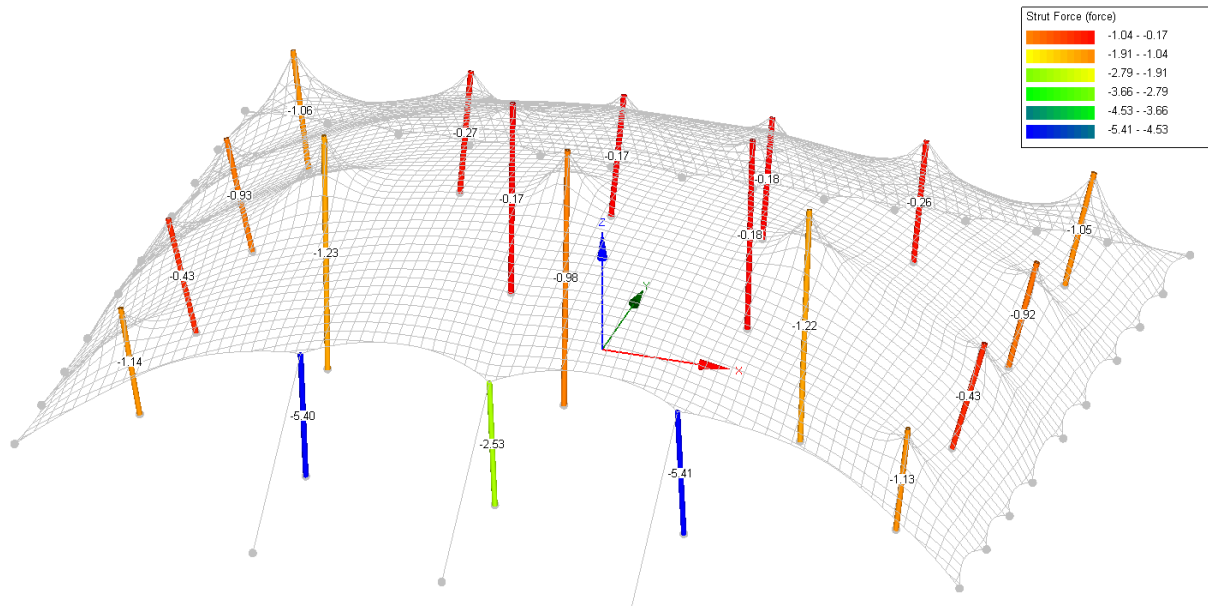
Annex B.2.4.3. Membrane edge



Annex B.2.4.4. Cable forces

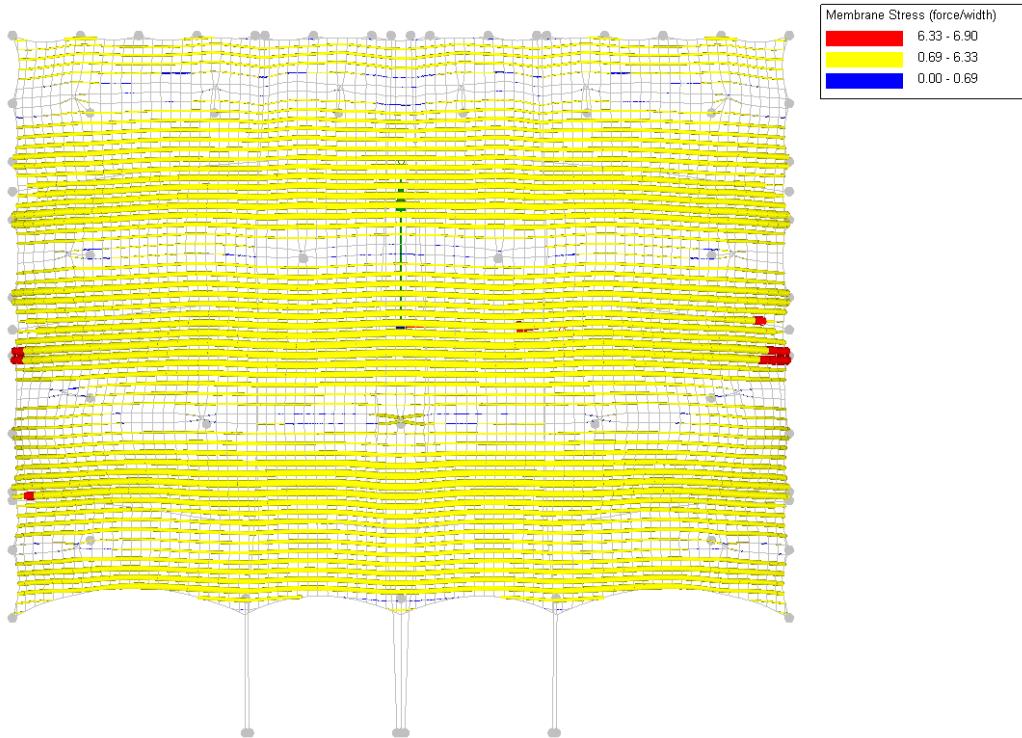


Annex B.2.4.5. Strut forces



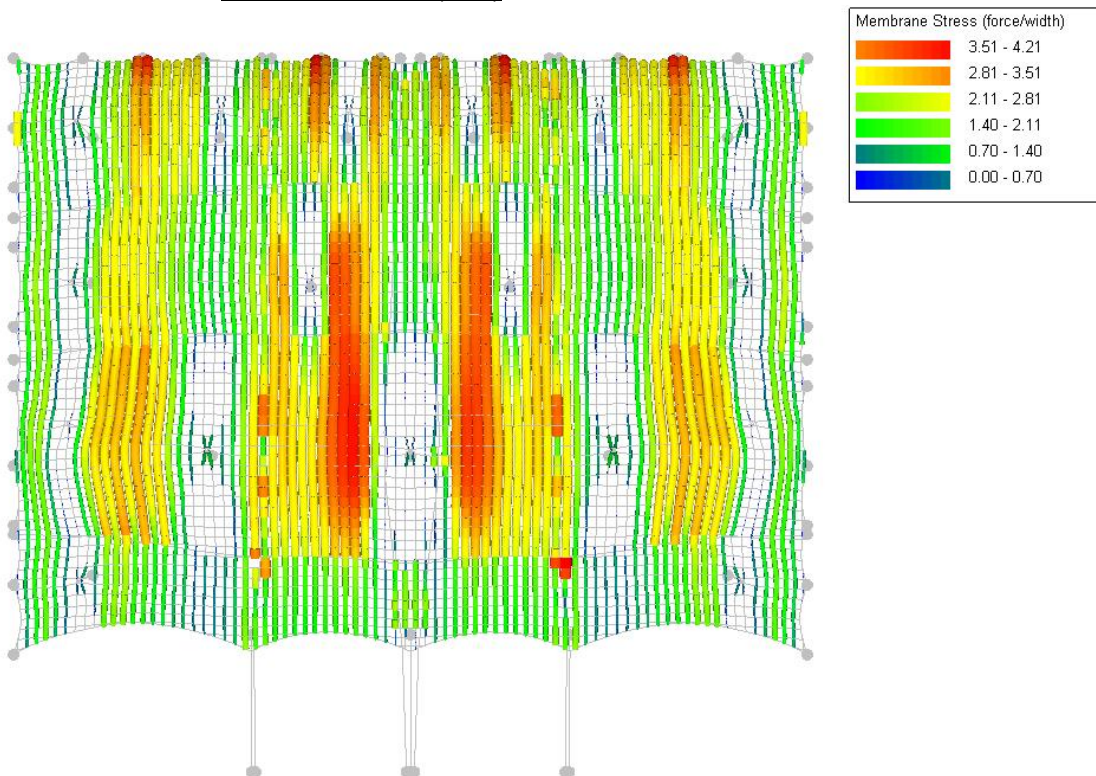
**Annex B.2.5. CO7 Own weight + Pretension + Wind suction – closed – full wind load**

**Annex B.2.5.1. Membrane stress (warp)**

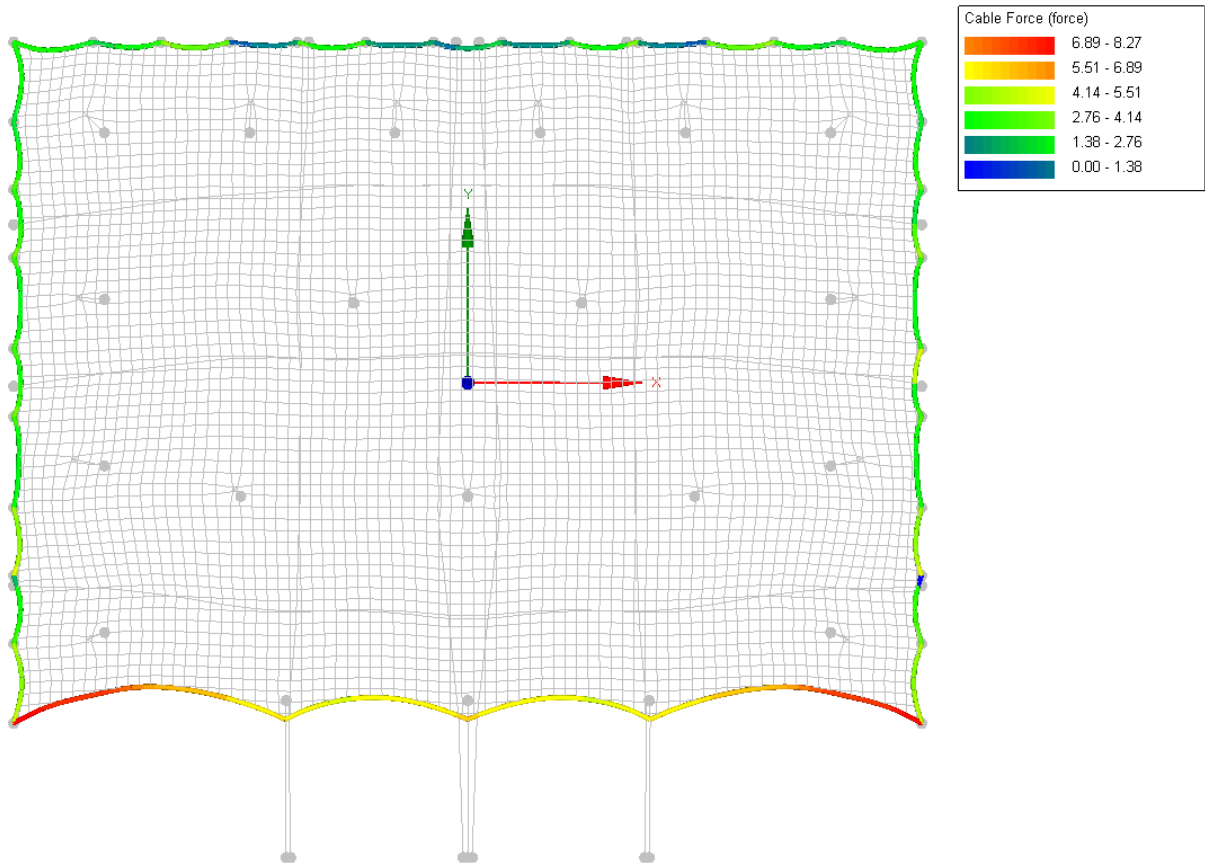


The warp and weft direction of the membrane must be fixed (warp direction along the length of the tent). The membrane stresses are exceeded in the red colored links. However, these are local stresses near the edge, where multiple layers of fabric are present. Therefore, this is acceptable.

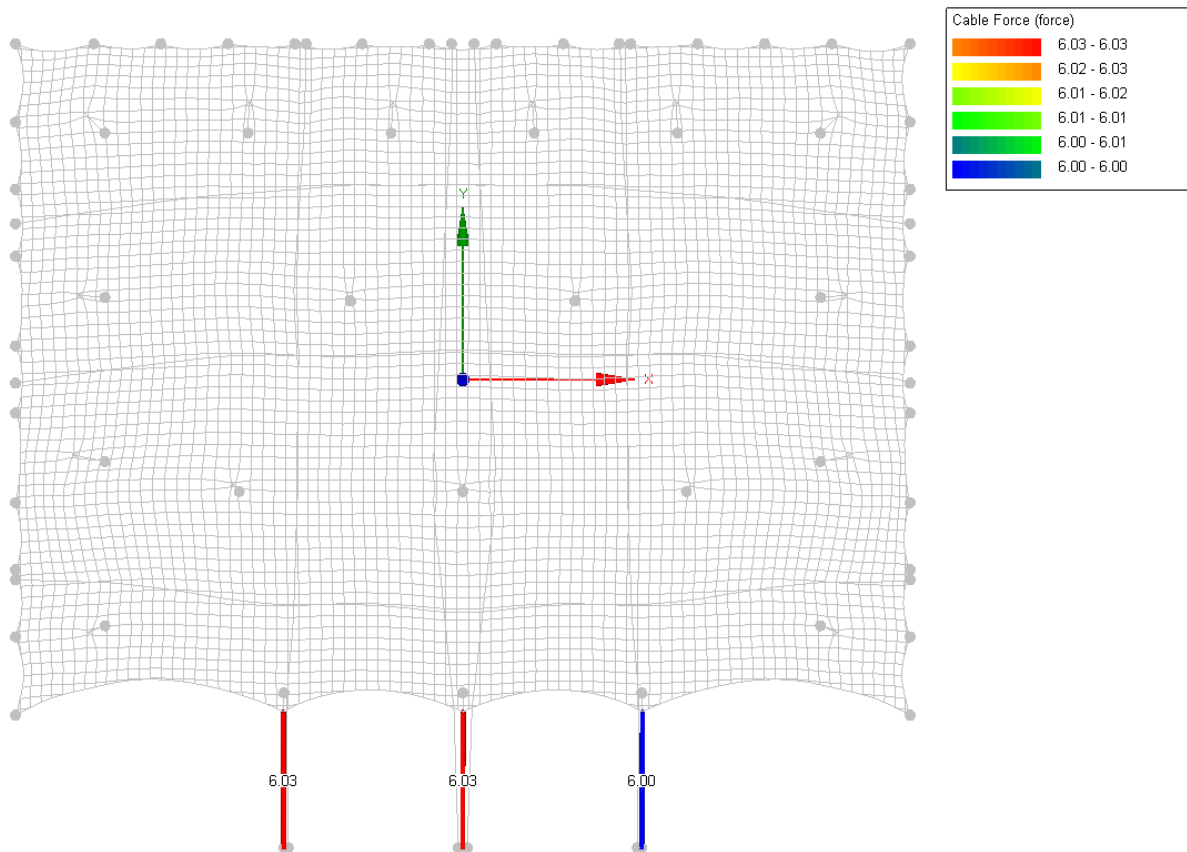
**Annex B.2.5.2. Membrane stress (weft)**



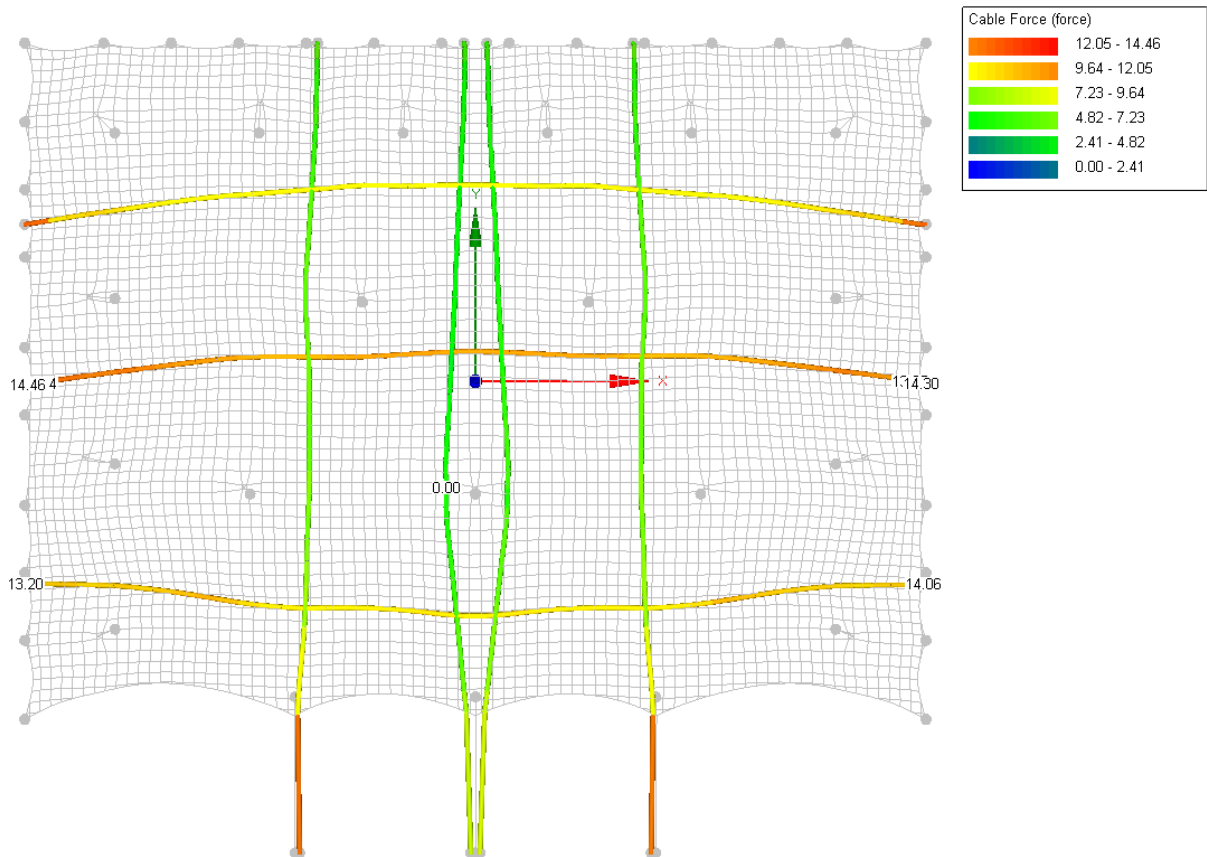
Annex B.2.5.3. Membrane edge



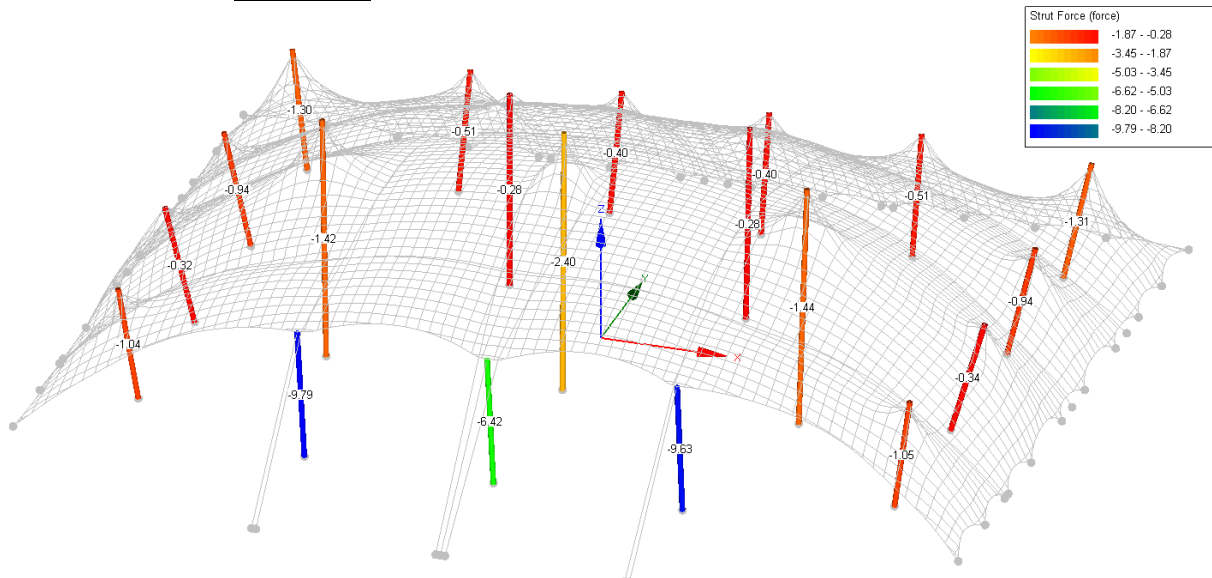
Annex B.2.5.4. Cable forces



Annex B.2.5.5. Storm belts



Annex B.2.5.6. Strut forces



## Annex C. Member checks

### Annex C.1. Center pole 5.5m, floating – wood

Woodtype		Gum pole	
Streight type		<b>D35</b>	
Material factor	$\gamma_M$	1.3	NEN-EN 1995-1-1:2005 table 2.3
Climate class		<b>2</b>	NEN-EN 1995-1-1:2005 article 2.3.1.3
Straightness factor	$\beta_c$	0.2	NEN-EN 1995-1-1:2005 equ. 6.29
Diameter	D	<b>120</b> mm	
Length (buckling)	$l_{buc,y}$	<b>5.5</b> m	
Effective area	A	11309.73355 mm <sup>2</sup>	
Moment of inertia	$I_y$	10178760.2 mm <sup>4</sup>	
Elastic modules	$W_{el,y}$	169646.0033 mm <sup>3</sup>	
Charistic pressure strenght	$f_{c0k}$	25 N/mm <sup>2</sup>	
Charistic bending strenght	$f_{c0k}$	35 N/mm <sup>2</sup>	
Modules of elasticity	$E_{0.05}$	8.7 kN/m <sup>2</sup>	
	$i_y$	30.0 mm	
Slenderness	$\lambda_y$	183.3	
Relative slenderness	$\lambda_{rel,y}$	3.13	NEN-EN 1995-1-1:2005 equ. 6.21
	$k_y$	5.68	NEN-EN 1995-1-1:2005 equ. 6.21
Buckling factor	$k_{cy}$	0.0960	NEN-EN 1995-1-1:2005 equ. 6.25

#### Streight check

Pressure force	$F_d$	<b>16.60</b> kN	
Bending moment	$M_d$	<b>0.00</b> kNm	
Pressure stress	$\sigma_{c0d}$	1.47 N/mm <sup>2</sup>	
Bending stress	$\sigma_{c0d}$	0.00 N/mm <sup>2</sup>	
Load duration		<b>short</b>	NEN-EN 1995-1-1:2005 table 2.1
Modificationfactor	$k_{mod}$	0.90	NEN-EN 1995-1-1:2005 table 3.1
Design pressure strenght	$f_{c0d}$	17.31 N/mm <sup>2</sup>	NEN-EN 1995-1-1:2005 equ. 2.14
Design bending strenght	$f_{m0d}$	24.23 N/mm <sup>2</sup>	NEN-EN 1995-1-1:2005 equ. 2.14
Streight check		<b>0.88</b>	NEN-EN 1995-1-1:2005 equ. 6.23



## Annex C.2. Center pole 5.0m\*, floating – aluminium 90x3mm

Parameters		classification by thickness of round tube	
fo	160 N/mm <sup>2</sup>	t	3 mm
fu	195 N/mm <sup>2</sup>	D	90 mm
E	70000 N/mm <sup>2</sup>		
N	16.60 kN (druk)		
My	0.00 kNm		
Mz	0.00 kNm		
Lcr,y	5000 mm		
Lcr,z	5000 mm		
Iy	776703 mm <sup>4</sup>		
Iz	776703 mm <sup>4</sup>		
ey	45 mm		
ex	45 mm		
Wyel	17260 mm <sup>3</sup>	β	15.43 eq.(6.10)
Wypl	22716 mm <sup>3</sup>	ε	1.25
Wzel	17260 mm <sup>3</sup>	class	2 table (6.2)
Wzpl	22716 mm <sup>3</sup>	class override	Off
Aeff	820 mm <sup>2</sup>		
ym1	1.1		
ym2	1.25		

classification conditions – Table 6.2 – Slenderness parameters	
Class A	β1 < 13.75    β2 < 20.00    β3 < 27.50
class 1	β < β1
class 2	β1 < β < β2
class 3	β2 < β < β3
class 4	β3 < β

Compression art. (6.2.4)	
1 Ned / Nc,Rd < 1	eq. (6.23)
2 Ned / Nu,Rd < 1	eq. (6.21)
Ned	16.60 kN
Nc,Rd	119.27 kN
Nu,Rd	127.91 kN
UC1	0.14
UC2	0.13

Bending Moment art. (6.2.5)	
1 Myed / Myc,Rd < 1	eq. (6.25)
2 Myed / Myu,Rd < 1	eq. (6.24)
3 Mzed / Mzc,Rd < 1	eq. (6.25)
4 Mzed / Mzu,Rd < 1	eq. (6.24)
Myed	0.00 kN
Mzed	0.00 kN
α:y	1.32 table (6.4)
α:z	1.32 table (6.4)
Myc,Rd	3.30 kNm
Myu,Rd	2.69 kNm
Mzc,Rd	3.30 kNm
Mzu,Rd	2.69 kNm
UC1-y	-
UC2-y	-
UC3-z	-
UC4-z	-

Bending and Axial Force art. (6.2.9)	
$\left( \frac{N_{Ed}}{N_{t,Rd}} \right)^2 + \left( \frac{M_{y,Ed}}{M_{y,Rd}} \right)^2 + \left( \frac{M_{z,Ed}}{M_{z,Rd}} \right)^2 \leq 1.00$	
eq. (6.43) - (ω0 = 1) - (ψ = 1.3)	
UC	-

Buckling (compression) art. (6.3.1.1)	
Ned / Nb,Rd < 1	eq. (6.48)
Ned	16.60 kN
BC	A
α	0.20 table (6.6)
λ0	0.10 table (6.6)
χ	0.15 eq. (6.50)
φ	3.79 N
λ	2.47 eq. (6.51)
Ncr	21464.10 (z-axis)
Nb,Rd	17.88 kN
UC	0.93

Buckling (Bending and Axial Force) art. (6.3.3.1)	
$\left( \frac{N_{Ed}}{N_{t,Rd} + N_{b,Rd}} \right)^2 + \left( \frac{M_{y,Ed}}{M_{y,Rd}} \right)^2 + \left( \frac{M_{z,Ed}}{M_{z,Rd}} \right)^2 \leq 1.00$	
eq. (6.62) - (ω0 = 1) - (ωux = 1) - (ψ = 0.8)	
UC	0.94

### Annex C.3. Center pole 5.0m\*, floating – aluminium 76x5mm

Parameters		classification by thickness of round tube	
fo	160 N/mm <sup>2</sup>	t	5 mm
fu	195 N/mm <sup>2</sup>	D	76 mm
E	70000 N/mm <sup>2</sup>		
N	16.60 kN (druk)		
My	0.00 kNm		
Mz	0.00 kNm		
Lcr,y	5000 mm		
Lcr,z	5000 mm		
Iy	706242 mm <sup>4</sup>		
Iz	706242 mm <sup>4</sup>		
ey	38 mm		
ex	38 mm		
Wyel	18585 mm <sup>3</sup>	β	11.70 <small>eq (6.10)</small>
Wyp1	25247 mm <sup>3</sup>	ε	1.25
Wzel	18585 mm <sup>3</sup>	class	1 <small>table (6.2)</small>
Wzpl	25247 mm <sup>3</sup>	class override	Off
Aeff	1115 mm <sup>2</sup>		
vm1	1.1		
vm2	1.25		

classification conditions – Table 6.2 - Slenderness parameters			
Class A	β1	β2	β3
	13.75	20.00	27.50
class 1		True	β < β1
class 2		False	β1 < β < β2
class 3		False	β2 < β < β3
class 4		False	β3 < β

Compression art. (6.2.4)	
1 Ned / Nc,Rd < 1	eq (6.22)
2 Ned / Nu,Rd < 1	eq (6.21)
Ned	16.60 kN
Nc,Rd	162.22 kN
Nu,Rd	173.98 kN
UC1	0.10
UC2	0.10

Bending Moment art. (6.2.5)	
1 Myed / Myc,Rd < 1	eq (6.25)
2 Myed / Myu,Rd < 1	eq (6.24)
3 Mzed / Mzc,Rd < 1	eq (6.25)
4 Mzed / Mzu,Rd < 1	eq (6.24)
Myed	0.00 kN
Mzed	0.00 kN
α-y	1.36 <small>table (6.4)</small>
α-z	1.36 <small>table (6.4)</small>
Myc,Rd	3.67 kNm
Myu,Rd	2.90 kNm
Mzc,Rd	3.67 kNm
Mzu,Rd	2.90 kNm
UC1-y	-
UC2-y	-
UC3-z	-
UC4-z	-

Bending and Axial Force art. (6.2.9)	
$\left( \frac{\Delta F_{Ed,y}}{N_{Ed}} \right)^2 + \left( \frac{M_{Ed,y}}{M_{y,Rd}} \right)^2 + \left( \frac{M_{Ed,z}}{M_{z,Rd}} \right)^2 \leq 1.00$	
eq. (6.43) – (ω <sub>0</sub> = 1) – (ψ = 1.3)	
UC	-

Buckling (compression) art. (6.3.1.1)	
Ned / Nb,Rd < 1 <small>eq. (6.48)</small>	
Ned	16.60 kN
BC	A
α	0.20 <small>table (6.6)</small>
λ <sub>0</sub>	0.10 <small>table (6.6)</small>
χ	0.10 <small>eq. (6.50)</small>
Φ	5.36 N
λ	3.02 <small>eq. (6.51)</small>
Ncr	19516.92 <small>(z-axis)</small>
Nb,Rd	16.56 kN
UC	1.00

Buckling (Bending and Axial Force) art. (6.3.3.1)	
$\left( \frac{N_{Ed}}{N_{cr}} \right)^2 + \frac{1}{\chi^2} \left[ \left( \frac{M_{Ed,y}}{M_{y,Rd}} \right)^2 + \left( \frac{M_{Ed,z}}{M_{z,Rd}} \right)^2 \right] \leq 1.00$	
eq. (6.62) – (ω <sub>0</sub> = 1) – (ω <sub>1</sub> = 1) – (ψ = 0.8)	
UC	1.00



## Annex C.4. Center pole 5.0m, floating – wood

Woodtype		Gum pole	
Strenght type		<b>D35</b>	
Material factor	$\gamma_M$	1.3	NEN-EN 1995-1-1:2005 table 2.3
Climate class		<b>2</b>	NEN-EN 1995-1-1:2005 article 2.3.1.3
Straightness factor	$\beta_c$	0.2	NEN-EN 1995-1-1:2005 equ. 6.29
Diameter	D	<b>110</b> mm	
Length (buckling)	$l_{buc,y}$	<b>5</b> m	
Effective area	A	9503.317777 mm <sup>2</sup>	
Moment of inertia	$I_y$	7186884.069 mm <sup>4</sup>	
Elastic modules	$W_{el,y}$	130670.6194 mm <sup>3</sup>	
Charistic pressure strenght	$f_{c0k}$	25 N/mm <sup>2</sup>	
Charistic bending strenght	$f_{c0k}$	35 N/mm <sup>2</sup>	
Modules of elasticity	$E_{0.05}$	8.7 kN/m <sup>2</sup>	
	$i_y$	27.5 mm	
Slenderness	$\lambda_y$	181.8	
Relative slenderness	$\lambda_{rel,y}$	3.10	NEN-EN 1995-1-1:2005 equ. 6.21
	$k_y$	5.59	NEN-EN 1995-1-1:2005 equ. 6.21
Buckling factor	$k_{cy}$	0.0976	NEN-EN 1995-1-1:2005 equ. 6.25

### Strenght check

Pressure force	$F_d$	<b>14.40</b> kN	
Bending moment	$M_d$	<b>0.00</b> kNm	
Pressure stress	$\sigma_{c0d}$	1.52 N/mm <sup>2</sup>	
Bending stress	$\sigma_{c0d}$	0.00 N/mm <sup>2</sup>	
Load duration		<b>short</b>	NEN-EN 1995-1-1:2005 table 2.1
Modificationfactor	$k_{mod}$	0.90	NEN-EN 1995-1-1:2005 table 3.1
Design pressure strenght	$f_{c0d}$	17.31 N/mm <sup>2</sup>	NEN-EN 1995-1-1:2005 equ. 2.14
Design bending strenght	$f_{m0d}$	24.23 N/mm <sup>2</sup>	NEN-EN 1995-1-1:2005 equ. 2.14
Strenght check		<b>0.90</b>	NEN-EN 1995-1-1:2005 equ. 6.23

### Annex C.5. Center pole 5.0m, floating – aluminium 90x3mm

Bending and Axial Force art. (6.2.9)

$$\left( \frac{N_{Ed}}{N_{Rd}} \right)^{\psi} + \left( \frac{M_{y,Ed}}{M_{y,Rd}} \right)^{\psi} + \left( \frac{M_{z,Ed}}{M_{z,Rd}} \right)^{\psi} \leq 1.000$$

eq. (6.43) - ( $\omega_0 = 1$ ) - ( $\psi = 1.3$ )

UC  **0.81**

*Checks not necessary for bending moments*

Buckling (compression) art. (6.3.1.1)

Ned / Nb,Rd < 1 ..... eq. (6.48)

Ned ..... 14.40 kN

BC ..... A

$\alpha$  ..... 0.20 table (6.6)

$\lambda_0$  ..... 0.10 table (6.6)

$\chi$  ..... 0.15 eq. (6.50)

$\Phi$  ..... 3.79 N

$\lambda$  ..... 2.47 eq. (6.51)

Ncr ..... 21464.10 (z-axis)

Nb,Rd ..... 17.88 kN

UC  **0.81**

Buckling (Bending and Axial Force) art. (6.3.3.1)

$$\left( \frac{N_{Ed}}{N_{cr}} \right)^{\psi} + \frac{1}{\omega_0} \left[ \frac{M_{y,Ed}}{M_{y,Rd}} \right]^{\psi} + \left( \frac{M_{z,Ed}}{M_{z,Rd}} \right)^{\psi} \leq 1.000$$

eq. (6.62) - ( $\omega_0 = 1$ ) - ( $\psi = 0.8$ )

UC  **0.84**

*Checks not necessary for bending moments*

Compression art. (6.2.4)

1 Ned / Nc,Rd < 1 ..... eq. (6.22)

2 Ned / Nu,Rd < 1 ..... eq. (6.21)

Ned ..... 14.40 kN

Nc,Rd ..... 119.27 kN

Nu,Rd ..... 127.91 kN

UC1  **0.12**

UC2  **0.11**

Bending Moment art. (6.2.5)

1 Myed / Myc,Rd < 1 ..... eq. (6.25)

2 Myed / Myu,Rd < 1 ..... eq. (6.24)

3 Mzed / Mzc,Rd < 1 ..... eq. (6.25)

4 Mzed / Mzu,Rd < 1 ..... eq. (6.24)

Myed ..... 0.00 kN

Mzed ..... 0.00 kN

$\alpha_y$  ..... 1.32 table (6.4)

$\alpha_z$  ..... 1.32 table (6.4)

Myc,Rd ..... 3.30 kNm

Myu,Rd ..... 2.69 kNm

Mzc,Rd ..... 3.30 kNm

Mzu,Rd ..... 2.69 kNm

UC1-y  -

UC2-y  -

UC3-z  -

UC4-z  -

*Checks not necessary for bending moments*

Parameters

fo ..... 160 N/mm<sup>2</sup>

fu ..... 195 N/mm<sup>2</sup>

E ..... 70000 N/mm<sup>2</sup>

N ..... 14.40 kN (druk)

My ..... 0.00 kNm

Mz ..... 0.00 kNm

Lcr,y ..... 5000 mm

Lcr,z ..... 5000 mm

Iy ..... 776703 mm<sup>4</sup>

Iz ..... 776703 mm<sup>4</sup>

ey ..... 45 mm

ex ..... 45 mm

Wypl ..... 17260 mm<sup>3</sup>

Wypl ..... 22716 mm<sup>3</sup>

Wzpl ..... 17260 mm<sup>3</sup>

Wzpl ..... 22716 mm<sup>3</sup>

Aeff ..... 820 mm<sup>2</sup>

ym1 ..... 1.1

ym2 ..... 1.25

classification by thickness of round tube

t ..... 3 mm

D ..... 90 mm

$\beta$  ..... 16.43 eq. (6.10)

$\epsilon$  ..... 1.25

class ..... 2 table (6.2)

class override ..... Off

classification conditions - Table 6.2 - Slenderness parameters

Class A	$\beta_1$	$\beta_2$	$\beta_3$
	13.75	20.00	27.50
class 1	False	False	$\beta < \beta_1$
class 2	True	True	$\beta_1 < \beta < \beta_2$
class 3	False	False	$\beta_2 < \beta < \beta_3$
class 4	False	False	$\beta_3 < \beta$

### Annex C.6. Center pole 5.0m, floating – aluminium 76x4mm

Bending and Axial Force art. (6.2.9)

$$\left(\frac{N_{Ed}}{\alpha_{M1} N_{Rd}}\right)^{\psi} + \left(\frac{M_{y,Ed}}{\alpha_{M2} M_{y,Rd}}\right)^{\psi} + \left(\frac{M_{z,Ed}}{\alpha_{M3} M_{z,Rd}}\right)^{\psi} \leq 1.00$$

eq. (6.43) - ( $\omega_0 = 1$ ) - ( $\psi = 1.3$ )

UC **1**

*Check not necessary, no bending moments*

Buckling (compression) art. (6.3.1.1)

Ned / Nb,Rd < 1 ..... eq. (6.48)

Ned ..... 14.40 kN

BC ..... A

$\alpha$  ..... 0.20 table (6.6)

$\lambda_0$  ..... 0.10 table (6.6)

X ..... 0.10 eq. (6.50)

$\phi$  ..... 5.24 N

$\lambda$  ..... 2.98 eq. (6.51)

Ncr ..... 16252.25 (z-axis)

Nb,Rd ..... 13.78 kN

UC **8** 1.05

Buckling (Bending and Axial Force) art. (6.3.3.1)

$$\left(\frac{N_{Ed}}{X_{rel} \alpha_{M1} N_{Rd}}\right)^{\psi} + 1 - \left(\frac{M_{y,Ed}}{M_{y,Rd}}\right)^{\psi} + \left(\frac{M_{z,Ed}}{M_{z,Rd}}\right)^{\psi} \leq 1.00$$

eq. (6.62) - ( $\omega_0 = 1$ ) - ( $\psi = 0.8$ )

UC **8** 1.04

*Check not necessary, no bending moments*

Compression art. (6.2.4)

1 Ned / Nc,Rd < 1 ..... eq. (6.22)

2 Ned / Nu,Rd < 1 ..... eq. (6.21)

Ned ..... 14.40 kN

Nc,Rd ..... 131.60 kN

Nu,Rd ..... 141.15 kN

UC1 **1** 0.11

UC2 **1** 0.10

Bending Moment art. (6.2.5)

1 Myed / Myc,Rd < 1 ..... eq. (6.25)

2 Myed / Myu,Rd < 1 ..... eq. (6.24)

3 Mzed / Mzc,Rd < 1 ..... eq. (6.25)

4 Mzed / Mzu,Rd < 1 ..... eq. (6.24)

Myed ..... 0.00 kN

Mzed ..... 0.00 kN

$\alpha_1 \gamma$  ..... 1.34 table (6.4)

$\alpha_2 z$  ..... 1.34 table (6.4)

Myc,Rd ..... 3.02 kNm

Myu,Rd ..... 2.41 kNm

Mzc,Rd ..... 3.02 kNm

Mzu,Rd ..... 2.41 kNm

UC1-Y ..... -

UC2-Y ..... -

UC3-z ..... -

UC4-z ..... -

*Check not necessary, no bending moments*

Parameters

fo ..... 160 N/mm<sup>2</sup>

fu ..... 195 N/mm<sup>2</sup>

E ..... 70000 N/mm<sup>2</sup>

N ..... 14.40 kN (druk)

My ..... 0.00 kNm

Mz ..... 0.00 kNm

Lcr,y ..... 5000 mm

Lcr,z ..... 5000 mm

Iy ..... 588106 mm<sup>4</sup>

Iz ..... 588106 mm<sup>4</sup>

ey ..... 38 mm

ex ..... 38 mm

Wyel ..... 15476 mm<sup>3</sup>

Wypl ..... 20757 mm<sup>3</sup>

Wzel ..... 15476 mm<sup>3</sup>

Wzpl ..... 20757 mm<sup>3</sup>

Aeff ..... 905 mm<sup>2</sup>

ym1 ..... 1.1

ym2 ..... 1.25

classification by thickness of round tube

t ..... 4 mm

D ..... 76 mm

$\beta$  ..... 13.08 eq. (6.10)

$\epsilon$  ..... 1.25

class ..... 1 table (6.4)

class override ..... **Off**

classification conditions - Table 6.2 - Slenderness parameters

	$\beta_1$	$\beta_2$	$\beta_3$
Class A	13.75	20.00	27.50
Class 1	True	True	$\beta < \beta_1$
Class 2	False	False	$\beta_1 < \beta < \beta_2$
Class 3	False	False	$\beta_2 < \beta < \beta_3$
Class 4	False	False	$\beta_3 < \beta$

## Annex C.7. Center pole 5.5m, closed – wood

Woodtype		Gum pole	
Strenght type		<b>D35</b>	
Material factor	$\gamma_M$	1.3	NEN-EN 1995-1-1:2005 table 2.3
Climate class		<b>2</b>	NEN-EN 1995-1-1:2005 article 2.3.1.3
Straightness factor	$\beta_c$	0.2	NEN-EN 1995-1-1:2005 equ. 6.29
Diameter	D	<b>110</b> mm	
Length (buckling)	$l_{buc,y}$	<b>5.5</b> m	
Effective area	A	9503.317777 mm <sup>2</sup>	
Moment of inertia	$I_y$	7186884.069 mm <sup>4</sup>	
Elastic modules	$W_{el,y}$	130670.6194 mm <sup>3</sup>	
Charistic pressure strenght	$f_{c0k}$	25 N/mm <sup>2</sup>	
Charistic bending strenght	$f_{c0k}$	35 N/mm <sup>2</sup>	
Modules of elasticity	$E_{0.05}$	8.7 kN/m <sup>2</sup>	
	$i_y$	27.5 mm	
Slenderness	$\lambda_y$	200.0	
Relative slenderness	$\lambda_{rel,y}$	3.41	NEN-EN 1995-1-1:2005 equ. 6.21
	$k_y$	6.63	NEN-EN 1995-1-1:2005 equ. 6.21
Buckling factor	$k_{cy}$	0.08115	NEN-EN 1995-1-1:2005 equ. 6.25

### Strenght check

Pressure force	$F_d$	<b>11.30</b> kN	
Bending moment	$M_d$	<b>0.00</b> kNm	
Pressure stress	$\sigma_{c0d}$	1.19 N/mm <sup>2</sup>	
Bending stress	$\sigma_{c0d}$	0.00 N/mm <sup>2</sup>	
Load duration		<b>short</b>	NEN-EN 1995-1-1:2005 table 2.1
Modificationfactor	$k_{mod}$	0.90	NEN-EN 1995-1-1:2005 table 3.1
Design pressure strenght	$f_{c0d}$	17.31 N/mm <sup>2</sup>	NEN-EN 1995-1-1:2005 equ. 2.14
Design bending strenght	$f_{m0d}$	24.23 N/mm <sup>2</sup>	NEN-EN 1995-1-1:2005 equ. 2.14
Strenght check		<b>0.85</b>	NEN-EN 1995-1-1:2005 equ. 6.23

### Annex C.8. Center pole 5.5m, closed – aluminium 90x3mm

<p>Parameters</p> <p>fo 160 N/mm<sup>2</sup></p> <p>fu 195 N/mm<sup>2</sup></p> <p>E 70000 N/mm<sup>2</sup></p> <p>N 11.30 kN (druk)</p> <p>My 0.00 kNm</p> <p>Mz 0.00 kNm</p> <p>Lcr,y 5500 mm</p> <p>Lcr,z 5500 mm</p> <p>Iy 776703 mm<sup>4</sup></p> <p>Iz 776703 mm<sup>4</sup></p> <p>ey 45 mm</p> <p>ex 45 mm</p> <p>Wypl 17260 mm<sup>3</sup></p> <p>Wypl 22716 mm<sup>3</sup></p> <p>Wzpl 17260 mm<sup>3</sup></p> <p>Wzpl 22716 mm<sup>3</sup></p> <p>Aeff 820 mm<sup>2</sup></p> <p>ym1 1.1</p> <p>ym2 1.25</p>		<p>classification by thickness of round tube</p> <p>t 3 mm</p> <p>D 90 mm</p>																									
<p>class override</p>		<p>class 2</p>																									
<p>class override</p>		<p>Off</p>																									
<p>classification conditions - Table 6.2 - Slenderness parameters</p> <table border="1"> <thead> <tr> <th>Class</th> <th>β1</th> <th>β2</th> <th>β3</th> </tr> </thead> <tbody> <tr> <td>Class A</td> <td>13.75</td> <td>20.00</td> <td>27.50</td> </tr> <tr> <td>class 1</td> <td>False</td> <td>False</td> <td>β &lt; β1</td> </tr> <tr> <td>class 2</td> <td>True</td> <td>True</td> <td>β1 &lt; β &lt; β2</td> </tr> <tr> <td>class 3</td> <td>False</td> <td>False</td> <td>β2 &lt; β &lt; β3</td> </tr> <tr> <td>class 4</td> <td>False</td> <td>False</td> <td>β3 &lt; β</td> </tr> </tbody> </table>				Class	β1	β2	β3	Class A	13.75	20.00	27.50	class 1	False	False	β < β1	class 2	True	True	β1 < β < β2	class 3	False	False	β2 < β < β3	class 4	False	False	β3 < β
Class	β1	β2	β3																								
Class A	13.75	20.00	27.50																								
class 1	False	False	β < β1																								
class 2	True	True	β1 < β < β2																								
class 3	False	False	β2 < β < β3																								
class 4	False	False	β3 < β																								

Compression art. (6.2.4)

1 Ned / Nc,Rd < 1 **eq (6.22)**

2 Ned / Nu,Rd < 1 **eq (6.21)**

Ned 11.30 kN

Nc,Rd 119.27 kN

Nu,Rd 127.91 kN

UC1  0.09

UC2  0.09

Bending and Axial Force art. (6.2.9)

$$\left( \frac{\Delta F_{Ed}}{N_{Ed}} \right)^2 + \left( \frac{M_{y,Ed}}{M_{y,Rd}} \right)^2 + \left( \frac{M_{z,Ed}}{M_{z,Rd}} \right)^2 \leq 1.000$$

eq. (6.43) - (ω0 = 1) - (ψ = 1.3)

UC

*Check not necessary, no bending moments*

Buckling (compression) art. (6.3.1.1)

Ned / Nb,Rd < 1 **eq. (6.48)**

Ned 11.30 kN

BC A

α 0.20 table (6.6)

λ0 0.10 table (6.6)

χ 0.13 eq. (6.50)

φ 4.45 N

λ 2.72 eq. (6.51)

Ncr 17738.93 (z-axis)

Nb,Rd 14.92 kN

UC  0.76

Bending Moment art. (6.2.5)

1 Myed / Myc,Rd < 1 **eq (6.25)**

2 Myed / Myu,Rd < 1 **eq (6.24)**

3 Mzed / Mzc,Rd < 1 **eq (6.25)**

4 Mzed / Mzu,Rd < 1 **eq (6.24)**

Myed 0.00 kN

Mzed 0.00 kN

αy 1.32 table (6.4)

αz 1.32 table (6.4)

Myc,Rd 3.30 kNm

Myu,Rd 2.69 kNm

Mzc,Rd 3.30 kNm

Mzu,Rd 2.69 kNm

UC1-y -

UC2-y -

UC3-z -

UC4-z -

*Checks not necessary, no bending moments*

Buckling (Bending and Axial Force) art. (6.3.3.1)

$$\left( \frac{N_{Ed}}{N_{cr,0.9} N_{Rd}} \right)^2 + \frac{1}{\omega_0} \left[ \left( \frac{M_{y,Ed}}{M_{y,Rd}} \right)^2 + \left( \frac{M_{z,Ed}}{M_{z,Rd}} \right)^2 \right]^{1.75} \leq 1.000$$

eq. (6.62) - (ω0 = 1) - (ω0 = 1) - (ψ = 0.8)

UC  0.80

### Annex C.9. Center pole 5.5m, closed – aluminium 76x4mm

**Parameters**

fo	160 N/mm <sup>2</sup>
fu	195 N/mm <sup>2</sup>
E	70000 N/mm <sup>2</sup>
N	11.30 kN (druk)
My	0.00 kNm
Mz	0.00 kNm
Lcr-y	5500 mm
Lcr-z	5500 mm
Iy	588106 mm <sup>4</sup>
Iz	588106 mm <sup>4</sup>
ey	38 mm
ex	38 mm
Wypl	15476 mm <sup>3</sup>
Wzpl	20757 mm <sup>3</sup>
Wzel	15476 mm <sup>3</sup>
Wzpl	20757 mm <sup>3</sup>
Aeff	905 mm <sup>2</sup>
ym1	1.1
ym2	1.25

classification by thickness of round tube

t	4 mm
D	76 mm

classification conditions - Table 6.2 - Slenderness parameters

Class A	β1	β2	β3
	13.75	20.00	27.50
class 1		True	β < β1
class 2		False	β1 < β < β2
class 3		False	β2 < β < β3
class 4		False	β3 < β

**Bending and Axial Force art. (6.2.9)**

$$\left( \frac{N_{Ed}}{N_{Rd}} \right)^{\psi} + \left( \frac{M_{y,Ed}}{M_{y,Rd}} \right)^{\psi} + \left( \frac{M_{z,Ed}}{M_{z,Rd}} \right)^{\psi} \leq 1.000$$

eq. (6.43) - (ω0 = 1) - (ψ = 1.3)

UC -  
*Check not necessary, no bending moments*

**Buckling (compression) art. (6.3.1.1)**

Ned / Nb,Rd < 1 eq. (6.48)

Ned ..... 11.30 kN

BC ..... A

α ..... 0.20 table (6.6)

λ0 ..... 0.10 table (6.6)

χ ..... 0.09 eq. (6.50)

φ ..... 6.21 N

λ ..... 3.28 eq. (6.51)

Ncr ..... 13431.61 (z-axis)

Nb,Rd ..... 11.47 kN

UC 0.99

**Buckling (Bending and Axial Force) art. (6.3.3.1)**

$$\left( \frac{N_{Ed}}{N_{Ed,lim}} \right)^{\psi_{Nt}} + \frac{1}{\omega_0} \left[ \left( \frac{M_{y,Ed}}{M_{y,Rd}} \right)^{\psi} + \left( \frac{M_{z,Ed}}{M_{z,Rd}} \right)^{\psi} \right] \leq 1.000$$

eq. (6.62) - (ω0 = 1) - (ωxt = 1) - (ψ = 0.8)

UC 0.99  
*Check not necessary, no bending moments*

**Compression art. (6.2.4)**

1 Ned / Nc,Rd < 1 eq. (6.22)

2 Ned / Nu,Rd < 1 eq. (6.21)

Ned ..... 11.30 kN

Nc,Rd ..... 131.60 kN

Nu,Rd ..... 141.15 kN

UC1 0.09

UC2 0.08

**Bending Moment art. (6.2.5)**

1 Myed / Myc,Rd < 1 eq. (6.25)

2 Myed / Myu,Rd < 1 eq. (6.24)

3 Mzed / Mzc,Rd < 1 eq. (6.25)

4 Mzed / Mzu,Rd < 1 eq. (6.24)

Myed ..... 0.00 kN

Mzed ..... 0.00 kN

α-y ..... 1.34 table (6.4)

α-z ..... 1.34 table (6.4)

Myc,Rd ..... 3.02 kNm

Myu,Rd ..... 2.41 kNm

Mzc,Rd ..... 3.02 kNm

Mzu,Rd ..... 2.41 kNm

UC1-y -

UC2-y -

UC3-z -

UC4-z -  
*Check not necessary, no bending moments*

## Annex C.10. Center pole 5.0m, closed – wood

Woodtype		Gum pole	
Strenght type		<b>D35</b>	
Material factor	$\gamma_M$	1.3	NEN-EN 1995-1-1:2005 table 2.3
Climate class		<b>2</b>	NEN-EN 1995-1-1:2005 article 2.3.1.3
Straightness factor	$\beta_c$	0.2	NEN-EN 1995-1-1:2005 equ. 6.29
Diameter	D	<b>110</b> mm	
Length (buckling)	$l_{buc,y}$	<b>5</b> m	
Effective area	A	9503.317777 mm <sup>2</sup>	
Moment of inertia	$I_y$	7186884.069 mm <sup>4</sup>	
Elastic modules	$W_{el,y}$	130670.6194 mm <sup>3</sup>	
Charistic pressure strenght	$f_{c0k}$	25 N/mm <sup>2</sup>	
Charistic bending strenght	$f_{c0k}$	35 N/mm <sup>2</sup>	
Modules of elasticity	$E_{0.05}$	8.7 kN/m <sup>2</sup>	
	$i_y$	27.5 mm	
Slenderness	$\lambda_y$	181.8	
Relative slenderness	$\lambda_{rel,y}$	3.10	NEN-EN 1995-1-1:2005 equ. 6.21
	$k_y$	5.59	NEN-EN 1995-1-1:2005 equ. 6.21
Buckling factor	$k_{cy}$	0.09760	NEN-EN 1995-1-1:2005 equ. 6.25

### Strenght check

Pressure force	$F_d$	<b>12.70</b> kN	
Bending moment	$M_d$	<b>0.00</b> kNm	
Pressure stress	$\sigma_{c0d}$	1.34 N/mm <sup>2</sup>	
Bending stress	$\sigma_{c0d}$	0.00 N/mm <sup>2</sup>	
Load duration		<b>short</b>	NEN-EN 1995-1-1:2005 table 2.1
Modificationfactor	$k_{mod}$	0.90	NEN-EN 1995-1-1:2005 table 3.1
Design pressure strenght	$f_{c0d}$	17.31 N/mm <sup>2</sup>	NEN-EN 1995-1-1:2005 equ. 2.14
Design bending strenght	$f_{m0d}$	24.23 N/mm <sup>2</sup>	NEN-EN 1995-1-1:2005 equ. 2.14
Strenght check		<b>0.79</b>	NEN-EN 1995-1-1:2005 equ. 6.23

### Annex C.11. Center pole 5.0m, closed – Aluminium 90x3mm

<p><b>Parameters</b></p> <table style="width: 100%; border-collapse: collapse;"> <tr><td>fo</td><td>160 N/mm<sup>2</sup></td></tr> <tr><td>fu</td><td>195 N/mm<sup>2</sup></td></tr> <tr><td>E</td><td>70000 N/mm<sup>2</sup></td></tr> <tr><td>N</td><td>12.70 kN (druk)</td></tr> <tr><td>My</td><td>0.00 kNm</td></tr> <tr><td>Mz</td><td>0.00 kNm</td></tr> <tr><td>Lcr,y</td><td>5000 mm</td></tr> <tr><td>Lcr,z</td><td>5000 mm</td></tr> <tr><td>Iy</td><td>776703 mm<sup>4</sup></td></tr> <tr><td>Iz</td><td>776703 mm<sup>4</sup></td></tr> <tr><td>ey</td><td>45 mm</td></tr> <tr><td>ex</td><td>45 mm</td></tr> <tr><td>Wyel</td><td>17260 mm<sup>3</sup></td></tr> <tr><td>Wyp1</td><td>22716 mm<sup>3</sup></td></tr> <tr><td>Wzel</td><td>17260 mm<sup>3</sup></td></tr> <tr><td>Wzpl</td><td>22716 mm<sup>3</sup></td></tr> <tr><td>Aeff</td><td>820 mm<sup>2</sup></td></tr> <tr><td>ym1</td><td>1.1</td></tr> <tr><td>ym2</td><td>1.25</td></tr> </table> <p style="text-align: center;">classification by thickness of round tube</p> <table style="width: 100%; border-collapse: collapse;"> <tr><td>t</td><td>3 mm</td></tr> <tr><td>D</td><td>90 mm</td></tr> </table>	fo	160 N/mm <sup>2</sup>	fu	195 N/mm <sup>2</sup>	E	70000 N/mm <sup>2</sup>	N	12.70 kN (druk)	My	0.00 kNm	Mz	0.00 kNm	Lcr,y	5000 mm	Lcr,z	5000 mm	Iy	776703 mm <sup>4</sup>	Iz	776703 mm <sup>4</sup>	ey	45 mm	ex	45 mm	Wyel	17260 mm <sup>3</sup>	Wyp1	22716 mm <sup>3</sup>	Wzel	17260 mm <sup>3</sup>	Wzpl	22716 mm <sup>3</sup>	Aeff	820 mm <sup>2</sup>	ym1	1.1	ym2	1.25	t	3 mm	D	90 mm	<table style="width: 100%; border-collapse: collapse;"> <tr><td>1 Ned / Nc,Rd &lt; 1</td><td>eq (6.22)</td></tr> <tr><td>2 Ned / Nu,Rd &lt; 1</td><td>eq (6.21)</td></tr> <tr><td>Ned</td><td>12.70 kN</td></tr> <tr><td>Nc,Rd</td><td>119.27 kN</td></tr> <tr><td>Nu,Rd</td><td>127.91 kN</td></tr> <tr><td>UC1</td><td>✓ 0.11</td></tr> <tr><td>UC2</td><td>✓ 0.10</td></tr> </table> <p><b>Compression art. (6.2.4)</b></p>	1 Ned / Nc,Rd < 1	eq (6.22)	2 Ned / Nu,Rd < 1	eq (6.21)	Ned	12.70 kN	Nc,Rd	119.27 kN	Nu,Rd	127.91 kN	UC1	✓ 0.11	UC2	✓ 0.10
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### Annex C.12. Center pole 5.0m, closed – Aluminium 76x4mm

<p><b>Parameters</b></p> <table style="width: 100%; border-collapse: collapse;"> <tr><td>fo</td><td>160 N/mm<sup>2</sup></td></tr> <tr><td>fu</td><td>195 N/mm<sup>2</sup></td></tr> <tr><td>E</td><td>70000 N/mm<sup>2</sup></td></tr> <tr><td>N</td><td>12.70 kN (druk)</td></tr> <tr><td>My</td><td>0.00 kNm</td></tr> <tr><td>Mz</td><td>0.00 kNm</td></tr> <tr><td>Lcry</td><td>5000 mm</td></tr> <tr><td>Lcrz</td><td>5000 mm</td></tr> <tr><td>Iy</td><td>588106 mm<sup>4</sup></td></tr> <tr><td>Iz</td><td>588106 mm<sup>4</sup></td></tr> <tr><td>ey</td><td>38 mm</td></tr> <tr><td>ex</td><td>38 mm</td></tr> <tr><td>Wyel</td><td>15476 mm<sup>3</sup></td></tr> <tr><td>Wypl</td><td>20757 mm<sup>3</sup></td></tr> <tr><td>Wzel</td><td>15476 mm<sup>3</sup></td></tr> <tr><td>Wzpl</td><td>20757 mm<sup>3</sup></td></tr> <tr><td>Aeff</td><td>905 mm<sup>2</sup></td></tr> <tr><td>ym1</td><td>1.1</td></tr> <tr><td>ym2</td><td>1.25</td></tr> </table>	fo	160 N/mm <sup>2</sup>	fu	195 N/mm <sup>2</sup>	E	70000 N/mm <sup>2</sup>	N	12.70 kN (druk)	My	0.00 kNm	Mz	0.00 kNm	Lcry	5000 mm	Lcrz	5000 mm	Iy	588106 mm <sup>4</sup>	Iz	588106 mm <sup>4</sup>	ey	38 mm	ex	38 mm	Wyel	15476 mm <sup>3</sup>	Wypl	20757 mm <sup>3</sup>	Wzel	15476 mm <sup>3</sup>	Wzpl	20757 mm <sup>3</sup>	Aeff	905 mm <sup>2</sup>	ym1	1.1	ym2	1.25	<p>classification by thickness of round tube</p> <table style="width: 100%; border-collapse: collapse;"> <tr><td>t</td><td>4 mm</td></tr> <tr><td>D</td><td>76 mm</td></tr> </table> <table style="width: 100%; border-collapse: collapse;"> <tr><td>β</td><td>13.08</td><td>eq. (6.10)</td></tr> <tr><td>ε</td><td>1.25</td><td></td></tr> </table> <table style="width: 100%; border-collapse: collapse;"> <tr><td>class</td><td>1</td><td>table (6.2)</td></tr> </table> <p style="text-align: right; color: red;">Off</p>	t	4 mm	D	76 mm	β	13.08	eq. (6.10)	ε	1.25		class	1	table (6.2)	<p>classification conditions - Table 6.2 - Slenderness parameters</p> <table border="1" style="width: 100%; border-collapse: collapse; text-align: center;"> <tr><td>Class A</td><td>B1</td><td>B2</td><td>B3</td></tr> <tr><td></td><td>13.75</td><td>20.00</td><td>27.50</td></tr> <tr><td>class 1</td><td></td><td>True</td><td>B &lt; B1</td></tr> <tr><td>class 2</td><td></td><td>False</td><td>B1 &lt; B &lt; B2</td></tr> <tr><td>class 3</td><td></td><td>False</td><td>B2 &lt; B &lt; B3</td></tr> <tr><td>class 4</td><td></td><td>False</td><td>B3 &lt; B</td></tr> </table>	Class A	B1	B2	B3		13.75	20.00	27.50	class 1		True	B < B1	class 2		False	B1 < B < B2	class 3		False	B2 < B < B3	class 4		False	B3 < B
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class 3		False	B2 < B < B3																																																																										
class 4		False	B3 < B																																																																										
<p><b>Compression art. (6.2.4)</b></p> <table style="width: 100%; border-collapse: collapse;"> <tr><td>1 Ned / Nc,Rd &lt; 1</td><td>eq. (6.22)</td></tr> <tr><td>2 Ned / Nu,Rd &lt; 1</td><td>eq. (6.21)</td></tr> </table> <table style="width: 100%; border-collapse: collapse;"> <tr><td>Ned</td><td>12.70 kN</td></tr> <tr><td>Nc,Rd</td><td>131.60 kN</td></tr> <tr><td>Nu,Rd</td><td>141.15 kN</td></tr> </table> <table style="width: 100%; border-collapse: collapse;"> <tr><td>UC1</td><td>0.10</td></tr> <tr><td>UC2</td><td>0.09</td></tr> </table>	1 Ned / Nc,Rd < 1	eq. (6.22)	2 Ned / Nu,Rd < 1	eq. (6.21)	Ned	12.70 kN	Nc,Rd	131.60 kN	Nu,Rd	141.15 kN	UC1	0.10	UC2	0.09	<p><b>Bending Moment art. (6.2.5)</b></p> <table style="width: 100%; border-collapse: collapse;"> <tr><td>1 Myed / Myc,Rd &lt; 1</td><td>eq. (6.25)</td></tr> <tr><td>2 Myed / Myu,Rd &lt; 1</td><td>eq. (6.24)</td></tr> <tr><td>3 Mzed / Mzc,Rd &lt; 1</td><td>eq. (6.25)</td></tr> <tr><td>4 Mzed / Mzu,Rd &lt; 1</td><td>eq. (6.24)</td></tr> </table> <table style="width: 100%; border-collapse: collapse;"> <tr><td>Myed</td><td>0.00 kN</td></tr> <tr><td>Mzed</td><td>0.00 kN</td></tr> <tr><td>α;Y</td><td>1.34</td><td>table (6.4)</td></tr> <tr><td>α;Z</td><td>1.34</td><td>table (6.4)</td></tr> </table> <table style="width: 100%; border-collapse: collapse;"> <tr><td>Myc,Rd</td><td>3.02 kNm</td></tr> <tr><td>Myu,Rd</td><td>2.41 kNm</td></tr> <tr><td>Mzc,Rd</td><td>3.02 kNm</td></tr> <tr><td>Mzu,Rd</td><td>2.41 kNm</td></tr> </table> <table style="width: 100%; border-collapse: collapse;"> <tr><td>UC1-Y</td><td>-</td></tr> <tr><td>UC2-Y</td><td>-</td></tr> <tr><td>UC3-Z</td><td>-</td></tr> <tr><td>UC4-Z</td><td>-</td></tr> </table> <p style="text-align: right; color: red;">Check: not necessary, no bending moment</p>	1 Myed / Myc,Rd < 1	eq. (6.25)	2 Myed / Myu,Rd < 1	eq. (6.24)	3 Mzed / Mzc,Rd < 1	eq. (6.25)	4 Mzed / Mzu,Rd < 1	eq. (6.24)	Myed	0.00 kN	Mzed	0.00 kN	α;Y	1.34	table (6.4)	α;Z	1.34	table (6.4)	Myc,Rd	3.02 kNm	Myu,Rd	2.41 kNm	Mzc,Rd	3.02 kNm	Mzu,Rd	2.41 kNm	UC1-Y	-	UC2-Y	-	UC3-Z	-	UC4-Z	-	<p><b>Bending and Axial Force art. (6.2.9)</b></p> $\left(\frac{N_{Ed}}{N_{t,Rd}}\right)^2 + \left(\frac{M_{y,Ed}}{M_{y,Rd}}\right)^2 + \left(\frac{M_{z,Ed}}{M_{z,Rd}}\right)^2 \leq 1.00$ <p>eq. (6.43) - (ω0 = 1) - (ψ = 1.3)</p> <p style="text-align: center; border: 1px solid black; padding: 2px;">UC</p> <p style="text-align: right; color: red;">Check: not necessary, no bending moments</p>	<p><b>Buckling (compression) art. (6.3.1.1)</b></p> <table style="width: 100%; border-collapse: collapse;"> <tr><td>Ned / Nb,Rd &lt; 1</td><td>eq. (6.48)</td></tr> </table> <table style="width: 100%; border-collapse: collapse;"> <tr><td>Ned</td><td>12.70 kN</td></tr> </table> <table style="width: 100%; border-collapse: collapse;"> <tr><td>BC</td><td>A</td></tr> <tr><td>α</td><td>0.20</td><td>table (6.6)</td></tr> <tr><td>λ<sub>0</sub></td><td>0.10</td><td>table (6.6)</td></tr> <tr><td>χ</td><td>0.10</td><td>eq. (6.50)</td></tr> <tr><td>Φ</td><td>5.24 N</td></tr> <tr><td>λ</td><td>2.98</td><td>eq. (6.51)</td></tr> </table> <table style="width: 100%; border-collapse: collapse;"> <tr><td>Ncr</td><td>16252.25 (z-axis)</td></tr> </table> <table style="width: 100%; border-collapse: collapse;"> <tr><td>Nb,Rd</td><td>13.78 kN</td></tr> </table> <p style="text-align: center; border: 1px solid black; padding: 2px;">UC</p>	Ned / Nb,Rd < 1	eq. (6.48)	Ned	12.70 kN	BC	A	α	0.20	table (6.6)	λ <sub>0</sub>	0.10	table (6.6)	χ	0.10	eq. (6.50)	Φ	5.24 N	λ	2.98	eq. (6.51)	Ncr	16252.25 (z-axis)	Nb,Rd	13.78 kN	<p><b>Buckling (Bending and Axial Force) art. (6.3.3.1)</b></p> $\left(\frac{N_{Ed}}{\chi_{min}(N_{t,Rd})}\right)^2 + \frac{1}{\omega_0} \left[\left(\frac{M_{y,Ed}}{M_{y,Rd}}\right)^2 + \left(\frac{M_{z,Ed}}{M_{z,Rd}}\right)^2\right] \leq 1.00$ <p>eq. (6.62) - (ω0 = 1) - (ωx = 1) - (ψ = 0.8)</p> <p style="text-align: center; border: 1px solid black; padding: 2px;">UC</p> <p style="text-align: right; color: red;">Check: not necessary, no bending moment</p>	
1 Ned / Nc,Rd < 1	eq. (6.22)																																																																												
2 Ned / Nu,Rd < 1	eq. (6.21)																																																																												
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4 Mzed / Mzu,Rd < 1	eq. (6.24)																																																																												
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λ	2.98	eq. (6.51)																																																																											
Ncr	16252.25 (z-axis)																																																																												
Nb,Rd	13.78 kN																																																																												

### Annex C.13. Center pole 4.0m, closed – wood

Woodtype		Gum pole	
Strenght type		<b>D35</b>	
Material factor	$\gamma_M$	1.3	NEN-EN 1995-1-1:2005 table 2.3
Climate class		<b>2</b>	NEN-EN 1995-1-1:2005 article 2.3.1.3
Straightness factor	$\beta_c$	0.2	NEN-EN 1995-1-1:2005 equ. 6.29
Diameter	D	<b>85</b> mm	
Length (buckling)	$l_{buc,y}$	<b>4</b> m	
Effective area	A	5674.501731 mm <sup>2</sup>	
Moment of inertia	$I_y$	2562392.188 mm <sup>4</sup>	
Elastic modules	$W_{el,y}$	60291.58089 mm <sup>3</sup>	
Charistic pressure strenght	$f_{c0k}$	25 N/mm <sup>2</sup>	
Charistic bending strenght	$f_{c0k}$	35 N/mm <sup>2</sup>	
Modules of elasticity	$E_{0.05}$	8.7 kN/m <sup>2</sup>	
	$i_y$	21.3 mm	
Slenderness	$\lambda_y$	188.2	
Relative slenderness	$\lambda_{rely}$	3.21	NEN-EN 1995-1-1:2005 equ. 6.21
	$k_y$	5.95	NEN-EN 1995-1-1:2005 equ. 6.21
Buckling factor	$k_{cy}$	0.0913	NEN-EN 1995-1-1:2005 equ. 6.25

#### Strenght check

Pressure force	$F_d$	<b>8.30</b> kN	
Bending moment	$M_d$	<b>0.00</b> kNm	
Pressure stress	$\sigma_{c0d}$	1.46 N/mm <sup>2</sup>	
Bending stress	$\sigma_{c0d}$	0.00 N/mm <sup>2</sup>	
Load duration		<b>short</b>	NEN-EN 1995-1-1:2005 table 2.1
Modificationfactor	$k_{mod}$	0.90	NEN-EN 1995-1-1:2005 table 3.1
Design pressure strenght	$f_{c0d}$	17.31 N/mm <sup>2</sup>	NEN-EN 1995-1-1:2005 equ. 2.14
Design bending strenght	$f_{m0d}$	24.23 N/mm <sup>2</sup>	NEN-EN 1995-1-1:2005 equ. 2.14
Strenght check		<b>0.93</b>	NEN-EN 1995-1-1:2005 equ. 6.23

### Annex C.14. Center pole 4.0m, closed – Aluminium 76x4mm

Parameters		classification by thickness of round tube	
fo	160 N/mm <sup>2</sup>	t	3 mm
fu	195 N/mm <sup>2</sup>	D	76 mm
E	70000 N/mm <sup>2</sup>		
N	8.30 kN (druk)		
My	0.00 kNm		
Mz	0.00 kNm		
Lcr,y	4000 mm		
Lcr,z	4000 mm		
Iy	459074 mm <sup>4</sup>		
Iz	459074 mm <sup>4</sup>		
ey	38 mm		
ex	38 mm		
Wyel	12081 mm <sup>3</sup>		
Wypl	15996 mm <sup>3</sup>		
Wzel	12081 mm <sup>3</sup>		
Wzpl	15996 mm <sup>3</sup>		
Aeff	688 mm <sup>2</sup>		
vm1	1.1		
vm2	1.25		
			Off
			class override
classification conditions - Table 6.2 - Slenderness parameters			
Class A	B1 13.75	B2 20.00	B3 27.50
class 1	False	False	β < β1
class 2	True	True	β1 < β < β2
class 3	False	False	β2 < β < β3
class 4	False	False	β3 < β

Compression art. (6.2.4)	
1 Ned / Nc,Rd < 1	eq (6.22)
2 Ned / Nu,Rd < 1	eq (6.21)
Ned	8.30 kN
Nc,Rd	100.07 kN
Nu,Rd	107.33 kN
UC1	0.08
UC2	0.08

Bending Moment art. (6.2.5)	
1 Myed / Myc,Rd < 1	eq (6.25)
2 Myed / Myu,Rd < 1	eq (6.24)
3 Mzed / Mzc,Rd < 1	eq (6.25)
4 Mzed / Mzu,Rd < 1	eq (6.24)
Myed	0.00 kN
Mzed	0.00 kN
α:y	1.32 table (6.4)
α:z	1.32 table (6.4)
Myc,Rd	2.33 kNm
Myu,Rd	1.88 kNm
Mzc,Rd	2.33 kNm
Mzu,Rd	1.88 kNm
UC1-y	-
UC2-y	-
UC3-z	-
UC4-z	-

Bending and Axial Force art. (6.2.9)	
$\left( \frac{N_{Ed}}{N_{t,Rd}} \right)^{\psi} + \left( \frac{M_{y,Ed}}{M_{y,Rd}} \right)^{\psi} + \left( \frac{M_{z,Ed}}{M_{z,Rd}} \right)^{\psi} \leq 1.000$	
eq. (6.43) - (ω0 = 1) - (ψ = 1.3)	
UC	-

Buckling (compression) art. (6.3.1.1)	
Ned / Nb,Rd < 1	eq. (6.48)
Ned	8.30 kN
BC	A
α	0.20 table (6.6)
λ0	0.10 table (6.6)
χ	0.16 eq. (6.50)
Φ	3.50 N
λ	2.36 eq. (6.51)
Ncr	19822.59 (z-axis)
Nb,Rd	16.42 kN
UC	0.51

Buckling (Bending and Axial Force) art. (6.3.3.1)	
$\left( \frac{N_{Ed}}{N_{t,Rd}} \right)^{\psi_{ax}} + \frac{1}{\omega_0} \left[ \left( \frac{M_{y,Ed}}{M_{y,Rd}} \right)^{\psi} + \left( \frac{M_{z,Ed}}{M_{z,Rd}} \right)^{\psi} \right] \leq 1.000$	
eq. (6.62) - (ω0 = 1) - (ωax = 1) - (ψ = 0.8)	
UC	0.58

## Annex C.15. Entrance pole 3.0m, floating– wood

Woodtype		Gum pole	
Strenght type		<b>D35</b>	
Material factor	$\gamma_M$	1.3	NEN-EN 1995-1-1:2005 table 2.3
Climate class		<b>2</b>	NEN-EN 1995-1-1:2005 article 2.3.1.3
Straightness factor	$\beta_c$	0.2	NEN-EN 1995-1-1:2005 equ. 6.29
Diameter	D	<b>85</b> mm	
Length (buckling)	$l_{buc,y}$	<b>3</b> m	
Effective area	A	5674.501731 mm <sup>2</sup>	
Moment of inertia	$I_y$	2562392.188 mm <sup>4</sup>	
Elastic modules	$W_{el,y}$	60291.58089 mm <sup>3</sup>	
Charistic pressure strenght	$f_{c0k}$	25 N/mm <sup>2</sup>	
Charistic bending strenght	$f_{c0k}$	35 N/mm <sup>2</sup>	
Modules of elasticity	$E_{0.05}$	8.7 kN/m <sup>2</sup>	
	$i_y$	21.3 mm	
Slenderness	$\lambda_y$	141.2	
Relative slenderness	$\lambda_{rel,y}$	2.41	NEN-EN 1995-1-1:2005 equ. 6.21
	$k_y$	3.61	NEN-EN 1995-1-1:2005 equ. 6.21
Buckling factor	$k_{cy}$	0.15862	NEN-EN 1995-1-1:2005 equ. 6.25

### Strenght check

Pressure force	$F_d$	<b>13.34</b> kN	
Bending moment	$M_d$	<b>0.00</b> kNm	
Pressure stress	$\sigma_{c0d}$	2.35 N/mm <sup>2</sup>	
Bending stress	$\sigma_{c0d}$	0.00 N/mm <sup>2</sup>	
Load duration		<b>short</b>	NEN-EN 1995-1-1:2005 table 2.1
Modificationfactor	$k_{mod}$	0.90	NEN-EN 1995-1-1:2005 table 3.1
Design pressure strenght	$f_{c0d}$	17.31 N/mm <sup>2</sup>	NEN-EN 1995-1-1:2005 equ. 2.14
Design bending strenght	$f_{m0d}$	24.23 N/mm <sup>2</sup>	NEN-EN 1995-1-1:2005 equ. 2.14
Strenght check		<b>0.86</b>	NEN-EN 1995-1-1:2005 equ. 6.23

### Annex C.16. Entrance pole 3.0m, floating – Aluminium 76x3mm

Bending and Axial Force art. (6.2.9)

$$\left( \frac{N_{Ed}}{N_{Rd}} \right)^{\alpha} + \left( \frac{M_{y,Ed}}{M_{y,Rd}} \right)^{\beta} + \left( \frac{M_{z,Ed}}{M_{z,Rd}} \right)^{\beta} \leq 1.00$$

eq. (6.43) - ( $\omega_0 = 1$ ) - ( $\psi = 1.3$ )

UC -

*Check not necessary, no bending moments*

Buckling (compression) art. (6.3.1.1)

Ned / Nb,Rd < 1 ..... eq. (6.48)

Ned ..... 13.34 kN

BC ..... A

$\alpha$  ..... 0.20 table (6.6)

$\lambda_0$  ..... 0.10 table (6.6)

$\chi$  ..... 0.28 eq. (6.50)

$\phi$  ..... 2.23 N

$\lambda$  ..... 1.77 eq. (6.51)

Ncr ..... 35240.16 (z-axis)

Nb,Rd ..... 27.91 kN

UC ✓ 0.48

Buckling (Bending and Axial Force) art. (6.3.3.1)

$$\left( \frac{N_{Ed}}{X_{pl,Rd}} + \frac{1}{\omega_0} \left[ \frac{M_{y,Ed}}{M_{y,Rd}} \right]^{\beta} + \frac{1}{\omega_0} \left[ \frac{M_{z,Ed}}{M_{z,Rd}} \right]^{\beta} \right)^{1.00} \leq 1.00$$

eq. (6.62) - ( $\omega_0 = 1$ ) - ( $\psi = 0.8$ )

UC ✓ 0.55

Compression art. (6.2.4)

1 Ned / Nc,Rd < 1 ..... eq. (6.22)

2 Ned / Nu,Rd < 1 ..... eq. (6.21)

Ned ..... 13.34 kN

Nc,Rd ..... 100.07 kN

Nu,Rd ..... 107.33 kN

UC1 ✓ 0.13

UC2 ✓ 0.12

Bending Moment art. (6.2.5)

1 Myed / Myc,Rd < 1 ..... eq. (6.25)

2 Myed / Myu,Rd < 1 ..... eq. (6.24)

3 Mzed / Mzc,Rd < 1 ..... eq. (6.25)

4 Mzed / Mzu,Rd < 1 ..... eq. (6.24)

Myed ..... 0.00 kN

Mzed ..... 0.00 kN

$\alpha_y$  ..... 1.32 table (6.4)

$\alpha_z$  ..... 1.32 table (6.4)

Myc,Rd ..... 2.33 kNm

Myu,Rd ..... 1.88 kNm

Mzc,Rd ..... 2.33 kNm

Mzu,Rd ..... 1.88 kNm

UC1-y -

UC2-y -

UC3-z -

UC4-z -

*Check not necessary, no bending moments*

Parameters

classification by thickness of round tube

t ..... 3 mm

D ..... 76 mm

f<sub>o</sub> ..... 160 N/mm<sup>2</sup>

f<sub>u</sub> ..... 195 N/mm<sup>2</sup>

E ..... 70000 N/mm<sup>2</sup>

N ..... 13.34 kN (druk)

My ..... 0.00 kNm

Mz ..... 0.00 kNm

L<sub>cr,y</sub> ..... 3000 mm

L<sub>cr,z</sub> ..... 3000 mm

I<sub>y</sub> ..... 459074 mm<sup>4</sup>

I<sub>z</sub> ..... 459074 mm<sup>4</sup>

e<sub>y</sub> ..... 38 mm

e<sub>z</sub> ..... 38 mm

W<sub>yel</sub> ..... 12081 mm<sup>3</sup>

W<sub>yp1</sub> ..... 15996 mm<sup>3</sup>

W<sub>zpl</sub> ..... 12081 mm<sup>3</sup>

W<sub>zpl</sub> ..... 15996 mm<sup>3</sup>

A<sub>eff</sub> ..... 688 mm<sup>2</sup>

γ<sub>m1</sub> ..... 1.1

γ<sub>m2</sub> ..... 1.25

β ..... 15.10 ..... eq. (6.10)

ε ..... 1.25

class ..... 2 ..... table (6.2)

class override ..... off

classification conditions – Table 6.2 – Slenderness parameters

Class A	β1	β2	β3
	13.75	20.00	27.50
class 1		False	β < β1
class 2		True	β1 < β < β2
class 3		False	β2 < β < β3
class 4		False	β3 < β

## Annex C.17. Entrance pole 3.0m, closed– wood

Woodtype		Gum pole	
Strenght type		<b>D35</b>	
Material factor	$\gamma_M$	1.3	NEN-EN 1995-1-1:2005 table 2.3
Climate class		<b>2</b>	NEN-EN 1995-1-1:2005 article 2.3.1.3
Straightness factor	$\beta_c$	0.2	NEN-EN 1995-1-1:2005 equ. 6.29
Diameter	D	<b>85</b> mm	
Length (buckling)	$l_{buc,y}$	<b>3</b> m	
Effective area	A	5674.501731 mm <sup>2</sup>	
Moment of inertia	$I_y$	2562392.188 mm <sup>4</sup>	
Elastic modules	$W_{el,y}$	60291.58089 mm <sup>3</sup>	
Charistic pressure strenght	$f_{c0k}$	25 N/mm <sup>2</sup>	
Charistic bending strenght	$f_{c0k}$	35 N/mm <sup>2</sup>	
Modules of elasticity	$E_{0.05}$	8.7 kN/m <sup>2</sup>	
	$i_y$	21.3 mm	
Slenderness	$\lambda_y$	141.2	
Relative slenderness	$\lambda_{rel,y}$	2.41	NEN-EN 1995-1-1:2005 equ. 6.21
	$k_y$	3.61	NEN-EN 1995-1-1:2005 equ. 6.21
Buckling factor	$k_{cy}$	0.15862	NEN-EN 1995-1-1:2005 equ. 6.25

### Strenght check

Pressure force	$F_d$	<b>14.84</b> kN	
Bending moment	$M_d$	<b>0.00</b> kNm	
Pressure stress	$\sigma_{c0d}$	2.62 N/mm <sup>2</sup>	
Bending stress	$\sigma_{c0d}$	0.00 N/mm <sup>2</sup>	
Load duration		<b>short</b>	NEN-EN 1995-1-1:2005 table 2.1
Modificationfactor	$k_{mod}$	0.90	NEN-EN 1995-1-1:2005 table 3.1
Design pressure strenght	$f_{c0d}$	17.31 N/mm <sup>2</sup>	NEN-EN 1995-1-1:2005 equ. 2.14
Design bending strenght	$f_{m0d}$	24.23 N/mm <sup>2</sup>	NEN-EN 1995-1-1:2005 equ. 2.14
Strenght check		<b>0.95</b>	NEN-EN 1995-1-1:2005 equ. 6.23

### Annex C.18. Entrance pole 3.0m, closed – Aluminium 76x3mm

Parameters		classification by thickness of round tube	
fo	160 N/mm <sup>2</sup>	t	3 mm
fu	195 N/mm <sup>2</sup>	D	76 mm
E	70000 N/mm <sup>2</sup>		
N	14.84 kN (druk)		
My	0.00 kNm		
Mz	0.00 kNm		
Lcr,y	3000 mm		
Lcr,z	3000 mm		
Iy	459074 mm <sup>4</sup>		
Iz	459074 mm <sup>4</sup>		
ey	38 mm		
ex	38 mm		
Wyel	12081 mm <sup>3</sup>	β	15.10 <small>eq (6.10)</small>
Wypl	15996 mm <sup>3</sup>	ε	1.25
Wzel	12081 mm <sup>3</sup>	class	2 <small>table (6.2)</small>
Wzpl	15996 mm <sup>3</sup>	class override	Off
Aeff	688 mm <sup>2</sup>		
ym1	1.1		
ym2	1.25		

classification conditions - Table 6.2 - Slenderness parameters	
Class A	β1 < 13.75 and β2 < 20.00 and β3 < 27.50
class 1	β < β1
class 2	β1 < β < β2
class 3	β2 < β < β3
class 4	β3 < β

Compression art. (6.2.4)	
1 Ned / Nc,Rd < 1	eq (6.22)
2 Ned / Nu,Rd < 1	eq (6.21)
Ned	14.84 kN
Nc,Rd	100.07 kN
Nu,Rd	107.33 kN
UC1	0.15
UC2	0.14

Bending Moment art. (6.2.5)	
1 Myed / Myc,Rd < 1	eq (6.25)
2 Myed / Myu,Rd < 1	eq (6.24)
3 Mzed / Mzc,Rd < 1	eq (6.25)
4 Mzed / Mzu,Rd < 1	eq (6.24)
Myed	0.00 kN
Mzed	0.00 kN
α,y	1.32 <small>table (6.4)</small>
α,z	1.32 <small>table (6.4)</small>
Myc,Rd	2.33 kNm
Myu,Rd	1.88 kNm
Mzc,Rd	2.33 kNm
Mzu,Rd	1.88 kNm
UC1-y	-
UC2-y	-
UC3-z	-
UC4-z	-

Bending and Axial Force art. (6.2.9)	
eq (6.43) - (ω0 = 1) - (ψ = 1.3)	
UC	-

Buckling (compression) art. (6.3.1.1)	
Ned / Nb,Rd < 1	eq. (6.48)
Ned	14.84 kN
BC	A
α	0.20 <small>table (6.6)</small>
λ0	0.10 <small>table (6.6)</small>
χ	0.28 <small>eq. (6.50)</small>
Φ	2.23 N
λ	1.77 <small>eq. (6.51)</small>
Ncr	35240.16 <small>(z-axis)</small>
Nb,Rd	27.91 kN
UC	0.53

Buckling (Bending and Axial Force) art. (6.3.3.1)	
eq (6.62) - (ω0 = 1) - (ωx = 1) - (ψ = 0.8)	
UC	0.60

## Annex C.19. Corner pole 2.5m, floating – wood

Woodtype		Gum pole	
Strenght type		<b>D35</b>	
Material factor	$\gamma_M$	1.3	NEN-EN 1995-1-1:2005 table 2.3
Climate class		<b>2</b>	NEN-EN 1995-1-1:2005 article 2.3.1.3
Straightness factor	$\beta_c$	0.2	NEN-EN 1995-1-1:2005 equ. 6.29
Diameter	D	<b>65</b> mm	
Length (buckling)	$l_{buc,y}$	<b>2.5</b> m	
Effective area	A	3318.30724 mm <sup>2</sup>	
Moment of inertia	$I_y$	876240.5057 mm <sup>4</sup>	
Elastic modules	$W_{el,y}$	26961.24633 mm <sup>3</sup>	
Charistic pressure strenght	$f_{c0k}$	25 N/mm <sup>2</sup>	
Charistic bending strenght	$f_{c0k}$	35 N/mm <sup>2</sup>	
Modules of elasticity	$E_{0.05}$	8.7 kN/m <sup>2</sup>	
	$i_y$	16.3 mm	
Slenderness	$\lambda_y$	153.8	
Relative slenderness	$\lambda_{rely}$	2.63	NEN-EN 1995-1-1:2005 equ. 6.21
	$k_y$	4.18	NEN-EN 1995-1-1:2005 equ. 6.21
Buckling factor	$k_{cy}$	0.1346	NEN-EN 1995-1-1:2005 equ. 6.25

### Strenght check

Pressure force	$F_d$	<b>5.99</b> kN	
Bending moment	$M_d$	<b>0.00</b> kNm	
Pressure stress	$\sigma_{c0d}$	1.81 N/mm <sup>2</sup>	
Bending stress	$\sigma_{c0d}$	0.00 N/mm <sup>2</sup>	
Load duration		<b>short</b>	NEN-EN 1995-1-1:2005 table 2.1
Modificationfactor	$k_{mod}$	0.90	NEN-EN 1995-1-1:2005 table 3.1
Design pressure strenght	$f_{c0d}$	17.31 N/mm <sup>2</sup>	NEN-EN 1995-1-1:2005 equ. 2.14
Design bending strenght	$f_{m0d}$	24.23 N/mm <sup>2</sup>	NEN-EN 1995-1-1:2005 equ. 2.14
Strenght check		<b>0.77</b>	NEN-EN 1995-1-1:2005 equ. 6.23



## Annex C.20. Corner pole 2.5m, floating – Aluminium 50x3mm

Parameters		classification by thickness of round tube	
f <sub>o</sub>	160 N/mm <sup>2</sup>	t	3 mm
f <sub>u</sub>	195 N/mm <sup>2</sup>	D	50 mm
E	70000 N/mm <sup>2</sup>		
N	5.99 kN (druk)		
M <sub>y</sub>	0.00 kNm		
M <sub>z</sub>	0.00 kNm		
L <sub>cr,y</sub>	2500 mm		
L <sub>cr,z</sub>	2500 mm		
I <sub>y</sub>	122812 mm <sup>4</sup>		
I <sub>z</sub>	122812 mm <sup>4</sup>		
e <sub>y</sub>	25 mm		
e <sub>x</sub>	25 mm		
W <sub>yel</sub>	4912 mm <sup>3</sup>		
W <sub>ypl</sub>	6636 mm <sup>3</sup>		
W <sub>zel</sub>	4912 mm <sup>3</sup>		
W <sub>zpl</sub>	6636 mm <sup>3</sup>		
A <sub>eff</sub>	443 mm <sup>2</sup>		
γ <sub>m1</sub>	1.1		
γ <sub>m2</sub>	1.25		
		class	1
		class override	Off

classification conditions - Table 6.2 - Slenderness parameters	
Class A	β <sub>1</sub> 13.75    β <sub>2</sub> 20.00    β <sub>3</sub> 27.50
Class 1	β < β <sub>1</sub>
Class 2	β <sub>1</sub> < β < β <sub>2</sub>
Class 3	β <sub>2</sub> < β < β <sub>3</sub>
Class 4	β <sub>3</sub> < β

Compression art. (6.2.4)	
1 N <sub>ed</sub> / N <sub>c,Rd</sub> < 1	eq (6.22)
2 N <sub>ed</sub> / N <sub>u,Rd</sub> < 1	eq (6.21)
N <sub>ed</sub>	5.99 kN
N <sub>c,Rd</sub>	64.43 kN
N <sub>u,Rd</sub>	69.10 kN
UC1	0.09
UC2	0.09

Bending Moment art. (6.2.5)	
1 M <sub>y,ed</sub> / M <sub>y,c,Rd</sub> < 1	eq (6.25)
2 M <sub>y,ed</sub> / M <sub>y,u,Rd</sub> < 1	eq (6.24)
3 M <sub>z,ed</sub> / M <sub>z,c,Rd</sub> < 1	eq (6.25)
4 M <sub>z,ed</sub> / M <sub>z,u,Rd</sub> < 1	eq (6.24)
M <sub>y,ed</sub>	0.00 kNm
M <sub>z,ed</sub>	0.00 kNm
α <sub>y</sub>	1.35 table (6.4)
α <sub>z</sub>	1.35 table (6.4)
M <sub>y,c,Rd</sub>	0.97 kNm
M <sub>y,u,Rd</sub>	0.77 kNm
M <sub>z,c,Rd</sub>	0.97 kNm
M <sub>z,u,Rd</sub>	0.77 kNm
UC1-y	-
UC2-y	-
UC3-z	-
UC4-z	-

Bending and Axial Force art. (6.2.9)	
$\left( \frac{N_{ed}}{N_{t,Rd}} \right)^2 + \left[ \frac{M_{y,ed}}{(M_{y,Rd})_{lim}} \right]^2 + \left[ \frac{M_{z,ed}}{(M_{z,Rd})_{lim}} \right]^2 \leq 1.000$	
eq. (6.43) - (ω <sub>0</sub> = 1) - (ψ = 1.3)	
UC	-

Buckling (compression) art. (6.3.1.1)	
N <sub>ed</sub> / N <sub>b,Rd</sub> < 1	eq. (6.48)
N <sub>ed</sub>	5.99 kN
BC	A
α	0.20 table (6.6)
λ <sub>0</sub>	0.10 table (6.6)
χ	0.17 eq. (6.50)
Φ	3.33 N
λ	2.28 eq. (6.51)
N <sub>cr</sub>	13575.58 (z-axis)
N <sub>b,Rd</sub>	11.21 kN
UC	0.53

Buckling (Bending and Axial Force) art. (6.3.3.1)	
$\left( \frac{N_{ed}}{N_{t,Rd}} \right)^2 + \left[ \frac{M_{y,ed}}{(M_{y,Rd})_{lim}} \right]^2 + \left[ \frac{M_{z,ed}}{(M_{z,Rd})_{lim}} \right]^2 \leq 1.000$	
eq. (6.62) - (ω <sub>0</sub> = 1) - (ψ = 0.8)	
UC	0.61

## Annex C.21. Corner pole 2.5m, closed – wood

Woodtype		Gum pole	
Strenght type		<b>D35</b>	
Material factor	$\gamma_M$	1.3	NEN-EN 1995-1-1:2005 table 2.3
Climate class		<b>2</b>	NEN-EN 1995-1-1:2005 article 2.3.1.3
Straightness factor	$\beta_c$	0.2	NEN-EN 1995-1-1:2005 equ. 6.29
Diameter	D	<b>70</b> mm	
Length (buckling)	$l_{buc,y}$	<b>2.5</b> m	
Effective area	A	3848.451001 mm <sup>2</sup>	
Moment of inertia	$I_y$	1178588.119 mm <sup>4</sup>	
Elastic modules	$W_{el,y}$	33673.94626 mm <sup>3</sup>	
Charistic pressure strenght	$f_{c0k}$	25 N/mm <sup>2</sup>	
Charistic bending strenght	$f_{c0k}$	35 N/mm <sup>2</sup>	
Modules of elasticity	$E_{0.05}$	8.7 kN/m <sup>2</sup>	
	$i_y$	17.5 mm	
Slenderness	$\lambda_y$	142.9	
Relative slenderness	$\lambda_{rel,y}$	2.44	NEN-EN 1995-1-1:2005 equ. 6.21
	$k_y$	3.68	NEN-EN 1995-1-1:2005 equ. 6.21
Buckling factor	$k_{cy}$	0.15509	NEN-EN 1995-1-1:2005 equ. 6.25

### Strenght check

Pressure force	$F_d$	<b>8.78</b> kN	
Bending moment	$M_d$	<b>0.00</b> kNm	
Pressure stress	$\sigma_{c0d}$	2.28 N/mm <sup>2</sup>	
Bending stress	$\sigma_{c0d}$	0.00 N/mm <sup>2</sup>	
Load duration		<b>short</b>	NEN-EN 1995-1-1:2005 table 2.1
Modificationfactor	$k_{mod}$	0.90	NEN-EN 1995-1-1:2005 table 3.1
Design pressure strenght	$f_{c0d}$	17.31 N/mm <sup>2</sup>	NEN-EN 1995-1-1:2005 equ. 2.14
Design bending strenght	$f_{m0d}$	24.23 N/mm <sup>2</sup>	NEN-EN 1995-1-1:2005 equ. 2.14
Strenght check		<b>0.85</b>	NEN-EN 1995-1-1:2005 equ. 6.23

## Annex C.22. Corner pole 2.5m, closed – Aluminium 50x3mm

Bending and Axial Force art. (6.2.9)

$$\left( \frac{N_{Ed}}{N_{t,Rd}} \right)^{\psi} + \left( \frac{M_{y,Ed}}{M_{y,Rd}} \right)^{\psi} + \left( \frac{M_{z,Ed}}{M_{z,Rd}} \right)^{\psi} \leq 1.000$$

eq. (6.43) - ( $\psi = 1$ ) - ( $\psi = 1.3$ )

UC

*Checks not necessary, no bending moments*

Buckling (compression) art. (6.3.1.1)

$N_{Ed} / N_{b,Rd} < 1$  eq. (6.48)

$N_{Ed}$  8.78 kN

BC A

$\alpha$  0.20 table (6.6)

$\lambda_0$  0.10 table (6.6)

$\chi$  0.17 eq. (6.50)

$\phi$  3.33 N

$\lambda$  2.28 eq. (6.51)

$N_{cr}$  13575.58 (z-axis)

$N_{b,Rd}$  11.21 kN

UC  0.78

Buckling (Bending and Axial Force) art. (6.3.3.1)

$$\left( \frac{N_{Ed}}{N_{t,Rd} + N_{b,Rd}} \right)^{\psi} + \frac{1}{\omega} \left[ \left( \frac{M_{y,Ed}}{M_{y,Rd}} \right)^{\psi} + \left( \frac{M_{z,Ed}}{M_{z,Rd}} \right)^{\psi} \right] \leq 1.000$$

eq. (6.62) - ( $\omega = 1$ ) - ( $\omega = 1$ ) - ( $\psi = 0.8$ )

UC  0.82

Compression art. (6.2.4)

1  $N_{Ed} / N_{c,Rd} < 1$  eq. (6.22)

2  $N_{Ed} / N_{u,Rd} < 1$  eq. (6.21)

$N_{c,Rd}$  8.78 kN

$N_{u,Rd}$  64.43 kN

$N_{u,Rd}$  69.10 kN

UC1  0.14

UC2  0.13

Bending Moment art. (6.2.5)

1  $M_{y,Ed} / M_{y,Rd} < 1$  eq. (6.25)

2  $M_{y,Ed} / M_{y,Rd} < 1$  eq. (6.24)

3  $M_{z,Ed} / M_{z,Rd} < 1$  eq. (6.25)

4  $M_{z,Ed} / M_{z,Rd} < 1$  eq. (6.24)

$M_{y,Ed}$  0.00 kN

$M_{z,Ed}$  0.00 kN

$\alpha_y$  1.35 table (6.4)

$\alpha_z$  1.35 table (6.4)

$M_{y,Rd}$  0.97 kNm

$M_{z,Rd}$  0.77 kNm

$M_{y,Rd}$  0.97 kNm

$M_{z,Rd}$  0.77 kNm

UC1-y -

UC2-y -

UC3-z -

UC4-z -

*Checks not necessary, no bending moment*

Parameters

fo	160 N/mm <sup>2</sup>
fu	195 N/mm <sup>2</sup>
E	70000 N/mm <sup>2</sup>
N	8.78 kN (druk)
My	0.00 kNm
Mz	0.00 kNm
Lcry	2500 mm
Lcrz	2500 mm
Iy	122812 mm <sup>4</sup>
Iz	122812 mm <sup>4</sup>
ey	25 mm
ex	25 mm
Wyel	4912 mm <sup>3</sup>
Wypl	6636 mm <sup>3</sup>
Wzel	4912 mm <sup>3</sup>
Wzpl	6636 mm <sup>3</sup>
Aeff	443 mm <sup>2</sup>
ym1	1.1
ym2	1.25

classification by thickness of round tube

t 3 mm

D 50 mm

$\beta$  12.25 eq. (6.10)

$\epsilon$  1.25

class 1 table (6.2)

class override Off

classification conditions - Table 6.2 - Slenderness parameters

Class A	$\beta_1$ 13.75	$\beta_2$ 20.00	$\beta_3$ 27.50
class 1	True $\beta < \beta_1$		
class 2	False $\beta_1 < \beta < \beta_2$		
class 3	False $\beta_2 < \beta < \beta_3$		
class 4	False $\beta_3 < \beta$		

## Annex C.23. Side wall pole 2.5m, closed – wood

Woodtype		Gum pole	
Strenght type		<b>D35</b>	
Material factor	$\gamma_M$	1.3	NEN-EN 1995-1-1:2005 table 2.3
Climate class		<b>2</b>	NEN-EN 1995-1-1:2005 article 2.3.1.3
Straightness factor	$\beta_c$	0.2	NEN-EN 1995-1-1:2005 equ. 6.29
Diameter	D	<b>70</b> mm	
Length (buckling)	$l_{buc,y}$	<b>2.5</b> m	
Effective area	A	3848.451001 mm <sup>2</sup>	
Moment of inertia	$I_y$	1178588.119 mm <sup>4</sup>	
Elastic modules	$W_{el,y}$	33673.94626 mm <sup>3</sup>	
Charistic pressure strenght	$f_{c0k}$	25 N/mm <sup>2</sup>	
Charistic bending strenght	$f_{c0k}$	35 N/mm <sup>2</sup>	
Modules of elasticity	$E_{0.05}$	8.7 kN/m <sup>2</sup>	
	$i_y$	17.5 mm	
Slenderness	$\lambda_y$	142.9	
Relative slenderness	$\lambda_{rely}$	2.44	NEN-EN 1995-1-1:2005 equ. 6.21
	$k_y$	3.68	NEN-EN 1995-1-1:2005 equ. 6.21
Buckling factor	$k_{cy}$	0.15509	NEN-EN 1995-1-1:2005 equ. 6.25

### Strenght check

Pressure force	$F_d$	<b>8.19</b> kN	
Bending moment	$M_d$	<b>0.00</b> kNm	
Pressure stress	$\sigma_{c0d}$	2.13 N/mm <sup>2</sup>	
Bending stress	$\sigma_{c0d}$	0.00 N/mm <sup>2</sup>	
Load duration		<b>short</b>	NEN-EN 1995-1-1:2005 table 2.1
Modificationfactor	$k_{mod}$	0.90	NEN-EN 1995-1-1:2005 table 3.1
Design pressure strenght	$f_{c0d}$	17.31 N/mm <sup>2</sup>	NEN-EN 1995-1-1:2005 equ. 2.14
Design bending strenght	$f_{m0d}$	24.23 N/mm <sup>2</sup>	NEN-EN 1995-1-1:2005 equ. 2.14
Strenght check		<b>0.79</b>	NEN-EN 1995-1-1:2005 equ. 6.23

### Annex C.24. Side wall pole 2.5m, closed – Aluminium 50x3mm

Bending and Axial Force art. (6.2.9)

$$\left(\frac{N_{Ed}}{N_{t,Rd}}\right)^{\psi} + \left(\frac{M_{y,Ed}}{M_{y,Rd}}\right)^{\psi} + \left(\frac{M_{z,Ed}}{M_{z,Rd}}\right)^{\psi} \leq 1.000$$

eq. (6.43) – ( $\psi = 1$ ) – ( $\psi = 1.3$ )

UC  **0.73**

*Check not necessary, no bending moments*

Buckling (compression) art. (6.3.1.1)

$N_{Ed} / N_{b,Rd} < 1$  **eq. (6.48)**

$N_{Ed}$  ..... 8.19 kN

BC ..... A

$\alpha$  ..... 0.20 table (6.6)

$\lambda_0$  ..... 0.10 table (6.6)

$\chi$  ..... 0.17 eq. (6.50)

$\Phi$  ..... 3.33 N

$\lambda$  ..... 2.28 eq. (6.51)

Ncr ..... 13575.58 (z-axis)

Nb,Rd ..... 11.21 kN

UC  **0.73**

Buckling (Bending and Axial Force) art. (6.3.3.1)

$$\left(\frac{N_{Ed}}{\chi_{min} N_{t,Rd}}\right)^{\psi} + 1 \left(\frac{M_{y,Ed}}{M_{y,Rd}}\right)^{\psi} + \left(\frac{M_{z,Ed}}{M_{z,Rd}}\right)^{\psi} \leq 1.00$$

eq. (6.62) – ( $\psi = 1$ ) – ( $\psi = 1$ ) – ( $\psi = 0.8$ )

UC  **0.78**

*Check not necessary, no bending moments*

Compression art. (6.2.4)

1  $N_{Ed} / N_{c,Rd} < 1$  **eq. (6.22)**

2  $N_{Ed} / N_{u,Rd} < 1$  **eq. (6.21)**

$N_{Ed}$  ..... 8.19 kN

$N_{c,Rd}$  ..... 64.43 kN

$N_{u,Rd}$  ..... 69.10 kN

UC1  **0.13**

UC2  **0.12**

Bending Moment art. (6.2.5)

1  $M_{y,Ed} / M_{y,Rd} < 1$  **eq. (6.25)**

2  $M_{y,Ed} / M_{y,u,Rd} < 1$  **eq. (6.24)**

3  $M_{z,Ed} / M_{z,Rd} < 1$  **eq. (6.25)**

4  $M_{z,Ed} / M_{z,u,Rd} < 1$  **eq. (6.24)**

$M_{y,Ed}$  ..... 0.00 kN

$M_{z,Ed}$  ..... 0.00 kN

$\alpha_y$  ..... 1.35 table (6.4)

$\alpha_z$  ..... 1.35 table (6.4)

$M_{y,Rd}$  ..... 0.97 kNm

$M_{y,u,Rd}$  ..... 0.77 kNm

$M_{z,Rd}$  ..... 0.97 kNm

$M_{z,u,Rd}$  ..... 0.77 kNm

UC1-y ..... -

UC2-y ..... -

UC3-z ..... -

UC4-z ..... -

*Check not necessary, no bending moment*

Parameters

fo ..... 160 N/mm<sup>2</sup>

fu ..... 195 N/mm<sup>2</sup>

E ..... 70000 N/mm<sup>2</sup>

N ..... 8.19 kN (druk)

My ..... 0.00 kNm

Mz ..... 0.00 kNm

Lcry ..... 2500 mm

Lcrz ..... 2500 mm

Iy ..... 122812 mm<sup>4</sup>

Iz ..... 122812 mm<sup>4</sup>

ey ..... 25 mm

ex ..... 25 mm

Wyel ..... 4912 mm<sup>3</sup>

Wypl ..... 6636 mm<sup>3</sup>

Wzel ..... 4912 mm<sup>3</sup>

Wzpl ..... 6636 mm<sup>3</sup>

Aeff ..... 443 mm<sup>2</sup>

vm1 ..... 1.1

vm2 ..... 1.25

classification by thickness of round tube

t ..... 3 mm

D ..... 50 mm

$\beta$  ..... 12.25 eq. (6.10)

$\epsilon$  ..... 1.25

class ..... 1 table (6.2)

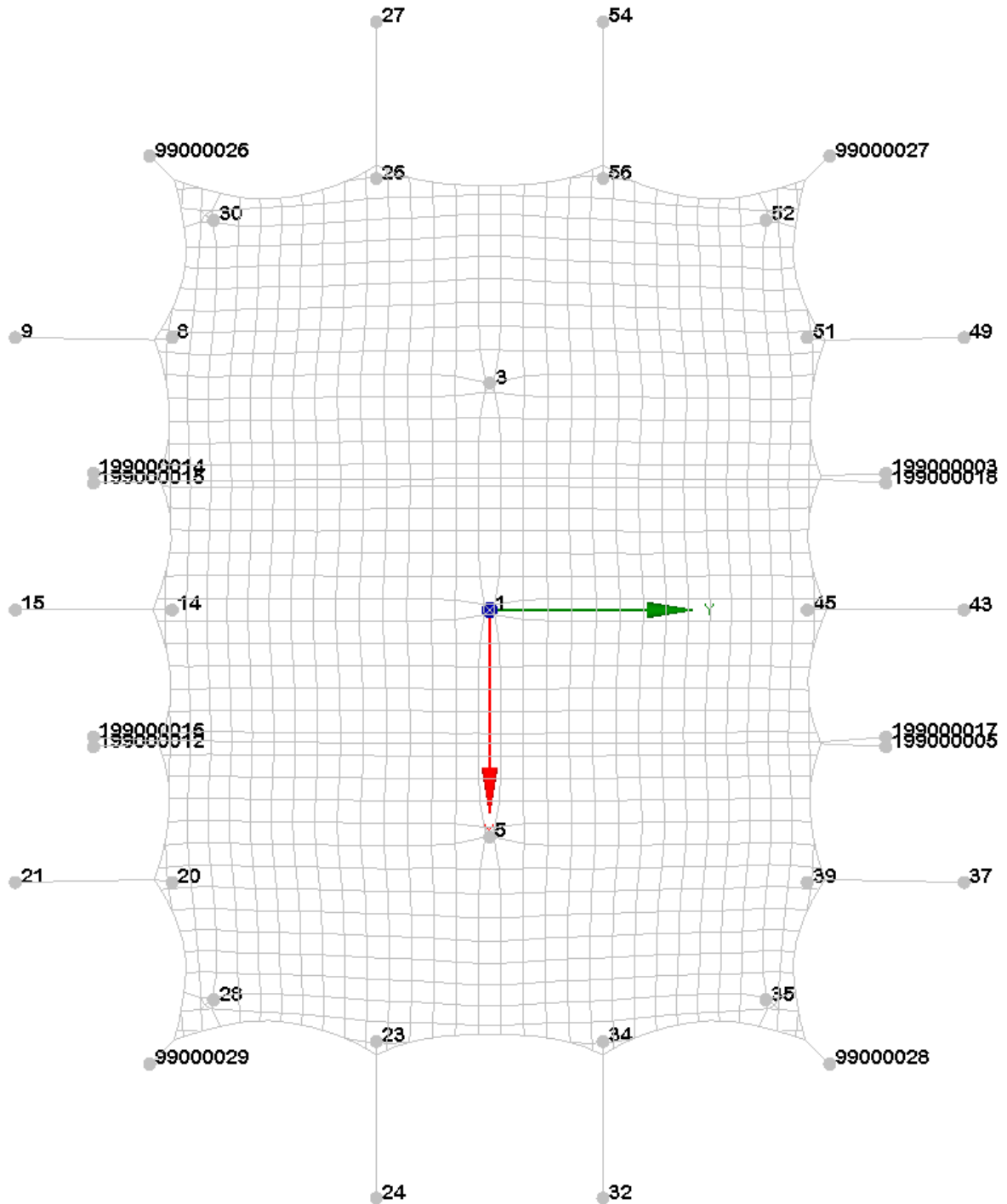
class override ..... Off

classification conditions - Table 6.2 - Slenderness parameters

	$\beta_1$	$\beta_2$	$\beta_3$
Class A	13.75	20.00	27.50
class 1		True	$\beta < \beta_1$
class 2		False	$\beta_1 < \beta < \beta_2$
class 3		False	$\beta_2 < \beta < \beta_3$
class 4		False	$\beta_3 < \beta$

**Annex D. Software output (reaction forces)**

**Annex D.1. Point numbers 20x15 - floating**



## Annex D.2. Reaction forces 20x15m - floating

### Annex D.2.1. CO1: Own weight + pretension

Att	Node	Koor X	Koor Y	Koor Z	Fx	Fy	Fz
Guy rope	9	-6.00	-10.46	0.00	0.00	-0.77	-0.77
Guy rope	15	0.00	-10.46	0.00	0.00	-0.65	-0.64
Guy rope	24	12.96	-2.50	0.00	0.97	-0.01	-0.96
Guy rope	27	-12.96	-2.50	0.00	-0.97	-0.01	-0.96
Guy rope	32	12.96	2.50	0.00	0.97	0.01	-0.96
Guy rope	37	6.00	10.46	0.00	0.00	0.77	-0.77
Guy rope	43	0.00	10.46	0.00	0.00	0.65	-0.64
Guy rope	49	-6.00	10.46	0.00	0.00	0.77	-0.77
Guy rope	54	-12.96	2.50	0.00	-0.97	0.01	-0.96
Guy rope	21	6.00	-10.46	0.00	0.00	-0.77	-0.77
Guy rope (corner)	99000026	-10.00	-7.50	0.00	-1.43	-1.60	-2.28
Guy rope (corner)	99000027	-10.00	7.50	0.00	-1.43	1.60	-2.28
Guy rope (corner)	99000028	10.00	7.50	0.00	1.43	1.60	-2.28
Guy rope (corner)	99000029	10.00	-7.50	0.00	1.43	-1.60	-2.28
Guy rope (valley)	199000003	-3.01	8.73	0.00	0.00	0.77	-1.11
Guy rope (valley)	199000005	3.01	8.73	0.00	0.00	0.77	-1.11
Guy rope (valley)	199000012	3.01	-8.73	0.00	0.00	-0.77	-1.11
Guy rope (valley)	199000014	-3.01	-8.73	0.00	0.00	-0.77	-1.11
Center pole 5.5m	1	0.00	0.00	0.00	0.00	0.00	1.52
Center pole 5.0m	3	-5.00	0.00	0.00	0.01	0.00	1.34
Center pole 5.0m	5	5.00	0.00	0.00	-0.01	0.00	1.34
Corner pole 2.5m	28	8.59	-6.09	0.00	0.13	-0.12	0.94
Corner pole 2.5m	30	-8.59	-6.09	0.00	-0.13	-0.12	0.94
Corner pole 2.5m	35	8.59	6.09	0.00	0.13	0.12	0.94
Corner pole 2.5m	52	-8.59	6.09	0.00	-0.13	0.12	0.94
Entrance pole 3.0m	8	-6.00	-7.00	0.00	0.00	-0.29	1.92
Entrance pole 3.0m	14	0.00	-7.00	0.00	0.00	-0.24	1.53
Entrance pole 3.0m	23	9.50	-2.50	0.00	0.23	0.01	1.59
Entrance pole 3.0m	26	-9.50	-2.50	0.00	-0.23	0.01	1.59
Entrance pole 3.0m	34	9.50	2.50	0.00	0.23	-0.01	1.59
Entrance pole 3.0m	39	6.00	7.00	0.00	0.00	0.29	1.92
Entrance pole 3.0m	45	0.00	7.00	0.00	0.00	0.24	1.53
Entrance pole 3.0m	51	-6.00	7.00	0.00	0.00	0.29	1.92
Entrance pole 3.0m	56	-9.50	2.50	0.00	-0.23	-0.01	1.59
Entrance pole 3.0m	20	6.00	-7.00	0.00	0.00	-0.29	1.92

### Annex D.2.2. CO2: Own weight + pretension + conventional / snow

Att	Node	Koor X	Koor Y	Koor Z	Fx	Fy	Fz
Guy rope	9	-6.00	-10.46	0.00	-0.07	-2.88	-2.81
Guy rope	15	0.00	-10.46	0.00	0.00	-3.06	-2.98
Guy rope	24	12.96	-2.50	0.00	2.93	-0.04	-2.84
Guy rope	27	-12.96	-2.50	0.00	-2.93	-0.04	-2.84
Guy rope	32	12.96	2.50	0.00	2.93	0.04	-2.84
Guy rope	37	6.00	10.46	0.00	0.07	2.88	-2.81
Guy rope	43	0.00	10.46	0.00	0.00	3.06	-2.98
Guy rope	49	-6.00	10.46	0.00	-0.07	2.88	-2.81
Guy rope	54	-12.96	2.50	0.00	-2.93	0.04	-2.84
Guy rope	21	6.00	-10.46	0.00	0.07	-2.88	-2.81
Guy rope (corner)	99000026	-10.00	-7.50	0.00	-3.38	-3.43	-5.11
Guy rope (corner)	99000027	-10.00	7.50	0.00	-3.38	3.43	-5.11
Guy rope (corner)	99000028	10.00	7.50	0.00	3.38	3.43	-5.11
Guy rope (corner)	99000029	10.00	-7.50	0.00	3.38	-3.43	-5.11
Guy rope (valley)	199000003	-3.01	8.73	0.00	-0.03	1.51	-2.02
Guy rope (valley)	199000005	3.01	8.73	0.00	0.03	1.51	-2.02
Guy rope (valley)	199000012	3.01	-8.73	0.00	0.03	-1.51	-2.02
Guy rope (valley)	199000014	-3.01	-8.73	0.00	-0.03	-1.51	-2.02
Center pole 5.5m	1	0.00	0.00	0.00	0.00	0.00	8.16
Center pole 5.0m	3	-5.00	0.00	0.00	0.12	0.00	6.97
Center pole 5.0m	5	5.00	0.00	0.00	-0.12	0.00	6.97
Corner pole 2.5m	28	8.59	-6.09	0.00	0.29	-0.33	2.61
Corner pole 2.5m	30	-8.59	-6.09	0.00	-0.29	-0.33	2.61
Corner pole 2.5m	35	8.59	6.09	0.00	0.29	0.33	2.61
Corner pole 2.5m	52	-8.59	6.09	0.00	-0.29	0.33	2.61
Entrance pole 3.0m	8	-6.00	-7.00	0.00	0.16	-0.84	6.15
Entrance pole 3.0m	14	0.00	-7.00	0.00	0.00	-0.79	5.75
Entrance pole 3.0m	23	9.50	-2.50	0.00	0.64	0.07	4.78
Entrance pole 3.0m	26	-9.50	-2.50	0.00	-0.64	0.07	4.78
Entrance pole 3.0m	34	9.50	2.50	0.00	0.64	-0.07	4.78
Entrance pole 3.0m	39	6.00	7.00	0.00	-0.16	0.84	6.15
Entrance pole 3.0m	45	0.00	7.00	0.00	0.00	0.79	5.75
Entrance pole 3.0m	51	-6.00	7.00	0.00	0.16	0.84	6.15
Entrance pole 3.0m	56	-9.50	2.50	0.00	-0.64	-0.07	4.78
Entrance pole 3.0m	20	6.00	-7.00	0.00	-0.16	-0.84	6.15



### Annex D.2.3. CO3: Own weight + pretension + wind pressure

Att	Node	Koor X	Koor Y	Koor Z	Fx	Fy	Fz
Guy rope	9	-6.00	-10.46	0.00	-0.16	-4.13	-3.99
Guy rope	15	0.00	-10.46	0.00	0.00	-4.63	-4.47
Guy rope	24	12.96	-2.50	0.00	4.26	-0.07	-4.10
Guy rope	27	-12.96	-2.50	0.00	-4.26	-0.07	-4.10
Guy rope	32	12.96	2.50	0.00	4.26	0.07	-4.10
Guy rope	37	6.00	10.46	0.00	0.16	4.13	-3.99
Guy rope	43	0.00	10.46	0.00	0.00	4.63	-4.47
Guy rope	49	-6.00	10.46	0.00	-0.16	4.13	-3.99
Guy rope	54	-12.96	2.50	0.00	-4.26	0.07	-4.10
Guy rope	21	6.00	-10.46	0.00	0.16	-4.13	-3.99
Guy rope (corner)	99000026	-10.00	-7.50	0.00	-4.86	-4.72	-7.11
Guy rope (corner)	99000027	-10.00	7.50	0.00	-4.86	4.71	-7.11
Guy rope (corner)	99000028	10.00	7.50	0.00	4.86	4.71	-7.11
Guy rope (corner)	99000029	10.00	-7.50	0.00	4.86	-4.71	-7.11
Guy rope (valley)	199000003	-3.01	8.73	0.00	-0.07	2.21	-2.75
Guy rope (valley)	199000005	3.01	8.73	0.00	0.08	2.21	-2.75
Guy rope (valley)	199000012	3.01	-8.73	0.00	0.07	-2.21	-2.75
Guy rope (valley)	199000014	-3.01	-8.73	0.00	-0.08	-2.21	-2.75
Center pole 5.5m	1	0.00	0.00	0.00	0.00	0.00	11.05
Center pole 5.0m	3	-5.00	0.00	0.00	0.26	0.00	9.55
Center pole 5.0m	5	5.00	0.00	0.00	-0.26	0.00	9.55
Corner pole 2.5m	28	8.59	-6.09	0.00	0.38	-0.49	3.87
Corner pole 2.5m	30	-8.59	-6.09	0.00	-0.38	-0.49	3.87
Corner pole 2.5m	35	8.59	6.09	0.00	0.38	0.49	3.87
Corner pole 2.5m	52	-8.59	6.09	0.00	-0.38	0.49	3.87
Entrance pole 3.0m	8	-6.00	-7.00	0.00	0.36	-1.11	8.76
Entrance pole 3.0m	14	0.00	-7.00	0.00	0.00	-1.07	8.58
Entrance pole 3.0m	23	9.50	-2.50	0.00	0.84	0.11	6.81
Entrance pole 3.0m	26	-9.50	-2.50	0.00	-0.84	0.11	6.81
Entrance pole 3.0m	34	9.50	2.50	0.00	0.84	-0.11	6.81
Entrance pole 3.0m	39	6.00	7.00	0.00	-0.36	1.11	8.76
Entrance pole 3.0m	45	0.00	7.00	0.00	0.00	1.07	8.58
Entrance pole 3.0m	51	-6.00	7.00	0.00	0.36	1.11	8.76
Entrance pole 3.0m	56	-9.50	2.50	0.00	-0.84	-0.11	6.81
Entrance pole 3.0m	20	6.00	-7.00	0.00	-0.36	-1.11	8.76

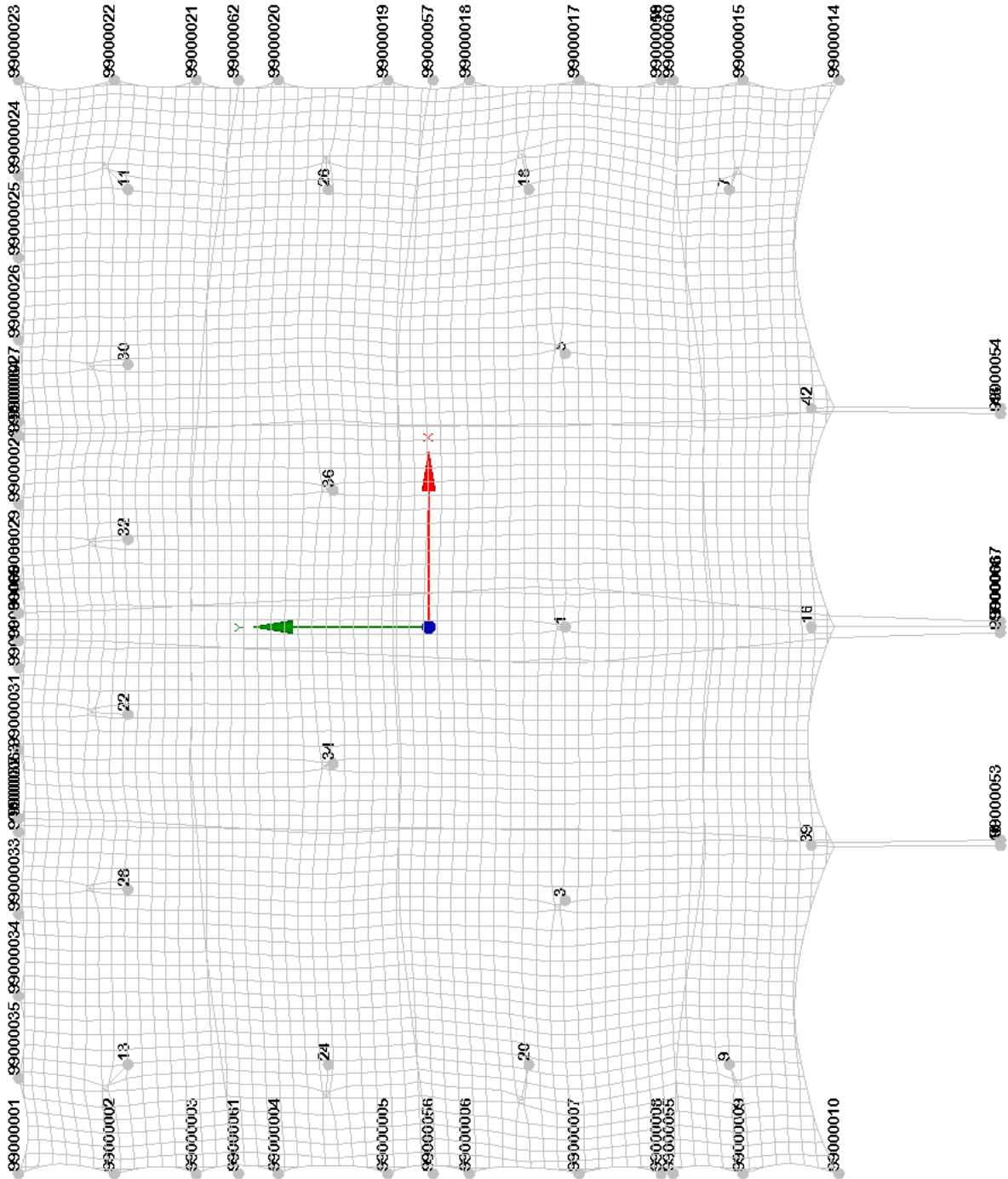
**Annex D.2.4. CO4: Own weight + pretension + wind suction – floating – reduction 0.53**

Att	Node	Koor X	Koor Y	Koor Z	Fx	Fy	Fz
Guy rope	9	-6.00	-10.46	0.00	-0.03	-2.49	-2.43
Guy rope	15	0.00	-10.46	0.00	0.00	-1.99	-1.96
Guy rope	24	12.96	-2.50	0.00	4.65	0.00	-4.46
Guy rope	27	-12.96	-2.50	0.00	-4.65	0.00	-4.46
Guy rope	32	12.96	2.50	0.00	4.65	0.00	-4.46
Guy rope	37	6.00	10.46	0.00	0.03	2.49	-2.43
Guy rope	43	0.00	10.46	0.00	0.00	1.99	-1.96
Guy rope	49	-6.00	10.46	0.00	-0.03	2.49	-2.43
Guy rope	54	-12.96	2.50	0.00	-4.65	0.00	-4.46
Guy rope	21	6.00	-10.46	0.00	0.03	-2.49	-2.43
Guy rope (corner)	99000026	-10.00	-7.50	0.00	-5.53	-5.81	-7.91
Guy rope (corner)	99000027	-10.00	7.50	0.00	-5.54	5.81	-7.91
Guy rope (corner)	99000028	10.00	7.50	0.00	5.53	5.81	-7.91
Guy rope (corner)	99000029	10.00	-7.50	0.00	5.53	-5.81	-7.91
Guy rope (valley)	199000003	-3.01	8.73	0.00	-0.22	6.39	-8.42
Guy rope (valley)	199000005	3.01	8.73	0.00	0.22	6.39	-8.42
Guy rope (valley)	199000012	3.01	-8.73	0.00	0.22	-6.39	-8.42
Guy rope (valley)	199000014	-3.01	-8.73	0.00	-0.22	-6.39	-8.42
Center pole 5.5m	1	0.00	0.00	0.00	0.00	0.00	0.25
Center pole 5.0m	3	-5.00	0.00	0.00	0.00	0.00	0.17
Center pole 5.0m	5	5.00	0.00	0.00	0.00	0.00	0.17
Corner pole 2.5m	28	8.59	-6.09	0.00	0.07	-0.08	0.97
Corner pole 2.5m	30	-8.59	-6.09	0.00	-0.07	-0.08	0.97
Corner pole 2.5m	35	8.59	6.09	0.00	0.07	0.08	0.97
Corner pole 2.5m	52	-8.59	6.09	0.00	-0.07	0.08	0.97
Entrance pole 3.0m	8	-6.00	-7.00	0.00	0.06	-0.72	5.16
Entrance pole 3.0m	14	0.00	-7.00	0.00	0.00	-0.54	3.70
Entrance pole 3.0m	23	9.50	-2.50	0.00	0.50	0.00	4.21
Entrance pole 3.0m	26	-9.50	-2.50	0.00	-0.50	0.00	4.21
Entrance pole 3.0m	34	9.50	2.50	0.00	0.50	0.00	4.21
Entrance pole 3.0m	39	6.00	7.00	0.00	-0.06	0.72	5.16
Entrance pole 3.0m	45	0.00	7.00	0.00	0.00	0.54	3.70
Entrance pole 3.0m	51	-6.00	7.00	0.00	0.06	0.72	5.16
Entrance pole 3.0m	56	-9.50	2.50	0.00	-0.50	0.00	4.21
Entrance pole 3.0m	20	6.00	-7.00	0.00	-0.06	-0.72	5.16

**Annex D.2.5. CO5: Own weight + pretension + wind suction – floating – full wind load**

Att	Node	Koor X	Koor Y	Koor Z	Fx	Fy	Fz
Storm belt	199000015	-2.80	-8.73	0.00	0.28	-9.68	-11.43
Storm belt	199000016	2.80	-8.73	0.00	-0.56	-9.54	-11.27
Storm belt	199000017	2.80	8.73	0.00	-0.71	9.40	-11.06
Storm belt	199000018	-2.80	8.73	0.00	0.52	9.67	-11.39
Guy rope	9	-6.00	-10.46	0.00	-0.05	-3.55	-3.44
Guy rope	15	0.00	-10.46	0.00	0.01	-2.53	-2.48
Guy rope	24	12.96	-2.50	0.00	7.12	-0.02	-6.73
Guy rope	27	-12.96	-2.50	0.00	-7.07	-0.02	-6.68
Guy rope	32	12.96	2.50	0.00	7.11	0.02	-6.71
Guy rope	37	6.00	10.46	0.00	0.07	3.57	-3.46
Guy rope	43	0.00	10.46	0.00	0.00	2.52	-2.47
Guy rope	49	-6.00	10.46	0.00	-0.06	3.55	-3.45
Guy rope	54	-12.96	2.50	0.00	-7.08	0.01	-6.69
Guy rope	21	6.00	-10.46	0.00	0.07	-3.57	-3.46
Guy rope (corner)	99000026	-10.00	-7.50	0.00	-7.71	-8.11	-11.11
Guy rope (corner)	99000027	-10.00	7.50	0.00	-7.79	8.10	-11.15
Guy rope (corner)	99000028	10.00	7.50	0.00	7.91	8.16	-11.24
Guy rope (corner)	99000029	10.00	-7.50	0.00	7.90	-8.17	-11.24
Guy rope (valley)	199000003	-3.01	8.73	0.00	-0.15	3.97	-4.63
Guy rope (valley)	199000005	3.01	8.73	0.00	0.19	4.20	-4.91
Guy rope (valley)	199000012	3.01	-8.73	0.00	0.18	-4.06	-4.75
Guy rope (valley)	199000014	-3.01	-8.73	0.00	-0.11	-3.96	-4.62
Center pole 5.5m	1	0.00	0.00	0.00	0.00	0.00	0.19
Center pole 5.0m	3	-5.00	0.00	0.00	0.00	0.00	0.04
Center pole 5.0m	5	5.00	0.00	0.00	0.00	0.00	0.06
Corner pole 2.5m	28	8.59	-6.09	0.00	0.04	-0.08	0.96
Corner pole 2.5m	30	-8.59	-6.09	0.00	-0.05	-0.07	0.94
Corner pole 2.5m	35	8.59	6.09	0.00	0.04	0.08	0.96
Corner pole 2.5m	52	-8.59	6.09	0.00	-0.05	0.07	0.95
Entrance pole 3.0m	8	-6.00	-7.00	0.00	0.11	-0.89	6.75
Entrance pole 3.0m	14	0.00	-7.00	0.00	-0.02	-0.70	4.97
Entrance pole 3.0m	23	9.50	-2.50	0.00	0.52	0.01	5.18
Entrance pole 3.0m	26	-9.50	-2.50	0.00	-0.51	0.02	5.10
Entrance pole 3.0m	34	9.50	2.50	0.00	0.52	-0.01	5.18
Entrance pole 3.0m	39	6.00	7.00	0.00	-0.14	0.91	6.95
Entrance pole 3.0m	45	0.00	7.00	0.00	-0.01	0.68	4.80
Entrance pole 3.0m	51	-6.00	7.00	0.00	0.13	0.90	6.85
Entrance pole 3.0m	56	-9.50	2.50	0.00	-0.51	-0.01	5.11
Entrance pole 3.0m	20	6.00	-7.00	0.00	-0.14	-0.91	6.94

### Annex D.3. Point numbers 20x15m – closed



## Annex D.4. Reaction forces 20x15m - closed

### Annex D.4.1. CO1: Own weight + pretension

Att	Node	Koor X	Koor Y	Koor Z	Fx	Fy	Fz
Guy rope	17	0.00	-10.46	0.00	0.00	-1.46	-1.45
Guy rope	40	-4.00	-10.46	0.00	0.02	-1.57	-1.56
Guy rope	43	4.00	-10.46	0.00	-0.02	-1.57	-1.56
Center pole 5.5m	1	0.00	-2.50	0.00	0.00	0.00	1.86
Center pole 5.0m	3	-5.00	-2.50	0.00	0.00	0.01	2.06
Center pole 5.0m	5	5.00	-2.50	0.00	0.00	0.01	2.06
Center pole 4.0m	34	-2.50	1.75	0.00	0.00	0.00	1.10
Center pole 4.0m	36	2.50	1.75	0.00	0.00	0.00	1.10
Corner pole 2.5m	7	8.00	-5.50	0.00	0.16	-0.16	1.08
Corner pole 2.5m	9	-8.00	-5.50	0.00	-0.16	-0.16	1.09
Corner pole 2.5m	11	8.00	5.50	0.00	0.24	0.24	1.55
Corner pole 2.5m	13	-8.00	5.50	0.00	-0.24	0.24	1.55
Entrance pole 3.0m	16	0.00	-7.00	0.00	0.00	-0.19	1.20
Side wall pole 2.5m	18	8.00	-1.83	0.00	0.19	0.01	0.86
Side wall pole 2.5m	20	-8.00	-1.83	0.00	-0.19	0.01	0.86
Side wall pole 2.5m	22	-1.60	5.50	0.00	0.01	0.20	0.94
Side wall pole 2.5m	24	-8.00	1.83	0.00	-0.24	0.00	1.10
Side wall pole 2.5m	26	8.00	1.83	0.00	0.24	0.00	1.10
Side wall pole 2.5m	28	-4.80	5.50	0.00	0.01	0.23	1.05
Side wall pole 2.5m	30	4.80	5.50	0.00	-0.01	0.23	1.05
Side wall pole 2.5m	32	1.60	5.50	0.00	-0.01	0.20	0.94
Entrance pole 3.0m	39	-4.00	-7.00	0.00	-0.04	-0.42	2.60
Entrance pole 3.0m	42	4.00	-7.00	0.00	0.04	-0.42	2.61
Ground point (corner)	99000001	-10.00	7.50	0.00	-0.97	0.98	-0.17
Ground point	99000002	-10.00	5.75	0.00	-0.51	0.07	-0.38
Ground point	99000003	-10.00	4.25	0.00	-0.57	-0.13	-0.48
Ground point	99000004	-10.00	2.75	0.00	-0.63	0.14	-0.64
Ground point	99000005	-10.00	0.75	0.00	-0.57	-0.20	-0.63
Ground point	99000006	-10.00	-0.75	0.00	-0.56	0.19	-0.63
Ground point	99000007	-10.00	-2.75	0.00	-0.69	-0.07	-0.76
Ground point	99000008	-10.00	-4.25	0.00	-0.52	-0.04	-0.52
Ground point	99000009	-10.00	-5.75	0.00	-0.51	0.00	-0.49
Ground point (corner)	99000010	-10.00	-7.50	0.00	-2.91	-1.73	-1.86
Ground point (corner)	99000014	10.00	-7.50	0.00	2.91	-1.73	-1.86
Ground point	99000015	10.00	-5.75	0.00	0.51	0.00	-0.49
Ground point	99000016	10.00	-4.25	0.00	0.52	-0.04	-0.52
Ground point	99000017	10.00	-2.75	0.00	0.69	-0.07	-0.76
Ground point	99000018	10.00	-0.75	0.00	0.55	0.19	-0.63
Ground point	99000019	10.00	0.75	0.00	0.57	-0.20	-0.63
Ground point	99000020	10.00	2.75	0.00	0.63	0.13	-0.64
Ground point	99000021	10.00	4.25	0.00	0.57	-0.13	-0.48

Ground point	99000022	10.00	5.75	0.00	0.50	0.07	-0.38
Ground point (corner)	99000023	10.00	7.50	0.00	0.97	0.98	-0.17
Ground point	99000024	8.25	7.50	0.00	0.06	0.49	-0.38
Ground point	99000025	6.75	7.50	0.00	-0.08	0.55	-0.50
Ground point	99000026	5.25	7.50	0.00	-0.18	0.45	-0.48
Ground point	99000027	3.75	7.50	0.00	0.16	0.51	-0.55
Ground point	99000028	2.25	7.50	0.00	-0.12	0.58	-0.65
Ground point	99000029	0.75	7.50	0.00	0.10	0.50	-0.57
Ground point	99000030	-0.75	7.50	0.00	-0.10	0.50	-0.57
Ground point	99000031	-2.25	7.50	0.00	0.12	0.58	-0.65
Ground point	99000032	-3.75	7.50	0.00	-0.16	0.51	-0.55
Ground point	99000033	-5.25	7.50	0.00	0.18	0.45	-0.48
Ground point	99000034	-6.75	7.50	0.00	0.08	0.55	-0.50
Ground point	99000035	-8.25	7.50	0.00	-0.06	0.49	-0.38

**Annex D.4.2. CO2: Own weight + pretension + conventional / snow**

Att	Node	Koor X	Koor Y	Koor Z	Fx	Fy	Fz
Guy rope	17	0.00	-10.46	0.00	0.00	-2.80	-2.76
Guy rope	40	-4.00	-10.46	0.00	0.05	-2.97	-2.93
Guy rope	43	4.00	-10.46	0.00	-0.05	-2.97	-2.93
Center pole 5.5m	1	0.00	-2.50	0.00	0.00	-0.02	5.74
Center pole 5.0m	3	-5.00	-2.50	0.00	0.11	0.10	6.00
Center pole 5.0m	5	5.00	-2.50	0.00	-0.11	0.10	6.00
Center pole 4.0m	34	-2.50	1.75	0.00	0.02	0.02	3.80
Center pole 4.0m	36	2.50	1.75	0.00	-0.02	0.02	3.80
Corner pole 2.5m	7	8.00	-5.50	0.00	0.33	-0.24	2.17
Corner pole 2.5m	9	-8.00	-5.50	0.00	-0.33	-0.24	2.17
Corner pole 2.5m	11	8.00	5.50	0.00	0.51	0.53	3.33
Corner pole 2.5m	13	-8.00	5.50	0.00	-0.51	0.53	3.33
Entrance pole 3.0m	16	0.00	-7.00	0.00	0.00	-0.42	2.79
Side wall pole 2.5m	18	8.00	-1.83	0.00	0.44	0.04	2.10
Side wall pole 2.5m	20	-8.00	-1.83	0.00	-0.44	0.04	2.10
Side wall pole 2.5m	22	-1.60	5.50	0.00	0.03	0.51	2.43
Side wall pole 2.5m	24	-8.00	1.83	0.00	-0.61	0.02	3.12
Side wall pole 2.5m	26	8.00	1.83	0.00	0.61	0.02	3.11
Side wall pole 2.5m	28	-4.80	5.50	0.00	0.05	0.60	2.81
Side wall pole 2.5m	30	4.80	5.50	0.00	-0.05	0.60	2.81
Side wall pole 2.5m	32	1.60	5.50	0.00	-0.03	0.51	2.43
Entrance pole 3.0m	39	-4.00	-7.00	0.00	-0.08	-0.71	4.75
Entrance pole 3.0m	42	4.00	-7.00	0.00	0.08	-0.71	4.75
Ground point (corner)	99000001	-10.00	7.50	0.00	-1.31	1.41	-0.21
Ground point	99000002	-10.00	5.75	0.00	-1.03	0.09	-0.73
Ground point	99000003	-10.00	4.25	0.00	-0.57	-0.66	-0.42
Ground point	99000004	-10.00	2.75	0.00	-1.13	1.62	-1.16
Ground point	99000005	-10.00	0.75	0.00	-0.94	-1.75	-1.02
Ground point	99000006	-10.00	-0.75	0.00	-0.64	1.25	-0.69
Ground point	99000007	-10.00	-2.75	0.00	-1.73	-0.62	-1.73
Ground point	99000008	-10.00	-4.25	0.00	-0.41	-0.54	-0.33
Ground point	99000009	-10.00	-5.75	0.00	-0.68	-0.12	-0.59
Ground point (corner)	99000010	-10.00	-7.50	0.00	-3.67	-2.12	-2.32
Ground point (corner)	99000014	10.00	-7.50	0.00	3.67	-2.12	-2.32
Ground point	99000015	10.00	-5.75	0.00	0.68	-0.12	-0.59
Ground point	99000016	10.00	-4.25	0.00	0.41	-0.54	-0.33
Ground point	99000017	10.00	-2.75	0.00	1.73	-0.61	-1.73
Ground point	99000018	10.00	-0.75	0.00	0.64	1.24	-0.69
Ground point	99000019	10.00	0.75	0.00	0.93	-1.75	-1.02
Ground point	99000020	10.00	2.75	0.00	1.13	1.61	-1.16
Ground point	99000021	10.00	4.25	0.00	0.57	-0.66	-0.42
Ground point	99000022	10.00	5.75	0.00	1.02	0.09	-0.73
Ground point (corner)	99000023	10.00	7.50	0.00	1.31	1.40	-0.21

Ground point	99000024	8.25	7.50	0.00	0.07	0.88	-0.62
Ground point	99000025	6.75	7.50	0.00	-0.24	0.64	-0.52
Ground point	99000026	5.25	7.50	0.00	0.36	0.96	-1.01
Ground point	99000027	3.75	7.50	0.00	-0.13	0.76	-0.77
Ground point	99000028	2.25	7.50	0.00	0.13	1.11	-1.17
Ground point	99000029	0.75	7.50	0.00	-0.26	0.84	-0.89
Ground point	99000030	-0.75	7.50	0.00	0.26	0.83	-0.89
Ground point	99000031	-2.25	7.50	0.00	-0.13	1.11	-1.17
Ground point	99000032	-3.75	7.50	0.00	0.13	0.76	-0.77
Ground point	99000033	-5.25	7.50	0.00	-0.36	0.96	-1.01
Ground point	99000034	-6.75	7.50	0.00	0.24	0.64	-0.52
Ground point	99000035	-8.25	7.50	0.00	-0.07	0.88	-0.62



**Annex D.4.3. CO3: Own weight + pretension + wind pressure**

Att	Node	Koor X	Koor Y	Koor Z	Fx	Fy	Fz
Guy rope	17	0.00	-10.46	0.00	0.00	-3.78	-3.71
Guy rope	40	-4.00	-10.46	0.00	0.08	-4.15	-4.05
Guy rope	43	4.00	-10.46	0.00	-0.08	-4.15	-4.05
Center pole 5.5m	1	0.00	-2.50	0.00	0.00	-0.04	7.48
Center pole 5.0m	3	-5.00	-2.50	0.00	0.22	0.20	8.38
Center pole 5.0m	5	5.00	-2.50	0.00	-0.22	0.20	8.39
Center pole 4.0m	34	-2.50	1.75	0.00	0.05	0.02	5.49
Center pole 4.0m	36	2.50	1.75	0.00	-0.05	0.01	5.49
Corner pole 2.5m	7	8.00	-5.50	0.00	0.59	-0.25	3.74
Corner pole 2.5m	9	-8.00	-5.50	0.00	-0.59	-0.25	3.75
Corner pole 2.5m	11	8.00	5.50	0.00	0.85	0.88	5.65
Corner pole 2.5m	13	-8.00	5.50	0.00	-0.85	0.88	5.66
Entrance pole 3.0m	16	0.00	-7.00	0.00	0.00	-0.57	3.94
Side wall pole 2.5m	18	8.00	-1.83	0.00	0.75	0.14	3.83
Side wall pole 2.5m	20	-8.00	-1.83	0.00	-0.75	0.14	3.84
Side wall pole 2.5m	22	-1.60	5.50	0.00	0.08	0.86	4.21
Side wall pole 2.5m	24	-8.00	1.83	0.00	-0.97	0.01	5.29
Side wall pole 2.5m	26	8.00	1.83	0.00	0.97	0.01	5.29
Side wall pole 2.5m	28	-4.80	5.50	0.00	0.13	0.98	4.68
Side wall pole 2.5m	30	4.80	5.50	0.00	-0.13	0.98	4.68
Side wall pole 2.5m	32	1.60	5.50	0.00	-0.08	0.86	4.22
Entrance pole 3.0m	39	-4.00	-7.00	0.00	-0.13	-0.92	6.54
Entrance pole 3.0m	42	4.00	-7.00	0.00	0.13	-0.92	6.54
Ground point (corner)	99000001	-10.00	7.50	0.00	-2.07	2.19	-0.33
Ground point	99000002	-10.00	5.75	0.00	-1.90	0.20	-1.32
Ground point	99000003	-10.00	4.25	0.00	-1.21	-0.91	-0.81
Ground point	99000004	-10.00	2.75	0.00	-2.11	2.25	-1.95
Ground point	99000005	-10.00	0.75	0.00	-1.90	-2.45	-1.82
Ground point	99000006	-10.00	-0.75	0.00	-1.48	1.82	-1.38
Ground point	99000007	-10.00	-2.75	0.00	-3.17	-1.20	-2.89
Ground point	99000008	-10.00	-4.25	0.00	-1.03	-0.75	-0.71
Ground point	99000009	-10.00	-5.75	0.00	-1.41	-0.24	-1.15
Ground point (corner)	99000010	-10.00	-7.50	0.00	-4.80	-3.33	-3.08
Ground point (corner)	99000014	10.00	-7.50	0.00	4.80	-3.33	-3.07
Ground point	99000015	10.00	-5.75	0.00	1.41	-0.24	-1.15
Ground point	99000016	10.00	-4.25	0.00	1.03	-0.75	-0.71
Ground point	99000017	10.00	-2.75	0.00	3.17	-1.19	-2.89
Ground point	99000018	10.00	-0.75	0.00	1.48	1.82	-1.38
Ground point	99000019	10.00	0.75	0.00	1.90	-2.45	-1.82
Ground point	99000020	10.00	2.75	0.00	2.11	2.25	-1.95
Ground point	99000021	10.00	4.25	0.00	1.21	-0.91	-0.81
Ground point	99000022	10.00	5.75	0.00	1.89	0.20	-1.32
Ground point (corner)	99000023	10.00	7.50	0.00	2.07	2.19	-0.33

Ground point	99000024	8.25	7.50	0.00	0.18	1.67	-1.15
Ground point	99000025	6.75	7.50	0.00	-0.34	1.28	-0.95
Ground point	99000026	5.25	7.50	0.00	0.54	1.74	-1.67
Ground point	99000027	3.75	7.50	0.00	-0.15	1.54	-1.39
Ground point	99000028	2.25	7.50	0.00	0.22	2.04	-1.97
Ground point	99000029	0.75	7.50	0.00	-0.50	1.68	-1.60
Ground point	99000030	-0.75	7.50	0.00	0.50	1.68	-1.60
Ground point	99000031	-2.25	7.50	0.00	-0.22	2.04	-1.97
Ground point	99000032	-3.75	7.50	0.00	0.15	1.54	-1.39
Ground point	99000033	-5.25	7.50	0.00	-0.53	1.74	-1.67
Ground point	99000034	-6.75	7.50	0.00	0.34	1.28	-0.95
Ground point	99000035	-8.25	7.50	0.00	-0.18	1.67	-1.15

**Annex D.4.4. CO6: Own weight + pretension + wind suction – closed – reduction 0.53**

Att	Node	Koor X	Koor Y	Koor Z	Fx	Fy	Fz
Guy rope	17	0.00	-10.46	0.00	0.00	-6.96	-6.69
Guy rope	40	-4.00	-10.46	0.00	-0.02	-5.91	-5.71
Guy rope	43	4.00	-10.46	0.00	0.02	-5.91	-5.71
Center pole 5.5m	1	0.00	-2.50	0.00	0.00	0.04	1.03
Center pole 5.0m	3	-5.00	-2.50	0.00	-0.03	0.04	1.27
Center pole 5.0m	5	5.00	-2.50	0.00	0.03	0.04	1.27
Center pole 4.0m	34	-2.50	1.75	0.00	0.00	0.01	0.21
Center pole 4.0m	36	2.50	1.75	0.00	0.00	0.01	0.21
Corner pole 2.5m	7	8.00	-5.50	0.00	0.15	-0.10	1.14
Corner pole 2.5m	9	-8.00	-5.50	0.00	-0.15	-0.10	1.14
Corner pole 2.5m	11	8.00	5.50	0.00	0.20	0.19	1.04
Corner pole 2.5m	13	-8.00	5.50	0.00	-0.20	0.19	1.04
Entrance pole 3.0m	16	0.00	-7.00	0.00	0.00	-0.30	2.54
Side wall pole 2.5m	18	8.00	-1.83	0.00	0.11	0.03	0.43
Side wall pole 2.5m	20	-8.00	-1.83	0.00	-0.11	0.03	0.43
Side wall pole 2.5m	22	-1.60	5.50	0.00	0.00	0.05	0.19
Side wall pole 2.5m	24	-8.00	1.83	0.00	-0.21	0.03	0.92
Side wall pole 2.5m	26	8.00	1.83	0.00	0.21	0.03	0.92
Side wall pole 2.5m	28	-4.80	5.50	0.00	0.00	0.08	0.26
Side wall pole 2.5m	30	4.80	5.50	0.00	0.00	0.08	0.26
Side wall pole 2.5m	32	1.60	5.50	0.00	0.00	0.05	0.19
Entrance pole 3.0m	39	-4.00	-7.00	0.00	0.02	-0.68	5.39
Entrance pole 3.0m	42	4.00	-7.00	0.00	-0.02	-0.68	5.39
Ground point (corner)	99000001	-10.00	7.50	0.00	-2.36	2.53	-0.78
Ground point	99000002	-10.00	5.75	0.00	-0.86	0.45	-0.95
Ground point	99000003	-10.00	4.25	0.00	-4.48	2.06	-4.23
Ground point	99000004	-10.00	2.75	0.00	-4.52	-2.90	-4.77
Ground point	99000005	-10.00	0.75	0.00	-4.42	2.31	-5.37
Ground point	99000006	-10.00	-0.75	0.00	-4.19	-3.09	-5.49
Ground point	99000007	-10.00	-2.75	0.00	-1.50	1.36	-2.66
Ground point	99000008	-10.00	-4.25	0.00	-4.45	0.42	-5.57
Ground point	99000009	-10.00	-5.75	0.00	-2.74	-0.98	-3.28
Ground point (corner)	99000010	-10.00	-7.50	0.00	-6.80	-6.24	-5.22
Ground point (corner)	99000014	10.00	-7.50	0.00	6.81	-6.24	-5.22
Ground point	99000015	10.00	-5.75	0.00	2.74	-0.98	-3.27
Ground point	99000016	10.00	-4.25	0.00	4.46	0.42	-5.58
Ground point	99000017	10.00	-2.75	0.00	1.50	1.37	-2.65
Ground point	99000018	10.00	-0.75	0.00	4.19	-3.09	-5.49
Ground point	99000019	10.00	0.75	0.00	4.42	2.31	-5.37
Ground point	99000020	10.00	2.75	0.00	4.52	-2.90	-4.77
Ground point	99000021	10.00	4.25	0.00	4.48	2.06	-4.23
Ground point	99000022	10.00	5.75	0.00	0.86	0.46	-0.95
Ground point (corner)	99000023	10.00	7.50	0.00	2.35	2.52	-0.78

Ground point	99000024	8.25	7.50	0.00	-0.21	0.52	-0.71
Ground point	99000025	6.75	7.50	0.00	0.78	1.43	-2.35
Ground point	99000026	5.25	7.50	0.00	-1.44	0.69	-1.45
Ground point	99000027	3.75	7.50	0.00	1.48	1.47	-2.89
Ground point	99000028	2.25	7.50	0.00	-1.66	1.13	-2.44
Ground point	99000029	0.75	7.50	0.00	1.89	1.52	-3.07
Ground point	99000030	-0.75	7.50	0.00	-1.89	1.53	-3.07
Ground point	99000031	-2.25	7.50	0.00	1.66	1.13	-2.44
Ground point	99000032	-3.75	7.50	0.00	-1.47	1.47	-2.89
Ground point	99000033	-5.25	7.50	0.00	1.44	0.68	-1.45
Ground point	99000034	-6.75	7.50	0.00	-0.79	1.43	-2.35
Ground point	99000035	-8.25	7.50	0.00	0.23	0.52	-0.71

### Annex D.4.5. CO7: Own weight + pretension + wind suction – closed – full wind load

Att	Node	Koor X	Koor Y	Koor Z	Fx	Fy	Fz
Storm belt	99000053	-3.90	-10.46	0.00	0.13	-8.82	-8.49
Storm belt	99000054	3.90	-10.46	0.00	-0.13	-8.70	-8.36
Storm belt	99000055	-10.00	-4.48	0.00	-8.97	1.12	-9.62
Storm belt	99000056	-10.00	-0.08	0.00	-9.90	-1.31	-10.45
Storm belt	99000057	10.00	-0.08	0.00	9.52	-3.87	-9.94
Storm belt	99000060	10.00	-4.48	0.00	9.34	3.36	-9.95
Storm belt	99000061	-10.00	3.48	0.00	-9.71	-1.21	-7.94
Storm belt	99000062	10.00	3.48	0.00	9.73	-0.22	-7.94
Storm belt	99000063	-3.50	7.50	0.00	0.47	3.70	-5.21
Storm belt	99000064	3.50	7.50	0.00	-1.09	3.48	-4.92
Storm belt	99000065	-0.25	7.50	0.00	-0.64	3.55	-4.85
Storm belt	99000066	0.25	7.50	0.00	-0.34	3.40	-4.65
Storm belt	99000067	0.10	-10.46	0.00	-0.22	-6.54	-6.14
Storm belt	99000068	-0.10	-10.46	0.00	0.20	-6.31	-5.94
Guy rope	17	0.00	-10.46	0.00	0.00	-4.31	-4.21
Guy rope	40	-4.00	-10.46	0.00	0.03	-4.31	-4.21
Guy rope	43	4.00	-10.46	0.00	-0.03	-4.29	-4.19
Center pole 5.5m	1	0.00	-2.50	0.00	0.00	0.05	2.45
Center pole 5.0m	3	-5.00	-2.50	0.00	-0.04	0.04	1.47
Center pole 5.0m	5	5.00	-2.50	0.00	0.04	0.04	1.49
Center pole 4.0m	34	-2.50	1.75	0.00	0.00	0.01	0.31
Center pole 4.0m	36	2.50	1.75	0.00	0.00	0.01	0.32
Corner pole 2.5m	7	8.00	-5.50	0.00	0.16	-0.07	1.05
Corner pole 2.5m	9	-8.00	-5.50	0.00	-0.16	-0.07	1.05
Corner pole 2.5m	11	8.00	5.50	0.00	0.24	0.23	1.29
Corner pole 2.5m	13	-8.00	5.50	0.00	-0.24	0.23	1.29
Entrance pole 3.0m	16	0.00	-7.00	0.00	0.01	-0.90	6.38
Side wall pole 2.5m	18	8.00	-1.83	0.00	0.09	0.02	0.34
Side wall pole 2.5m	20	-8.00	-1.83	0.00	-0.09	0.02	0.34
Side wall pole 2.5m	22	-1.60	5.50	0.00	0.01	0.11	0.41
Side wall pole 2.5m	24	-8.00	1.83	0.00	-0.22	0.02	0.93
Side wall pole 2.5m	26	8.00	1.83	0.00	0.22	0.02	0.94
Side wall pole 2.5m	28	-4.80	5.50	0.00	0.00	0.14	0.50
Side wall pole 2.5m	30	4.80	5.50	0.00	-0.01	0.14	0.51
Side wall pole 2.5m	32	1.60	5.50	0.00	-0.01	0.11	0.42
Entrance pole 3.0m	39	-4.00	-7.00	0.00	-0.06	-1.36	9.71
Entrance pole 3.0m	42	4.00	-7.00	0.00	0.07	-1.34	9.57
Ground point (corner)	99000001	-10.00	7.50	0.00	-3.77	3.73	-1.37
Ground point	99000002	-10.00	5.75	0.00	-1.09	0.00	-1.37
Ground point	99000003	-10.00	4.25	0.00	-3.03	-0.30	-3.27
Ground point	99000004	-10.00	2.75	0.00	-3.52	-0.72	-4.31
Ground point	99000005	-10.00	0.75	0.00	-3.00	0.09	-4.19
Ground point	99000006	-10.00	-0.75	0.00	-3.22	-0.65	-4.94

Ground point	99000007	-10.00	-2.75	0.00	-1.84	1.43	-4.03
Ground point	99000008	-10.00	-4.25	0.00	-2.78	-2.61	-3.79
Ground point	99000009	-10.00	-5.75	0.00	-2.28	0.99	-3.21
Ground point (corner)	99000010	-10.00	-7.50	0.00	-7.20	-7.41	-6.31
Ground point (corner)	99000014	10.00	-7.50	0.00	7.12	-7.36	-6.25
Ground point	99000015	10.00	-5.75	0.00	2.35	0.53	-3.26
Ground point	99000016	10.00	-4.25	0.00	2.37	-4.36	-3.42
Ground point	99000017	10.00	-2.75	0.00	1.87	1.37	-4.11
Ground point	99000018	10.00	-0.75	0.00	3.21	0.04	-5.04
Ground point	99000019	10.00	0.75	0.00	3.40	1.81	-4.56
Ground point	99000020	10.00	2.75	0.00	3.63	-1.11	-4.36
Ground point	99000021	10.00	4.25	0.00	2.94	-0.78	-3.23
Ground point	99000022	10.00	5.75	0.00	1.10	0.01	-1.38
Ground point (corner)	99000023	10.00	7.50	0.00	3.76	3.72	-1.36
Ground point	99000024	8.25	7.50	0.00	-0.31	1.03	-1.44
Ground point	99000025	6.75	7.50	0.00	0.97	2.51	-4.10
Ground point	99000026	5.25	7.50	0.00	-2.43	1.25	-2.62
Ground point	99000027	3.75	7.50	0.00	2.35	1.67	-2.77
Ground point	99000028	2.25	7.50	0.00	-1.31	1.58	-3.33
Ground point	99000029	0.75	7.50	0.00	0.56	1.32	-2.47
Ground point	99000030	-0.75	7.50	0.00	0.35	1.11	-2.24
Ground point	99000031	-2.25	7.50	0.00	1.31	1.57	-3.33
Ground point	99000032	-3.75	7.50	0.00	-1.69	1.50	-2.53
Ground point	99000033	-5.25	7.50	0.00	2.41	1.25	-2.63
Ground point	99000034	-6.75	7.50	0.00	-0.97	2.50	-4.09
Ground point	99000035	-8.25	7.50	0.00	0.33	1.03	-1.44

## Annex E. Anchoring of ground points and stormbelts

Explanation of abbreviations:

Fx, Fy and Fz	Reaction forces for x-, y- and z-direction	
Fh,d	Force acting horizontally at the anchor	$1.2 \times \sqrt{(F_x^2 + F_y^2)}$
Fz,d	Force acting vertically at the anchor	$1.2 \times F_z$
Fa	Total Force acting at the anchor	$\sqrt{(F_{h,d}^2 + F_{z,d}^2)}$
$\beta$	Angle of total force	$90 - \tan^{-1} (F_{z,d} / F_{h,d})$
f	for $0 < \beta < 45$	$((f; 45 - f; 0) / 45) \times \beta + f; 0$
n	amount of anchors needed at specific location	
Frd	Capacity of 'n' anchors	$(f \times n \times \varnothing \times L' / 1000)$
UC	unity check	$F_a / F_{rd}$

For the calculations below, anchors  $\varnothing 35 \times 1200\text{mm}$  are used in dense cohesionless soil conditions.

## Annex E.1. Verification of anchoring ground points 20x15m – closed

### Annex E.1.1. Corner, front

#### CO1. Own weight + pretension

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC	
99000010	-2.91	-1.73	-1.86	4.06	-2.23	4.63	61.18	17	1	7.14	0.65	OK
99000014	2.91	-1.73	-1.86	4.06	-2.23	4.63	61.18	17	1	7.14	0.65	OK

#### CO2. Own weight + pretension + conventional / snow

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC	
99000010	-3.67	-2.12	-2.32	5.09	-2.79	5.80	61.30	17	1	7.14	0.81	OK
99000014	3.67	-2.12	-2.32	5.09	-2.79	5.80	61.30	17	1	7.14	0.81	OK

#### CO3. Own weight + pretension + wind pressure

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC	
99000010	-4.80	-3.33	-3.08	7.02	-3.69	7.93	62.25	17	2	14.28	0.56	OK
99000014	4.80	-3.33	-3.07	7.01	-3.69	7.93	62.26	17	2	14.28	0.56	OK

#### CO6. Own weight + pretension + wind suction – closed – reduction 0.53

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC	
99000010	-6.80	-6.24	-5.22	11.08	-6.26	12.73	60.53	17	2	14.28	0.89	OK
99000014	6.81	-6.24	-5.22	11.08	-6.26	12.73	60.52	17	2	14.28	0.89	OK

#### CO7. Own weight + pretension + wind suction – closed – full wind load

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC	
99000010	-7.20	-7.41	-6.31	12.40	-7.58	14.53	58.56	17	2	14.28	1.02	ACCEPTABEL
99000014	7.12	-7.36	-6.25	12.28	-7.50	14.39	58.57	17	2	14.28	1.01	ACCEPTABEL



### Annex E.1.2. Corner, back

#### CO1. Own weight + pretension

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC	
99000001	-0.97	0.98	-0.17	1.65	-0.21	1.67	82.88	17	1	7.14	0.23	OK
99000023	0.97	0.98	-0.17	1.65	-0.21	1.66	82.87	17	1	7.14	0.23	OK

#### CO2. Own weight + pretension + conventional / snow

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC	
99000001	-1.31	1.41	-0.21	2.31	-0.25	2.32	83.76	17	1	7.14	0.33	OK
99000023	1.31	1.40	-0.21	2.30	-0.25	2.32	83.76	17	1	7.14	0.32	OK

#### CO3. Own weight + pretension + wind pressure

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC	
99000001	-2.07	2.19	-0.33	3.62	-0.40	3.64	83.69	17	1	7.14	0.51	OK
99000023	2.07	2.19	-0.33	3.61	-0.40	3.64	83.68	17	1	7.14	0.51	OK

#### CO6. Own weight + pretension + wind suction – closed – reduction 0.53

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC	
99000001	-2.36	2.53	-0.78	4.15	-0.94	4.25	77.21	17	1	7.14	0.60	OK
99000023	2.35	2.52	-0.78	4.14	-0.94	4.24	77.22	17	1	7.14	0.59	OK

#### CO7. Own weight + pretension + wind suction – closed – full wind load

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC	
99000001	-3.77	3.73	-1.37	6.37	-1.64	6.57	75.55	17	1	7.14	0.92	OK
99000023	3.76	3.72	-1.36	6.34	-1.63	6.55	75.56	17	1	7.14	0.92	OK

### Annex E.1.3. Short side, left

#### CO1. Own weight + pretension

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC	
99000002	-0.51	0.07	-0.38	0.61	-0.46	0.76	53.37					
99000003	-0.57	-0.13	-0.48	0.70	-0.58	0.91	50.58					
99000004	-0.63	0.14	-0.64	0.77	-0.77	1.09	45.28					
99000005	-0.57	-0.20	-0.63	0.73	-0.76	1.05	43.84					
99000006	-0.56	0.19	-0.63	0.70	-0.75	1.03	43.11					
99000007	-0.69	-0.07	-0.76	0.83	-0.91	1.23	42.20					
99000008	-0.52	-0.04	-0.52	0.62	-0.62	0.88	45.09					
99000009	-0.51	0.00	-0.49	0.61	-0.59	0.85	46.05					
						0.98	46.19	17.00	1	7.14	0.14	OK

#### CO2. Own weight + pretension + conventional / snow

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC	
99000002	-1.03	0.09	-0.73	1.24	-0.88	1.52	54.54					
99000003	-0.57	-0.66	-0.42	1.04	-0.51	1.16	64.06					
99000004	-1.13	1.62	-1.16	2.37	-1.39	2.74	59.60					
99000005	-0.94	-1.75	-1.02	2.39	-1.22	2.68	62.82					
99000006	-0.64	1.25	-0.69	1.69	-0.83	1.88	63.72					
99000007	-1.73	-0.62	-1.73	2.20	-2.08	3.03	46.66					
99000008	-0.41	-0.54	-0.33	0.81	-0.40	0.90	63.80					
99000009	-0.68	-0.12	-0.59	0.83	-0.71	1.09	49.45					
						1.88	58.08	17.00	1	7.14	0.26	OK

#### CO3. Own weight + pretension + wind pressure

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC	
99000002	-1.90	0.20	-1.32	2.29	-1.59	2.79	55.25					
99000003	-1.21	-0.91	-0.81	1.82	-0.98	2.07	61.75					
99000004	-2.11	2.25	-1.95	3.71	-2.34	4.38	57.76					
99000005	-1.90	-2.45	-1.82	3.73	-2.18	4.32	59.63					
99000006	-1.48	1.82	-1.38	2.82	-1.66	3.27	59.50					
99000007	-3.17	-1.20	-2.89	4.06	-3.47	5.35	49.47					
99000008	-1.03	-0.75	-0.71	1.53	-0.85	1.75	60.94					
99000009	-1.41	-0.24	-1.15	1.72	-1.38	2.20	51.26					
						3.27	56.95	17.00	1	7.14	0.46	OK

CO6. Own weight + pretension + wind suction – closed – reduction 0.53

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC
99000002	-0.86	0.45	-0.95	1.17	-1.14	1.63	45.70				
99000003	-4.48	2.06	-4.23	5.92	-5.08	7.80	49.36				
99000004	-4.52	-2.90	-4.77	6.44	-5.72	8.62	48.41				
99000005	-4.42	2.31	-5.37	5.99	-6.45	8.80	42.91				
99000006	-4.19	-3.09	-5.49	6.25	-6.59	9.08	43.49				
99000007	-1.50	1.36	-2.66	2.43	-3.19	4.01	37.35				
99000008	-4.45	0.42	-5.57	5.37	-6.69	8.58	38.73				
99000009	-2.74	-0.98	-3.28	3.49	-3.93	5.25	41.58				
						6.72	43.44	16.64	1	6.99	0.96 OK

CO7. Own weight + pretension + wind suction – closed – full wind load

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC
99000002	-1.09	0.00	-1.37	1.31	-1.64	2.10	38.51				
99000003	-3.03	-0.30	-3.27	3.65	-3.92	5.36	43.00				
99000004	-3.52	-0.72	-4.31	4.31	-5.17	6.74	39.81				
99000005	-3.00	0.09	-4.19	3.60	-5.03	6.18	35.62				
99000006	-3.22	-0.65	-4.94	3.94	-5.92	7.12	33.66				
99000007	-1.84	1.43	-4.03	2.79	-4.84	5.59	30.00				
99000008	-2.78	-2.61	-3.79	4.57	-4.55	6.45	45.11				
99000009	-2.28	0.99	-3.21	2.99	-3.85	4.88	37.78				
						5.55	37.94	15.35	1	6.45	0.86 OK

### Annex E.1.4. Short side, right

#### CO1. Own weight + pretension

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC	
99000015	0.51	0.00	-0.49	0.61	-0.59	0.85	46.08					
99000016	0.52	-0.04	-0.52	0.62	-0.62	0.88	45.13					
99000017	0.69	-0.07	-0.76	0.83	-0.91	1.23	42.22					
99000018	0.55	0.19	-0.63	0.70	-0.75	1.03	43.09					
99000019	0.57	-0.20	-0.63	0.73	-0.76	1.05	43.85					
99000020	0.63	0.13	-0.64	0.77	-0.77	1.09	45.30					
99000021	0.57	-0.13	-0.48	0.70	-0.58	0.91	50.60					
99000022	0.50	0.07	-0.38	0.61	-0.45	0.76	53.40					
						0.97	46.21	17.00	1	7.14	0.14	OK

#### CO2. Own weight + pretension + conventional / snow

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC	
99000015	0.68	-0.12	-0.59	0.83	-0.71	1.09	49.49					
99000016	0.41	-0.54	-0.33	0.81	-0.40	0.91	63.85					
99000017	1.73	-0.61	-1.73	2.20	-2.07	3.02	46.67					
99000018	0.64	1.24	-0.69	1.68	-0.83	1.87	63.70					
99000019	0.93	-1.75	-1.02	2.38	-1.22	2.68	62.82					
99000020	1.13	1.61	-1.16	2.36	-1.39	2.74	59.58					
99000021	0.57	-0.66	-0.42	1.04	-0.51	1.16	64.03					
99000022	1.02	0.09	-0.73	1.23	-0.88	1.51	54.57					
						1.87	58.09	17.00	1	7.14	0.26	OK

#### CO3. Own weight + pretension + wind pressure

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC	
99000015	1.41	-0.24	-1.15	1.72	-1.38	2.20	51.29					
99000016	1.03	-0.75	-0.71	1.53	-0.85	1.75	60.98					
99000017	3.17	-1.19	-2.89	4.06	-3.47	5.34	49.49					
99000018	1.48	1.82	-1.38	2.81	-1.66	3.27	59.48					
99000019	1.90	-2.45	-1.82	3.72	-2.18	4.31	59.63					
99000020	2.11	2.25	-1.95	3.70	-2.34	4.38	57.75					
99000021	1.21	-0.91	-0.81	1.81	-0.98	2.06	61.74					
99000022	1.89	0.20	-1.32	2.28	-1.58	2.77	55.29					
						3.26	56.96	17.00	1	7.14	0.46	OK

CO6. Own weight + pretension + wind suction – closed – reduction 0.53

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC	
99000015	2.74	-0.98	-3.27	3.49	-3.93	5.25	41.61					
99000016	4.46	0.42	-5.58	5.37	-6.69	8.58	38.75					
99000017	1.50	1.37	-2.65	2.43	-3.18	4.01	37.41					
99000018	4.19	-3.09	-5.49	6.25	-6.59	9.08	43.50					
99000019	4.42	2.31	-5.37	5.99	-6.44	8.80	42.91					
99000020	4.52	-2.90	-4.77	6.45	-5.72	8.62	48.43					
99000021	4.48	2.06	-4.23	5.92	-5.08	7.80	49.36					
99000022	0.86	0.46	-0.95	1.17	-1.14	1.63	45.80					
						6.72	43.47	16.64	1	6.99	0.96	OK

CO7. Own weight + pretension + wind suction – closed – full wind load

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC	
99000015	2.35	0.53	-3.26	2.89	-3.92	4.87	36.45					
99000016	2.37	-4.36	-3.42	5.96	-4.11	7.24	55.43					
99000017	1.87	1.37	-4.11	2.78	-4.93	5.66	29.40					
99000018	3.21	0.04	-5.04	3.86	-6.05	7.18	32.52					
99000019	3.40	1.81	-4.56	4.62	-5.47	7.16	40.18					
99000020	3.63	-1.11	-4.36	4.55	-5.23	6.93	41.07					
99000021	2.94	-0.78	-3.23	3.65	-3.87	5.32	43.28					
99000022	1.10	0.01	-1.38	1.32	-1.66	2.11	38.47					
						5.81	39.60	15.74	1	6.61	0.88	OK

### Annex E.1.5. Long side

#### CO1. Own weight + pretension

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC	
99000024	0.06	0.49	-0.38	0.60	-0.45	0.75	52.93					
99000025	-0.08	0.55	-0.50	0.67	-0.60	0.90	48.07					
99000026	-0.18	0.45	-0.48	0.58	-0.58	0.82	45.18					
99000027	0.16	0.51	-0.55	0.64	-0.66	0.92	43.99					
99000028	-0.12	0.58	-0.65	0.71	-0.78	1.05	42.11					
99000029	0.10	0.50	-0.57	0.61	-0.69	0.92	41.71					
99000030	-0.10	0.50	-0.57	0.61	-0.69	0.92	41.73					
99000031	0.12	0.58	-0.65	0.71	-0.78	1.05	42.12					
99000032	-0.16	0.51	-0.55	0.64	-0.66	0.92	43.98					
99000033	0.18	0.45	-0.48	0.59	-0.58	0.82	45.17					
99000034	0.08	0.55	-0.50	0.67	-0.60	0.90	48.04					
99000035	-0.06	0.49	-0.38	0.60	-0.45	0.75	52.92					
						0.89	45.66	17.00	1	7.14	0.13	OK

#### CO2. Own weight + pretension + conventional / snow

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC	
99000024	0.07	0.88	-0.62	1.05	-0.74	1.29	54.85					
99000025	-0.24	0.64	-0.52	0.82	-0.63	1.03	52.61					
99000026	0.36	0.96	-1.01	1.23	-1.21	1.72	45.47					
99000027	-0.13	0.76	-0.77	0.93	-0.93	1.31	45.01					
99000028	0.13	1.11	-1.17	1.34	-1.41	1.94	43.55					
99000029	-0.26	0.84	-0.89	1.05	-1.07	1.50	44.46					
99000030	0.26	0.83	-0.89	1.05	-1.07	1.50	44.46					
99000031	-0.13	1.11	-1.17	1.34	-1.41	1.94	43.55					
99000032	0.13	0.76	-0.77	0.93	-0.93	1.31	45.01					
99000033	-0.36	0.96	-1.01	1.23	-1.21	1.72	45.46					
99000034	0.24	0.64	-0.52	0.82	-0.63	1.03	52.57					
99000035	-0.07	0.88	-0.62	1.05	-0.74	1.29	54.85					
						1.47	47.66	17.00	1	7.14	0.21	OK

#### CO3. Own weight + pretension + wind pressure

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC
99000024	0.18	1.67	-1.15	2.01	-1.38	2.44	55.61				
99000025	-0.34	1.28	-0.95	1.59	-1.14	1.96	54.35				
99000026	0.54	1.74	-1.67	2.19	-2.00	2.97	47.49				
99000027	-0.15	1.54	-1.39	1.86	-1.67	2.50	48.06				
99000028	0.22	2.04	-1.97	2.47	-2.36	3.42	46.21				
99000029	-0.50	1.68	-1.60	2.11	-1.93	2.85	47.58				
99000030	0.50	1.68	-1.60	2.10	-1.92	2.85	47.58				
99000031	-0.22	2.04	-1.97	2.47	-2.36	3.42	46.22				
99000032	0.15	1.54	-1.39	1.86	-1.67	2.50	48.05				
99000033	-0.53	1.74	-1.67	2.19	-2.01	2.97	47.49				

99000034	0.34	1.28	-0.95	1.59	-1.14	1.96	54.32							
99000035	-0.18	1.67	-1.15	2.01	-1.38	2.44	55.62							
						2.69	49.88	17.00	1		7.14	0.38	OK	

CO6. Own weight + pretension + wind suction – closed – reduction 0.53

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC	
99000024	-0.21	0.52	-0.71	0.67	-0.85	1.09	38.30					
99000025	0.78	1.43	-2.35	1.96	-2.82	3.44	34.71					
99000026	-1.44	0.69	-1.45	1.91	-1.74	2.58	47.74					
99000027	1.48	1.47	-2.89	2.50	-3.47	4.28	35.76					
99000028	-1.66	1.13	-2.44	2.41	-2.93	3.79	39.44					
99000029	1.89	1.52	-3.07	2.91	-3.68	4.70	38.36					
99000030	-1.89	1.53	-3.07	2.91	-3.68	4.70	38.34					
99000031	1.66	1.13	-2.44	2.41	-2.92	3.79	39.44					
99000032	-1.47	1.47	-2.89	2.50	-3.47	4.28	35.73					
99000033	1.44	0.68	-1.45	1.91	-1.74	2.58	47.70					
99000034	-0.79	1.43	-2.35	1.96	-2.82	3.43	34.75					
99000035	0.23	0.52	-0.71	0.68	-0.85	1.09	38.57					
						3.31	39.07	15.62	1	6.56	0.50	OK

CO7. Own weight + pretension + wind suction – closed – full wind load

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC	
99000024	-0.31	1.03	-1.44	1.28	-1.73	2.15	36.64					
99000025	0.97	2.51	-4.10	3.23	-4.91	5.88	33.33					
99000026	-2.43	1.25	-2.62	3.27	-3.15	4.54	46.14					
99000027	2.35	1.67	-2.77	3.46	-3.32	4.79	46.15					
99000028	-1.31	1.58	-3.33	2.46	-4.00	4.69	31.60					
99000029	0.56	1.32	-2.47	1.72	-2.97	3.43	30.07					
99000030	0.35	1.11	-2.24	1.40	-2.69	3.03	27.50					
99000031	1.31	1.57	-3.33	2.45	-4.00	4.69	31.51					
99000032	-1.69	1.50	-2.53	2.71	-3.03	4.07	41.77					
99000033	2.41	1.25	-2.63	3.25	-3.16	4.53	45.89					
99000034	-0.97	2.50	-4.09	3.22	-4.90	5.87	33.29					
99000035	0.33	1.03	-1.44	1.30	-1.73	2.17	36.86					
						4.15	36.73	15.07	1	6.33	0.66	OK

### Annex E.1.6. Storm belts

#### CO7. Own weight + pretension + wind suction – closed – full wind load

Node	Fx	Fy	Fz	Fh,d	Fz,d	Fa	$\beta$	f	n	Frd	UC	
99000053	0.13	-8.82	-8.49	10.59	-10.19	14.69	46.09	17.00	2	14.28	1.03	ACCEPTABEL
99000054	-0.13	-8.70	-8.36	10.44	-10.04	14.48	46.13	17.00	2	14.28	1.01	ACCEPTABEL
99000055	-8.97	1.12	-9.62	10.84	-11.55	15.84	43.21	16.58	3	20.89	0.76	OK
99000056	-9.90	-1.31	-10.45	11.99	-12.54	17.35	43.70	16.70	3	21.04	0.82	OK
99000057	9.52	-3.87	-9.94	12.33	-11.93	17.15	45.93	17.00	3	21.42	0.80	OK
99000060	9.34	3.36	-9.95	11.92	-11.94	16.87	44.93	16.98	3	21.40	0.79	OK
99000061	-9.71	-1.21	-7.94	11.75	-9.53	15.12	50.96	17.00	3	21.42	0.71	OK
99000062	9.73	-0.22	-7.94	11.68	-9.53	15.08	50.79	17.00	3	21.42	0.70	OK
99000063	0.47	3.70	-5.21	4.48	-6.25	7.69	35.60	14.81	2	12.44	0.62	OK
99000064	-1.09	3.48	-4.92	4.38	-5.91	7.36	36.55	15.03	2	12.62	0.58	OK
99000065	-0.64	3.55	-4.85	4.33	-5.82	7.26	36.64	15.05	2	12.64	0.57	OK
99000066	-0.34	3.40	-4.65	4.10	-5.57	6.92	36.34	14.98	2	12.58	0.55	OK
99000067	-0.22	-6.54	-6.14	7.85	-7.37	10.77	46.79	17.00	2	14.28	0.75	OK
99000068	0.20	-6.31	-5.94	7.57	-7.13	10.40	46.73	17.00	2	14.28	0.73	OK